

Rebar Detailing, Shop Drawings and Rebar CNC using Revit + Sofistik

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Class summary

This class shows how ICA use Revit Structure, Inventor and SOFiSTiK Structure Detailing for CNC Rebar Fabrication.

We show how to use point clouds for Quality Assurance (QA), and some in house apps for rebar modeling

Key learning objectives

At the end of this class, you will be able to:

- Know how to use Revit for an effective shop drawing workflow
- Use SOFiSTiK Structural Detailing for exporting Rebar Data to a CNC rebar bending machines.
- Know some rebar tips and tricks
- Get ideas for using 3d scanner for QA.

Introduction

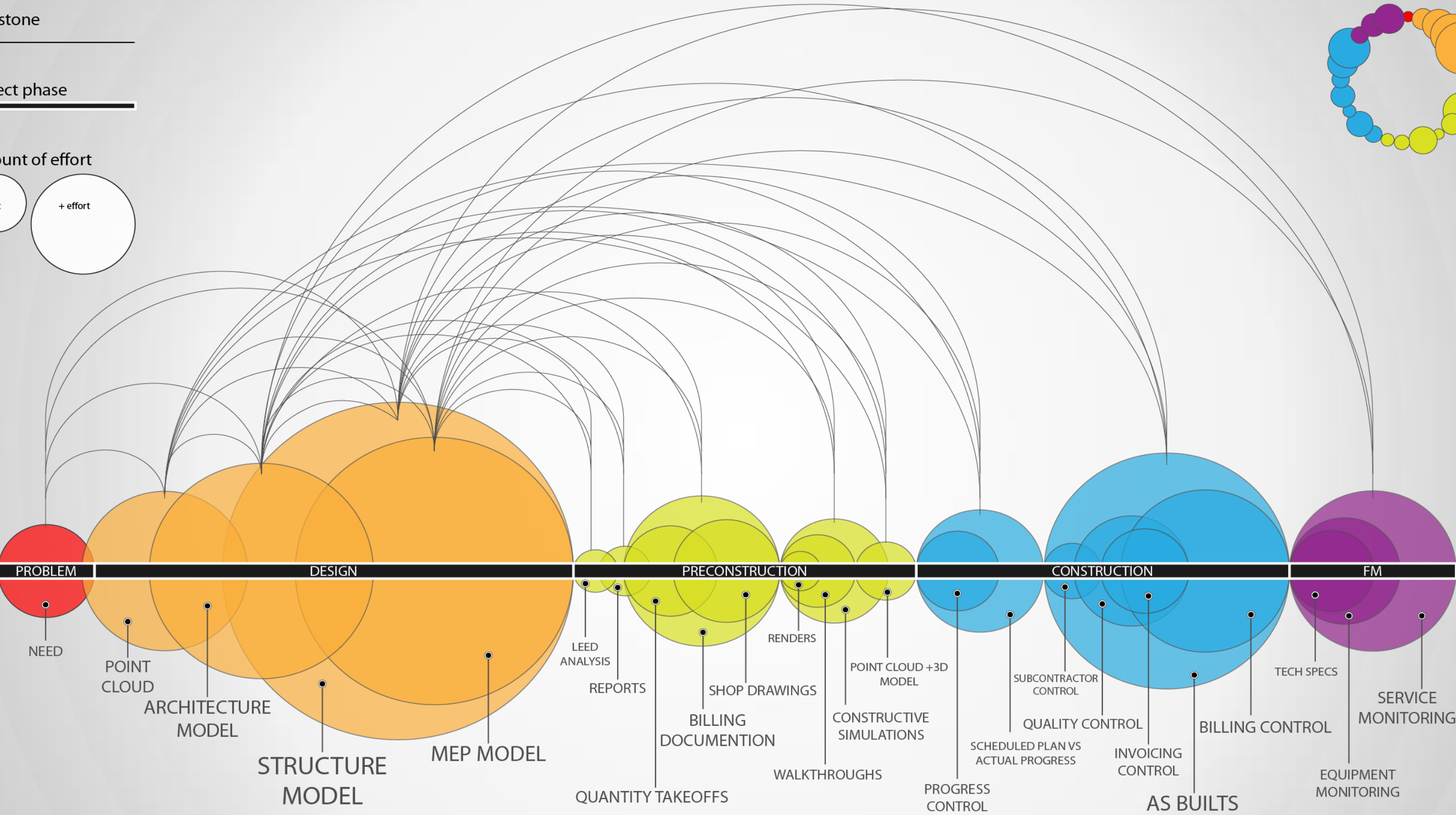
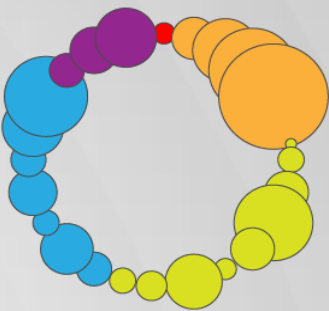
Milestone

Project phase

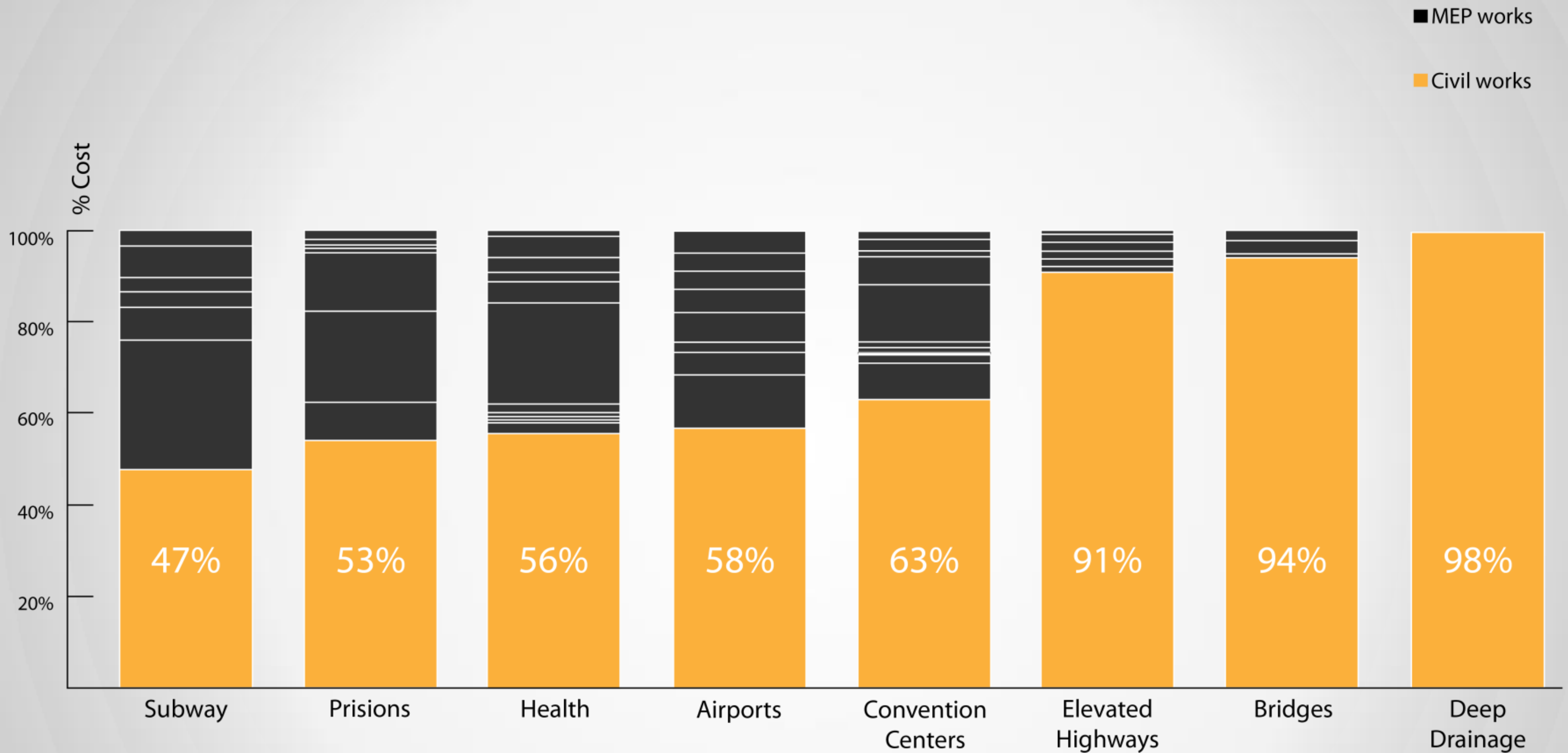
Amount of effort

- effort

+ effort



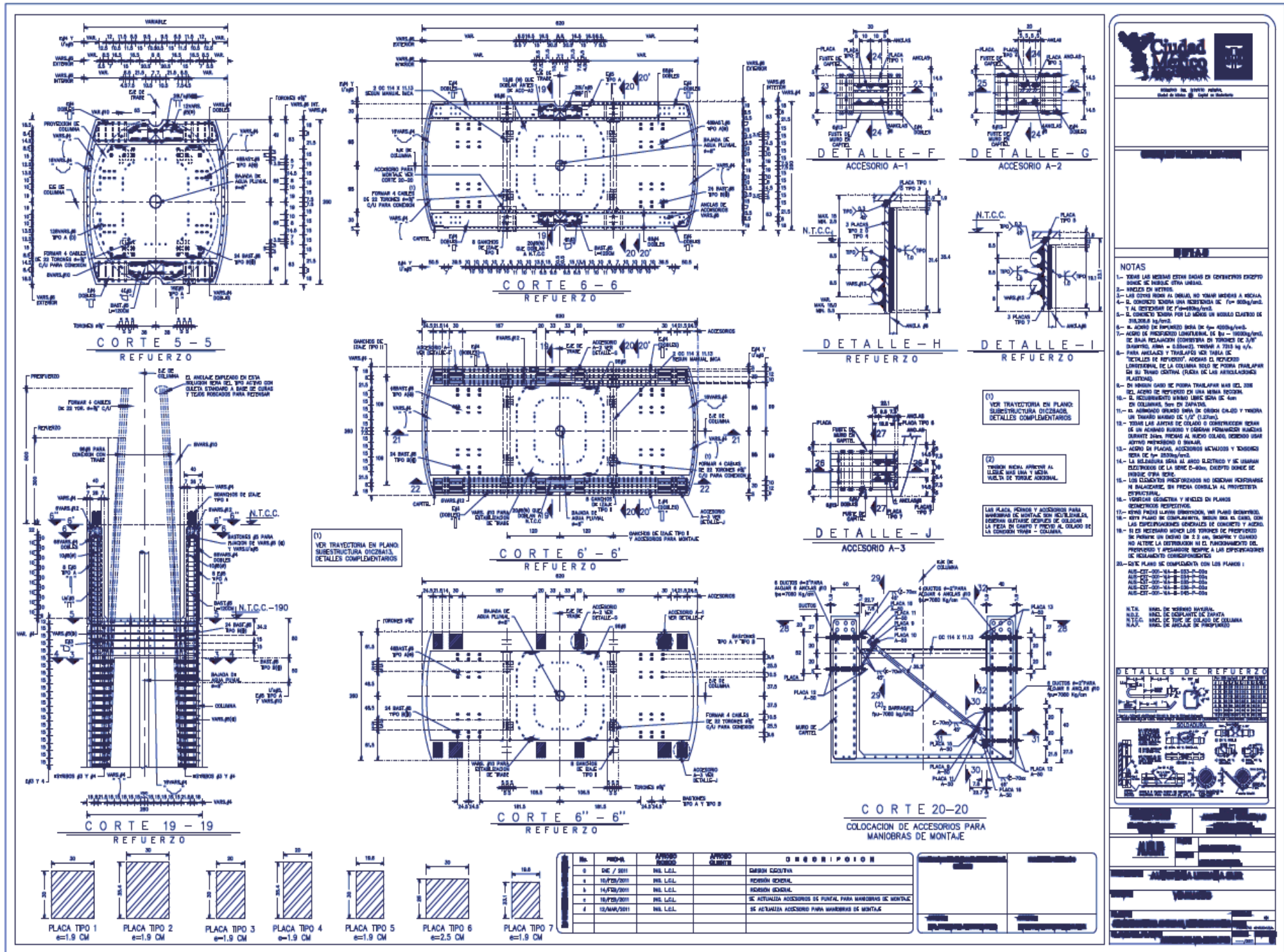
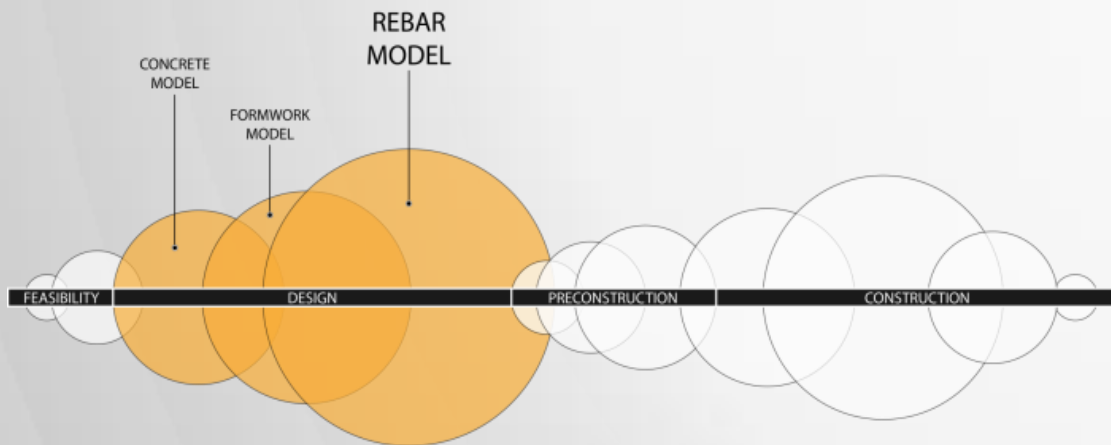




Source: ICA CU, ICA CE , PRET and Bimsa Reports

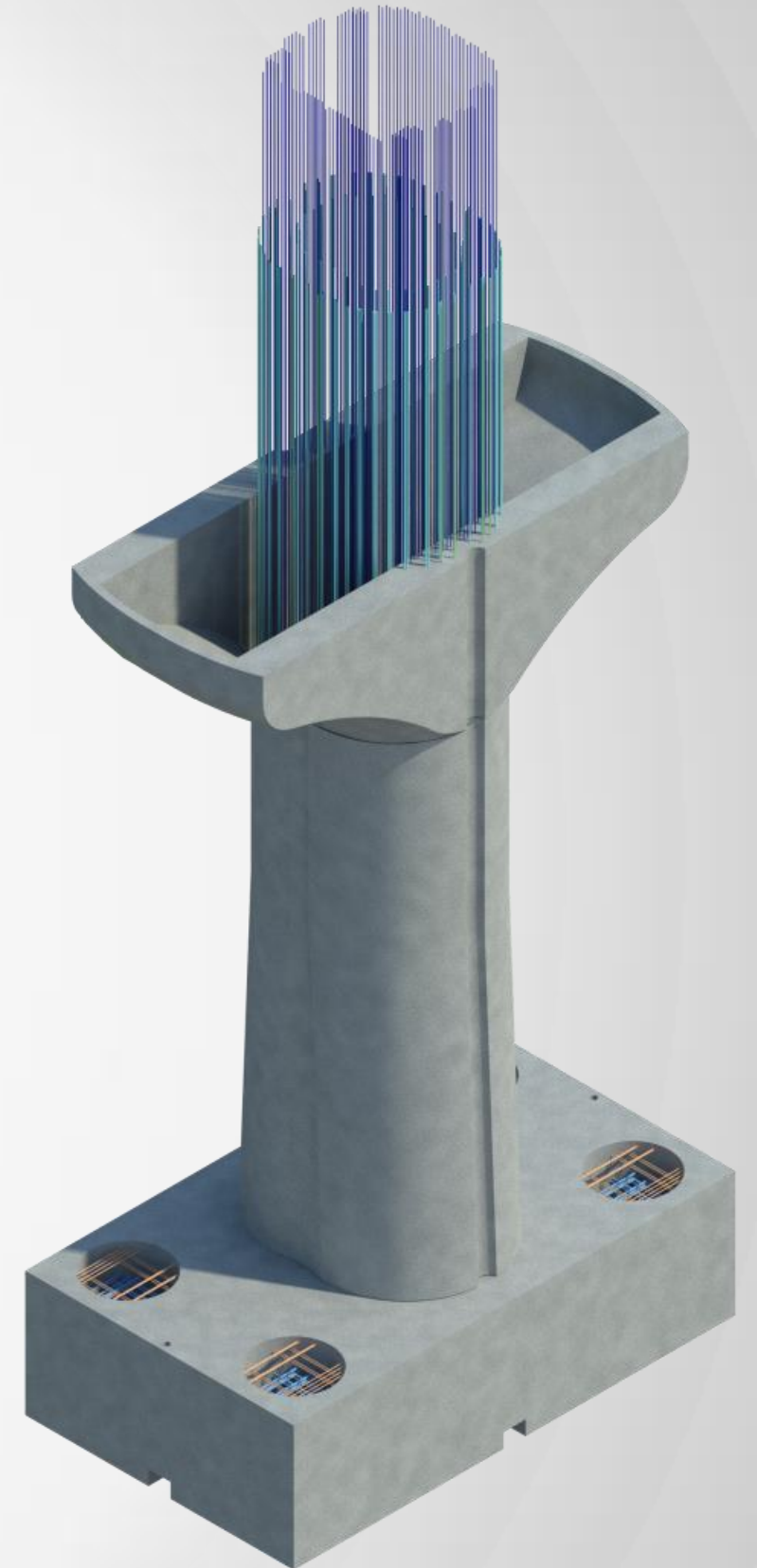
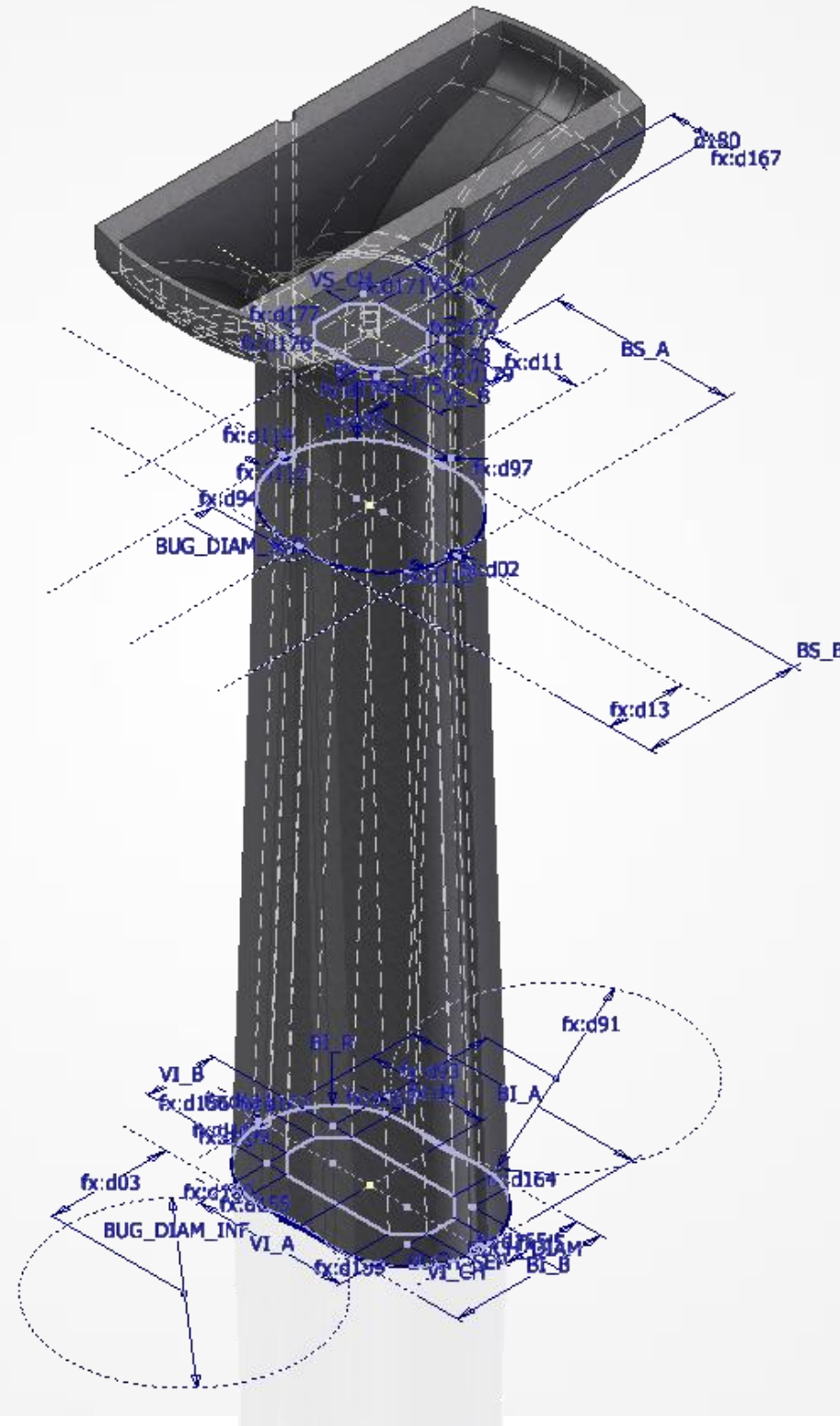
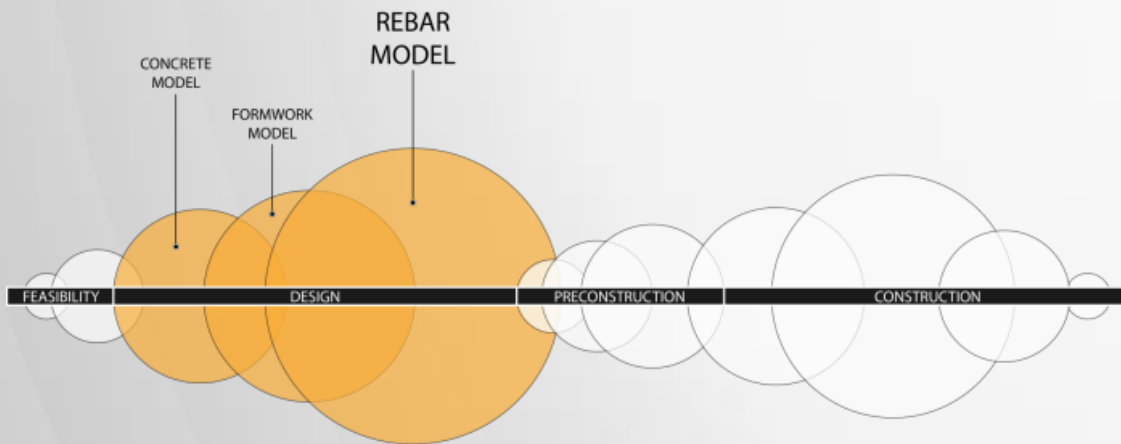
Workflow

PROJECT INPUT.



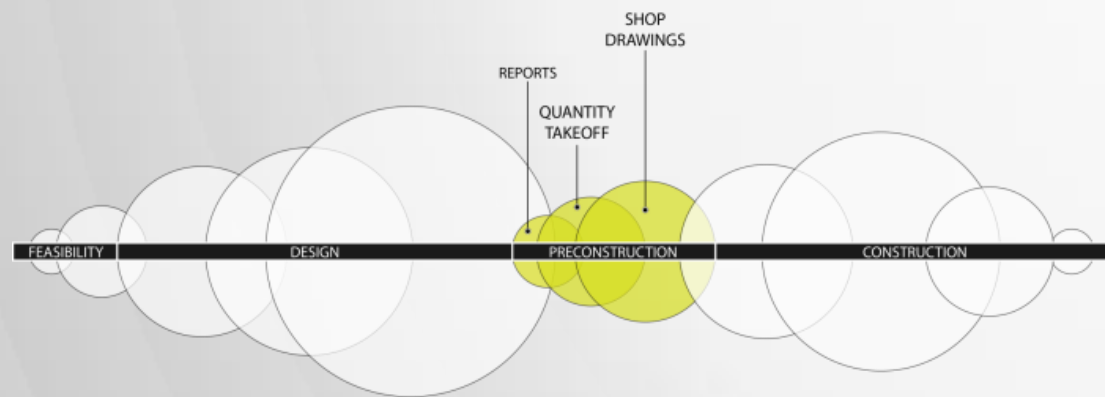
2

BIM MODEL.



3

REPORTS.



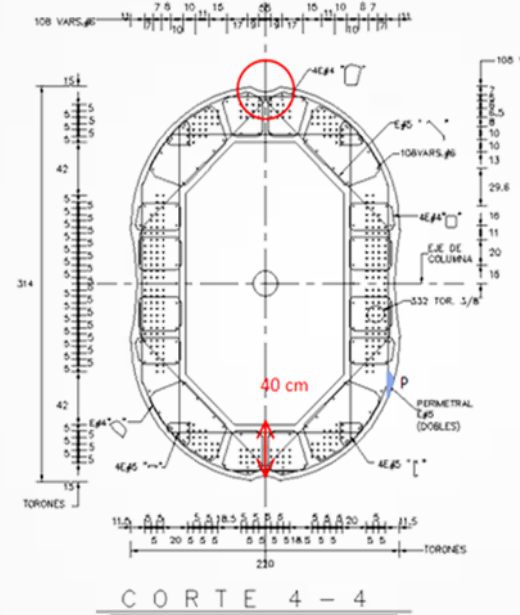
Reporte AS-CZ6A11-001

martes, 05 de octubre de 2010
01:17 p.m.

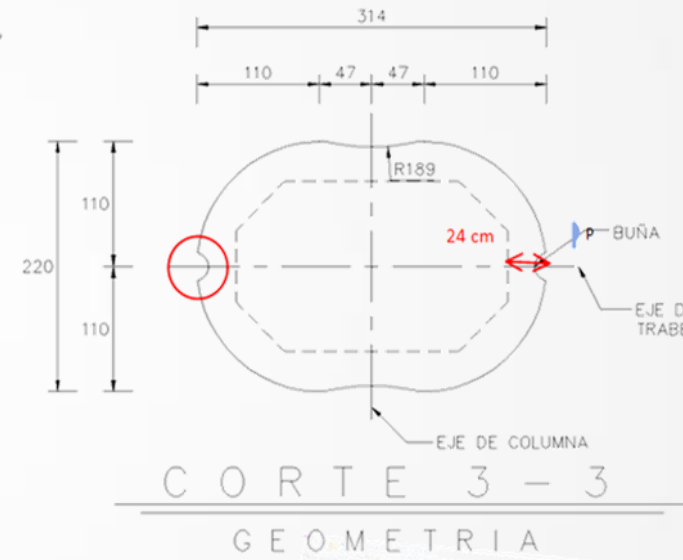
No. Proyecto 10_009 Arco-Sur

Revisión de Proyecto

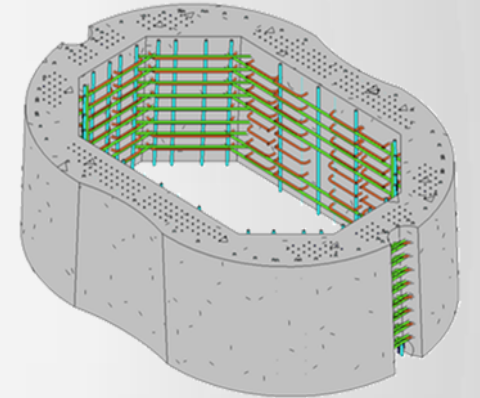
Clave Reporte	Pieza	Nivel	Plano	Estado	No.	Elemento	Descripción	Solución	Tipo	Ejes	Reportó	Soluciono	Fecha Reportado	Fecha Entregado	Fecha resuelto
001	CZ6A11	-	01CZ6A11			Fuste	El Aligeramiento y las buñas del fuste en el plano de geometría no coincide con el aligeramiento en el plano de acero de refuerzo			-	PLA		09/11/2010		



PLANO:
SUBESTRUCTURA 01CZ6A11, REFUERZO 2 DE 2

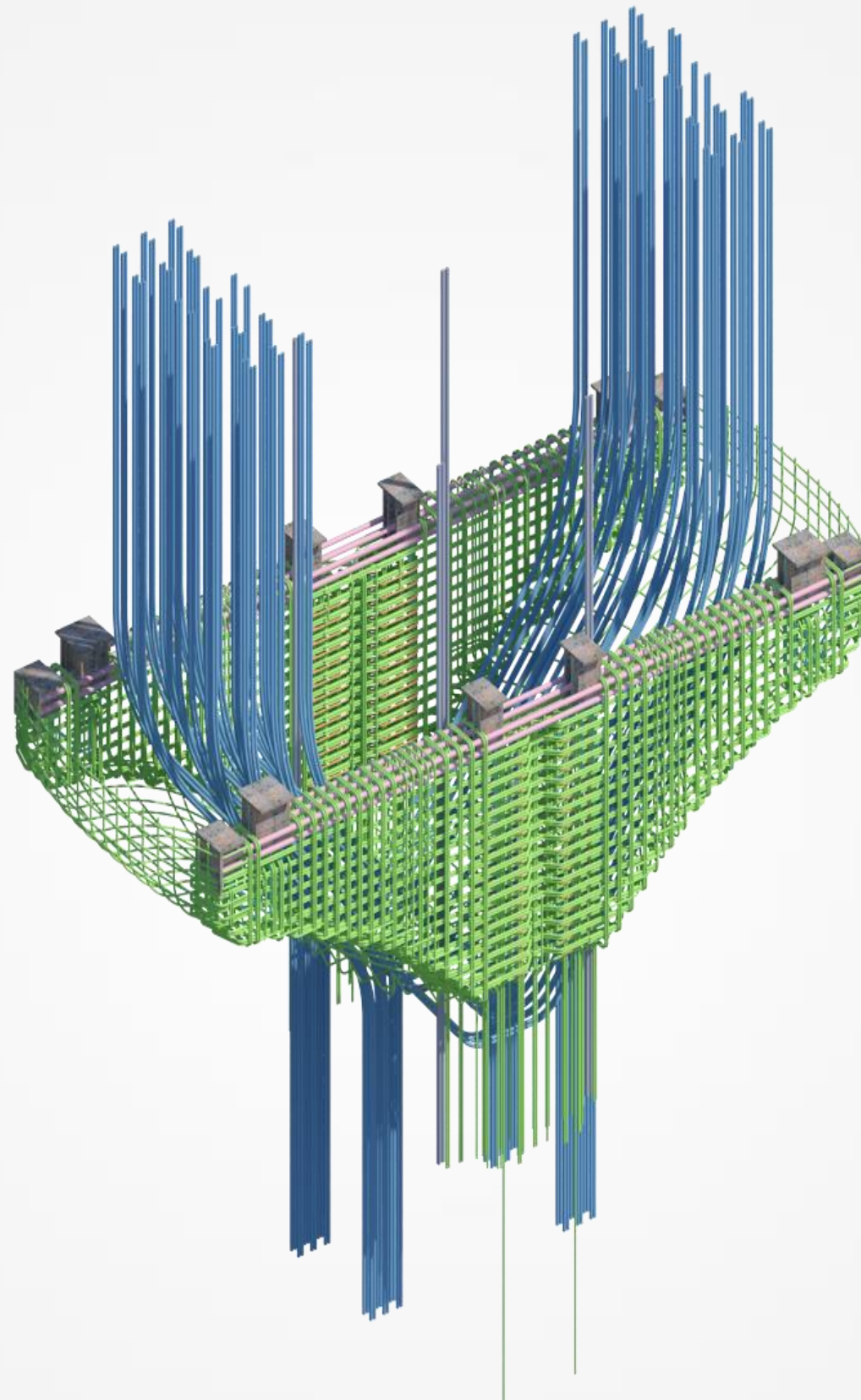
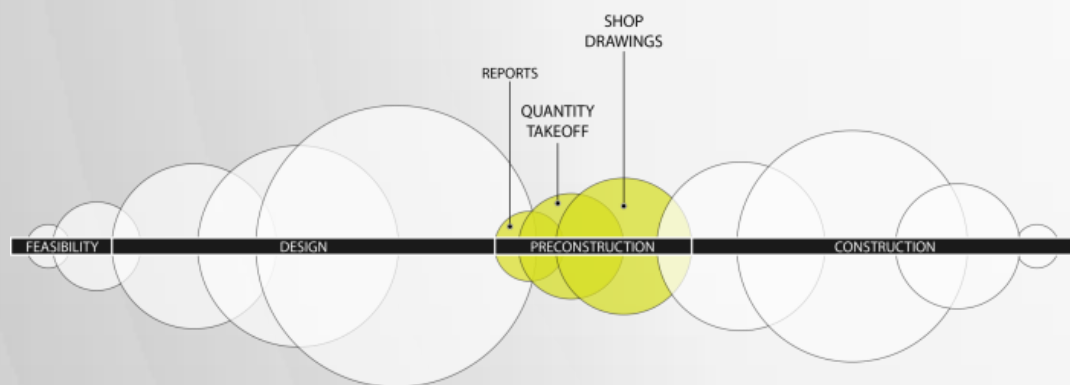


PLANO:
SUBESTRUCTURA 1CZ6A11, GEOMETRÍA 1 DE 2



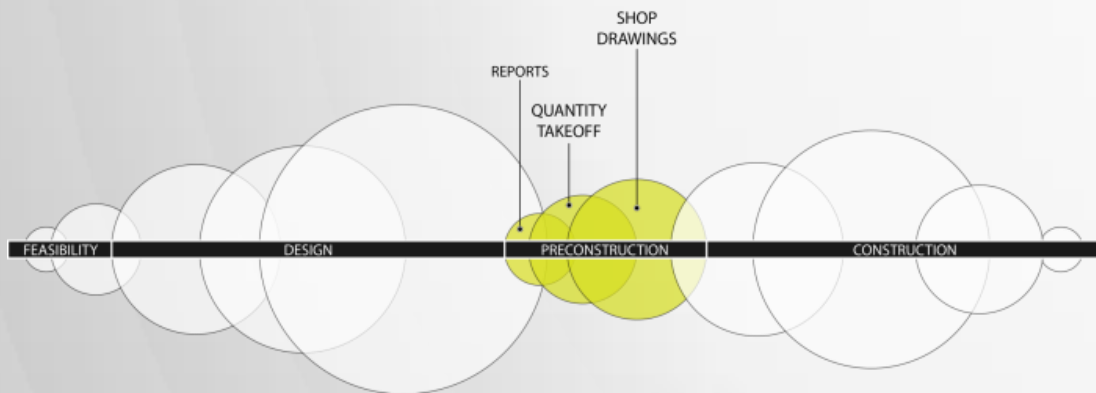
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SOLUTION.



5

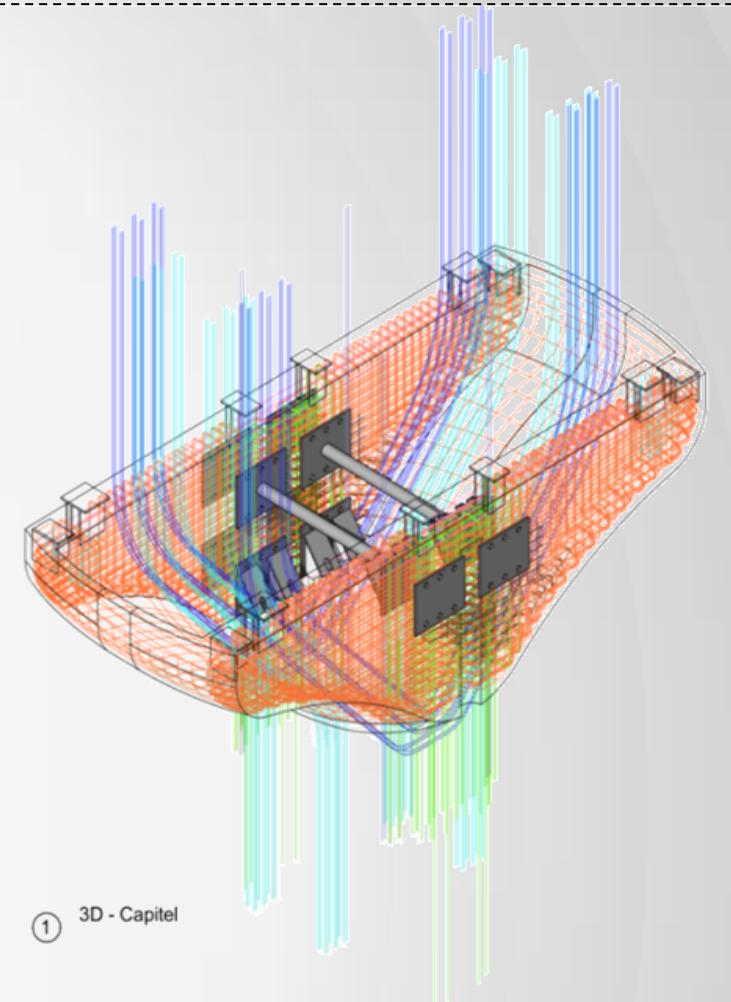
QUANTITY TAKEOFF.



03 - Acero en Capitel						
Codigo	Diametro (mm)	Longitud Unitaria	Cantidad	Suma Longitudinal	Peso Lineal	Peso Total (Kg)
Accesorio						
#06						
CA02	19 mm	0.54 m	20	10.72 m	2.25	24.12
CA02	19 mm	0.83 m	20	16.60 m	2.25	37.35
CA03	19 mm	0.81 m	16	13.01 m	2.25	29.27
			56	40.33 m		90.73
#12						
CA01	38 mm	5.65 m	12	67.82 m	8.938	606.21
CA04	38 mm	2.00 m	4	8.00 m	8.938	71.50
			16	75.82 m		677.71
			72	116.15 m		768.45
Capitel						
#03						
CE06	10 mm	1.35 m	64	86.53 m	0.56	46.46
			64	86.53 m		46.46
#04						
CE01	13 mm	3.54 m	4	14.15 m	0.996	14.09
CE01	13 mm	3.59 m	4	14.34 m	0.996	14.28
CE01	13 mm	3.66 m	4	14.65 m	0.996	14.60
CE01	13 mm	3.71 m	4	14.84 m	0.996	14.78
CE01	13 mm	3.74 m	4	14.97 m	0.996	14.91
CE01	13 mm	3.76 m	12	45.17 m	0.996	44.99
CE02	13 mm	2.98 m	2	5.95 m	0.996	5.93
CE02	13 mm	2.99 m	18	53.90 m	0.996	53.68
CE02	13 mm	3.12 m	2	6.25 m	0.996	6.22
CE02	13 mm	3.15 m	2	6.29 m	0.996	6.27
CE02	13 mm	3.17 m	2	6.35 m	0.996	6.32
CE02	13 mm	3.29 m	1	3.29 m	0.996	3.28
CE02	13 mm	3.31 m	2	6.62 m	0.996	6.59
CE02	13 mm	3.35 m	4	13.39 m	0.996	13.34
CE02	13 mm	3.51 m	4	14.04 m	0.996	13.98
CE03	13 mm	2.26 m	2	4.51 m	0.996	4.49
CE03	13 mm	2.94 m	2	5.87 m	0.996	5.85
CE03	13 mm	3.31 m	2	6.63 m	0.996	6.60
CE03	13 mm	3.36 m	1	3.36 m	0.996	3.35
CE03	13 mm	3.46 m	4	13.84 m	0.996	13.79
CE04	13 mm	1.60 m	8	12.76 m	0.996	12.71
CE04	13 mm	1.73 m	8	13.87 m	0.996	13.81
CE04	13 mm	1.81 m	8	14.52 m	0.996	14.46
CE04	13 mm	1.95 m	8	15.63 m	0.996	15.57
CE04	13 mm	2.10 m	8	16.81 m	0.996	16.74
CE04	13 mm	2.26 m	8	18.09 m	0.996	18.01
CE04	13 mm	2.39 m	8	19.13 m	0.996	19.05
CE04	13 mm	2.57 m	8	20.60 m	0.996	20.52
CE04	13 mm	2.78 m	8	22.21 m	0.996	22.12
CE04	13 mm	3.02 m	8	24.16 m	0.996	24.06
CE04	13 mm	3.33 m	8	26.64 m	0.996	26.53
CE04	13 mm	3.64 m	8	29.11 m	0.996	29.00
CE04	13 mm	3.78 m	8	30.25 m	0.996	30.13
CE04	13 mm	3.80 m	8	30.43 m	0.996	30.30
CE04	13 mm	3.83 m	24	92.00 m	0.996	91.63
CE05	13 mm	2.86 m	4	11.44 m	0.996	11.40
CE05	13 mm	3.01 m	4	12.05 m	0.996	12.00
CE05	13 mm	3.23 m	4	12.92 m	0.996	12.87
CE05	13 mm	3.57 m	4	14.28 m	0.996	14.22
CE05	13 mm	3.65 m	4	14.60 m	0.996	14.54
CE05	13 mm	3.71 m	4	14.82 m	0.996	14.76
CE05	13 mm	3.96 m	4	15.84 m	0.996	15.77
CE05	13 mm	4.35 m	4	17.41 m	0.996	17.34
CE05	13 mm	4.72 m	4	18.89 m	0.996	18.82
CE05	13 mm	5.12 m	4	20.49 m	0.996	20.40
CE05	13 mm	5.59 m	4	22.35 m	0.996	22.26
CE05	13 mm	6.48 m	4	25.93 m	0.996	25.83
CE05	13 mm	6.64 m	4	26.57 m	0.996	26.47

03 - Acero en Capitel						
Codigo	Diametro (mm)	Longitud Unitaria	Cantidad	Suma Longitudinal	Peso Lineal	Peso Total (Kg)
#05						
CE05	13 mm	6.79 m	16	108.57 m	0.996	106.13
CE08	13 mm	1.49 m	8	11.93 m	0.996	11.88
CE08	13 mm	1.90 m	12	22.82 m	0.996	22.73
CE08	13 mm	2.02 m	12	24.19 m	0.996	24.09
CE08	13 mm	2.33 m	8	18.65 m	0.996	18.57
CE08	13 mm	2.35 m	8	18.79 m	0.996	18.71
CE08	13 mm	2.54 m	8	20.32 m	0.996	20.24
CE08	13 mm	2.60 m	8	20.80 m	0.996	20.72
CE08	13 mm	2.72 m	8	21.76 m	0.996	21.67
CE08	13 mm	2.80 m	8	22.40 m	0.996	22.31
CE08	13 mm	2.90 m	8	23.24 m	0.996	23.14
CE08	13 mm	2.98 m	8	23.86 m	0.996	23.76
CE08	13 mm	3.08 m	8	24.61 m	0.996	24.51
CE08	13 mm	3.17 m	8	25.32 m	0.996	25.22
CE08	13 mm	3.28 m	8	26.24 m	0.996	26.13
CE08	13 mm	3.40 m	4	13.60 m	0.996	13.54
CE08	13 mm	3.50 m	4	14.01 m	0.996	13.95
CE08	13 mm	3.58 m	8	28.63 m	0.996	28.51
CE08	13 mm	3.68 m	4	14.73 m	0.996	14.67
CE08	13 mm	3.78 m	4	15.14 m	0.996	15.08
CE08	13 mm	3.93 m	8	31.41 m	0.996	31.28
CE08	13 mm	4.09 m	8	32.74 m	0.996	32.61
CE08	13 mm	4.33 m	8	34.65 m	0.996	34.51
CE08	13 mm	4.48 m	8	35.83 m	0.996	35.69
			400	1,516.44 m		1,510.37
#06						
CB03	16 mm	1.20 m	68	81.60 m	1.56	127.30
CB07	16 mm	5.99 m	8	47.88 m	1.56	74.69
CB07	16 mm	6.16 m	8	49.31 m	1.56	76.93
CB09	16 mm	6.06 m	4	24.24 m	1.56	37.81
CB09	16 mm	6.10 m	8	48.79 m	1.56	76.12
CB09	16 mm	6.11 m	4	24.43 m	1.56	38.11
CB09	16 mm	6.12 m	4	24.49 m	1.56	38.20
CB09	16 mm	6.15 m	8	49.20 m	1.56	76.74
CB10	16 mm	5.11 m	8	40.87 m	1.56	63.76
			120	390.81 m		609.66
#06						
CB01	19 mm	7.42 m	16	118.76 m	2.25	267.22
CB01	19 mm	7.47 m	16	119.49 m	2.25	268.84
CB01	19 mm	7.53 m	16	120.45 m	2.25	271.02
			48	358.71 m		807.09
#06						
CB02	25 mm	11.69 m	4	46.75 m	3.975	185.84
CB02	25 mm	11.77 m	4	47.09 m	3.975	187.18
CB02	25 mm	12.30 m	4	49.20 m	3.975	195.56
CB02	25 mm	12.40 m	4	49.60 m	3.975	197.15
CB02	25 mm	12.95 m	4	51.80 m	3.975	206.91
CB02	25 mm	13.05 m	4	52.20 m	3.975	207.30
			24	296.64 m		1,179.14
#10						
CC01	32 mm	4.94 m	8	39.50 m	6.225	245.86
			8	39.50 m		245.86
			724	2,688.62 m		4,400.58
736			796	2,804.77 m		5,169.03
Volumetria Capitel						
Pertenece a		No Pieza		Volumen		
Capitel		C26A11		7.762 m³		
				7.762 m³		

① 3D - Capitel



Reporto: STG



Proyecto:
AS - Columna 6 Carriles - 11 mts
C26A11 - 1100
Modelo No: 03
NP: 1
No. de Proyecto:
10_009
Dirección:

Reporte:

Resumen de Acero - Capitel

Descripción:

Version No: 20110404

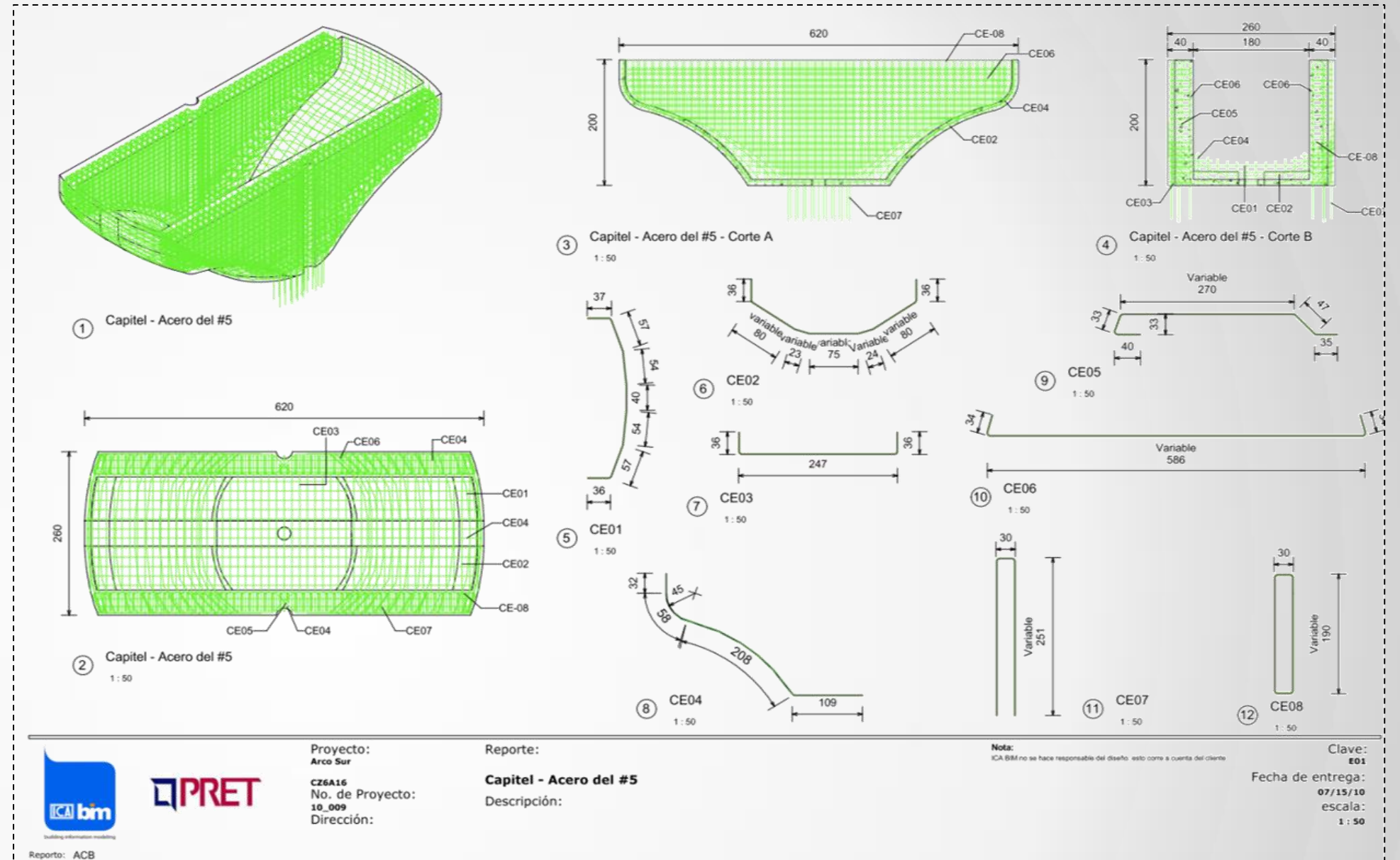
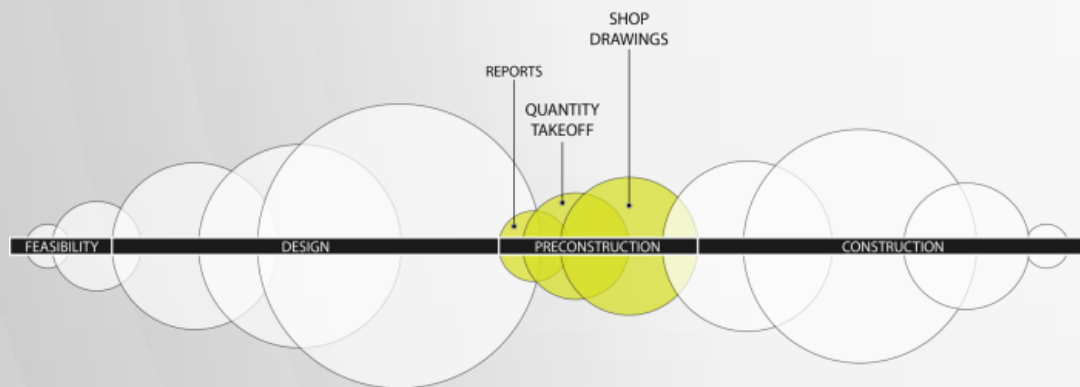
Nota:
ICA BIM no se hace responsable del diseño, esto corre a cuenta del cliente

Clave:
EC000

Fecha de entrega:
20/04/2011
escala:

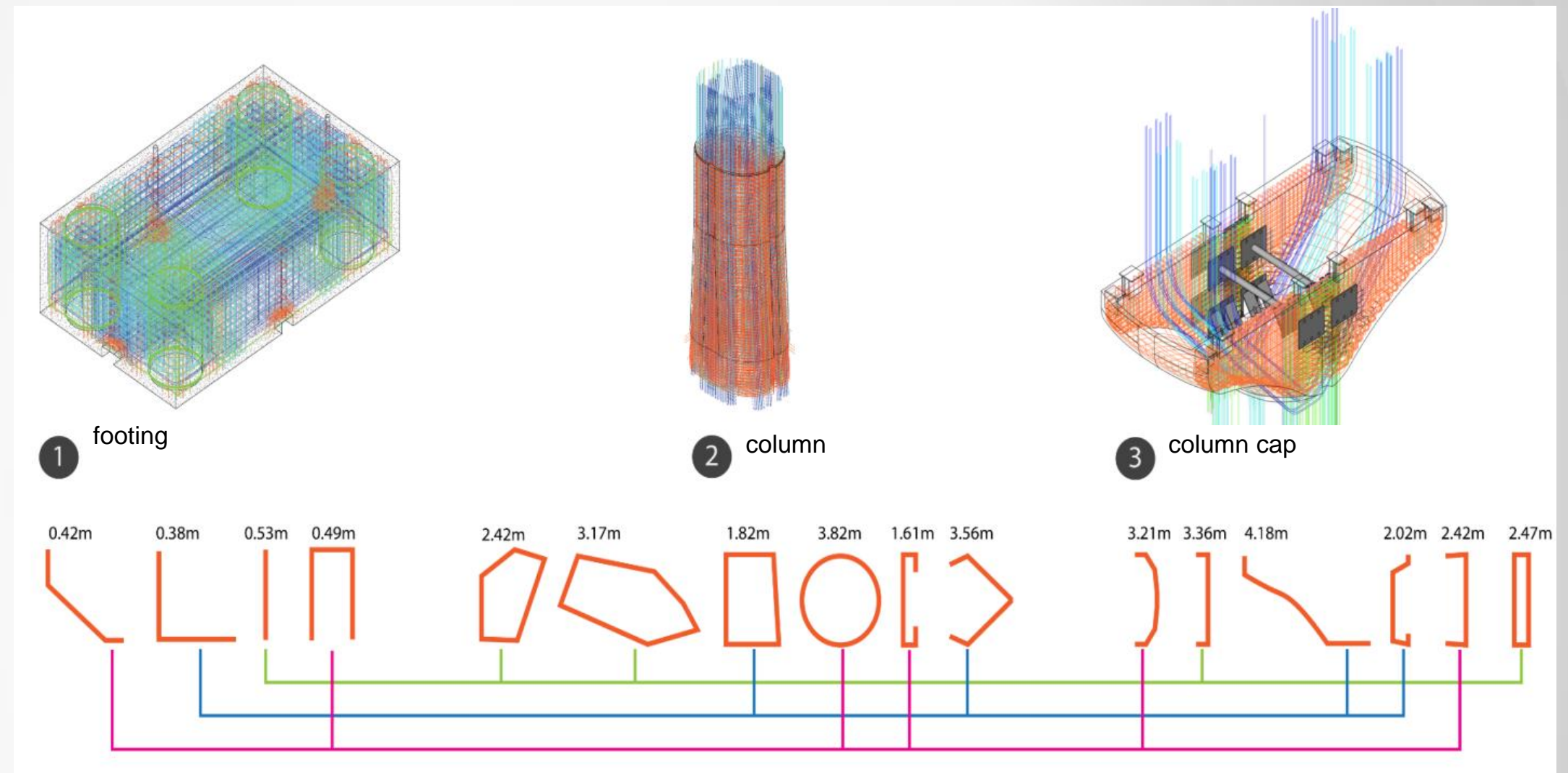
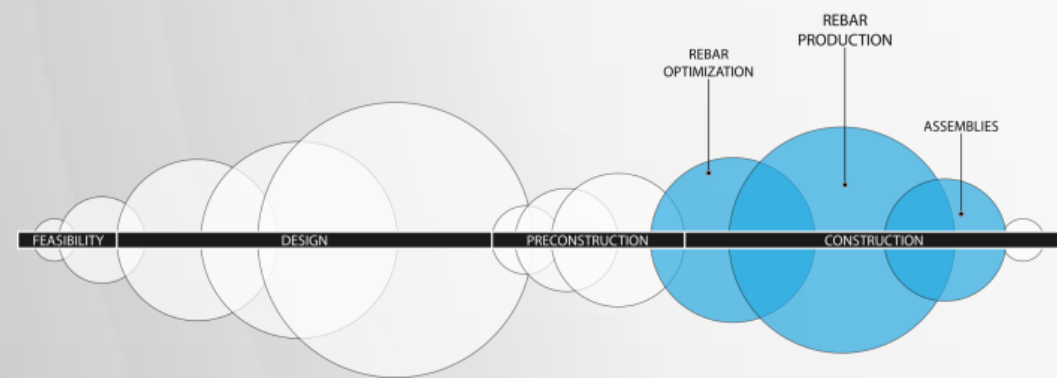
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SHOP DRAWINGS.



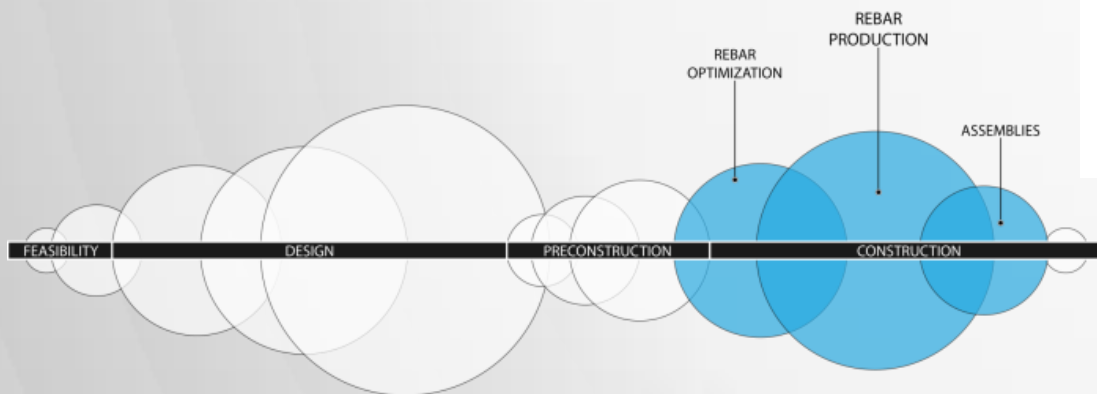
7

REBAR OPTIMIZATION.

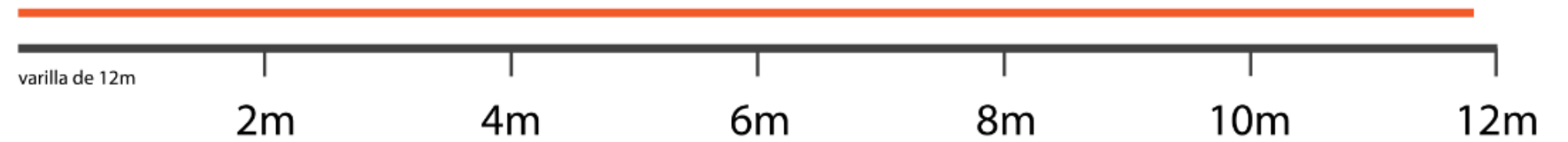


8

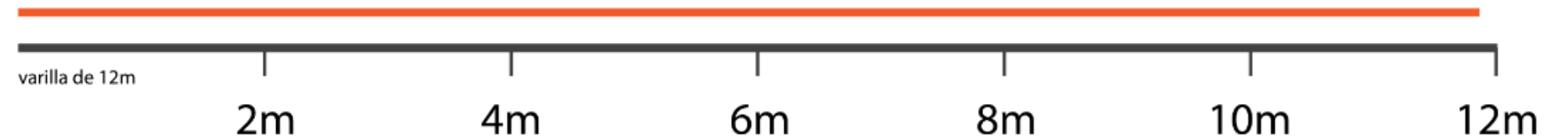
REBAR OPTIMIZATION.



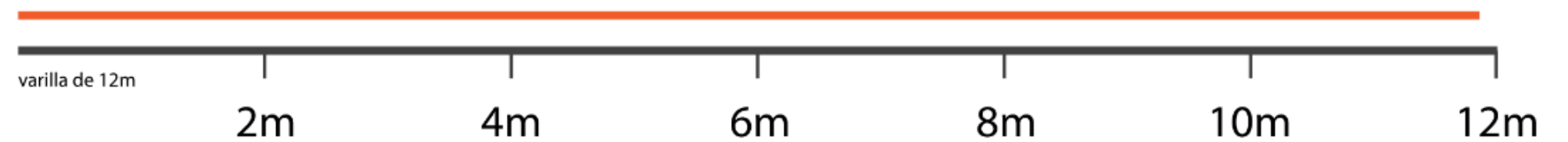
$$| + \text{pentagon} + \text{hexagon} +] + \text{rectangle} = 11.95\text{m} \text{ desperdicio} = 5\text{cm}$$



$$\text{L} + \text{rectangle} + \text{pentagon} + \text{trapezoid} + \text{bracket} = 11.96\text{m} \text{ desperdicio} = 4\text{cm}$$



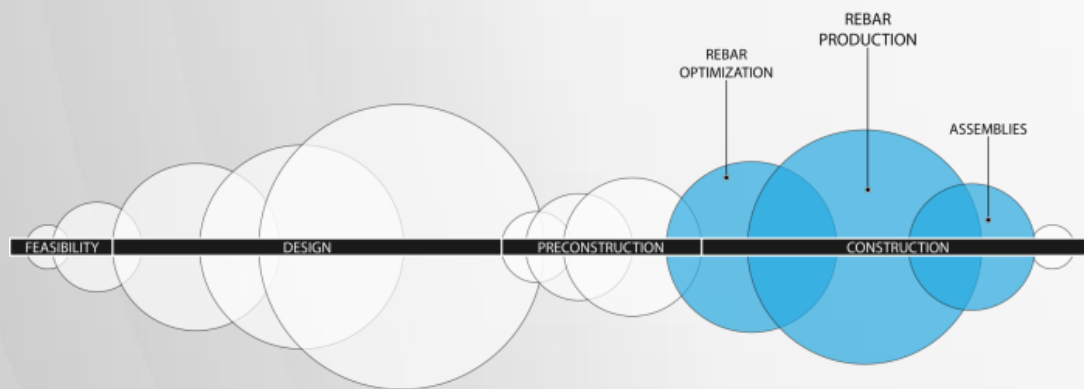
$$\text{L} + \text{rectangle} + \text{circle} + [+) +] = 11.97\text{m} \text{ desperdicio} = 3\text{cm}$$



9

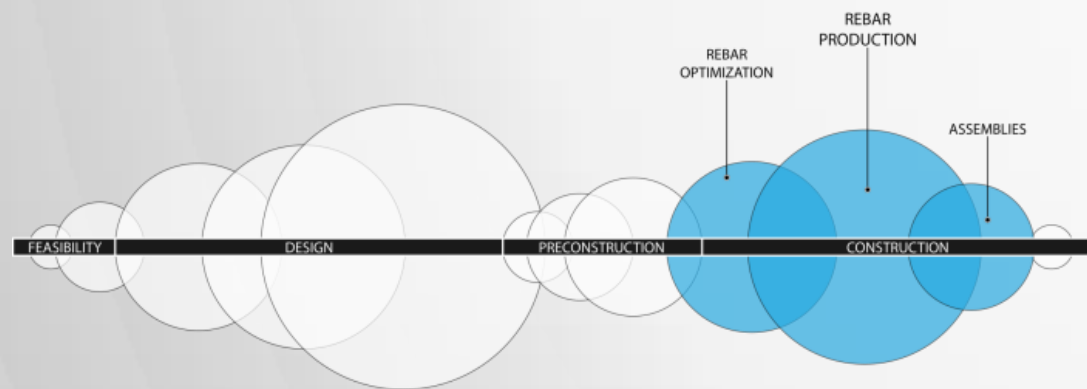
REBAR PRODUCTION.

Hand made production 1,500 ton = **400 people** in 1 month
Rebar production plant 1,500 ton = **20 people** in 1 month

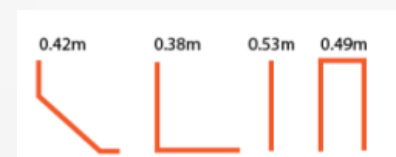


10

REBAR ASSEMBLIES.

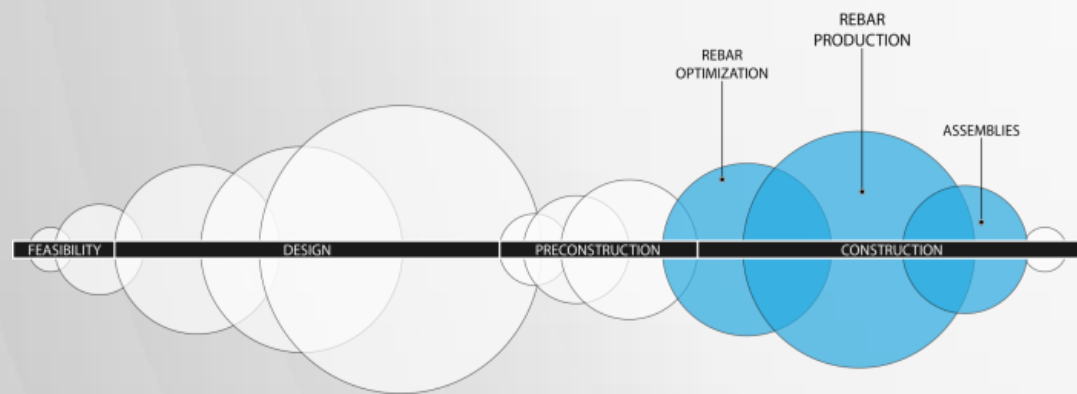


Less Equipment, accidents, cost and waste
More Quality, control and performance



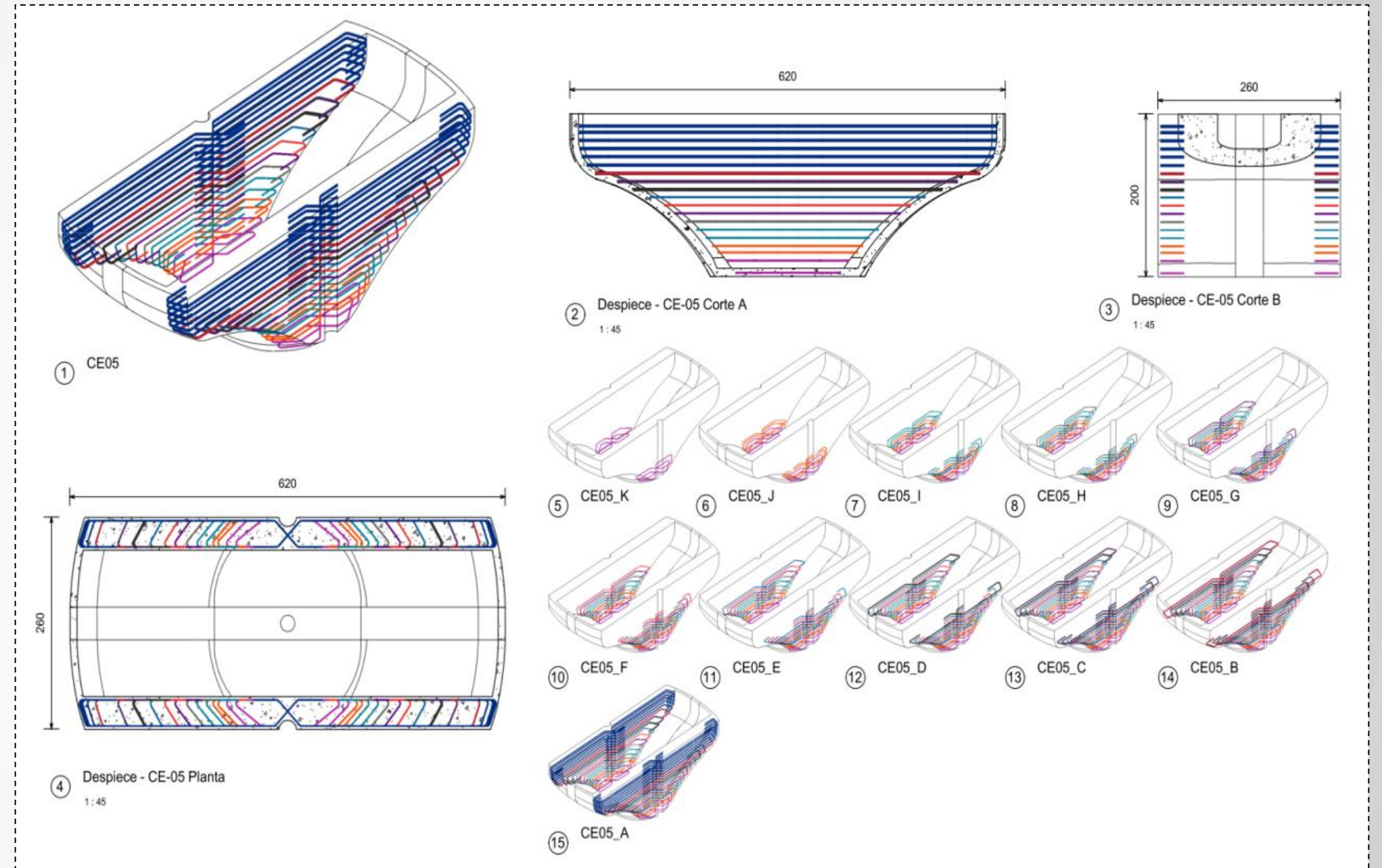
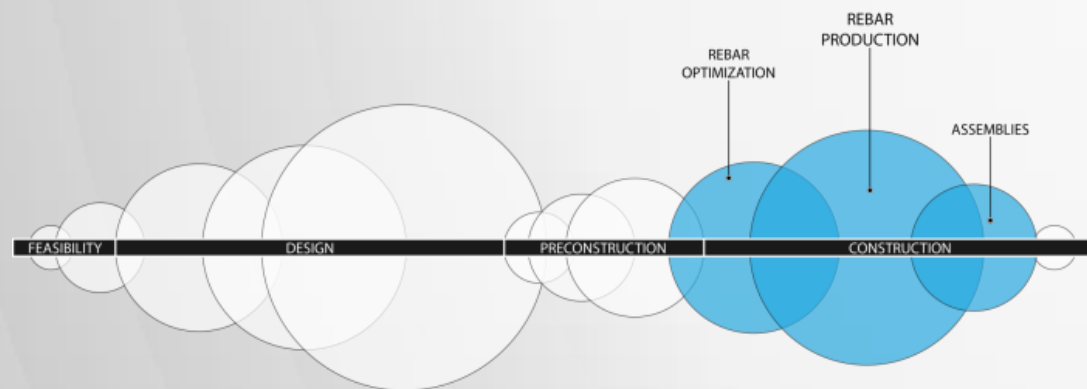
11

ASSEMBLIES MANUAL.



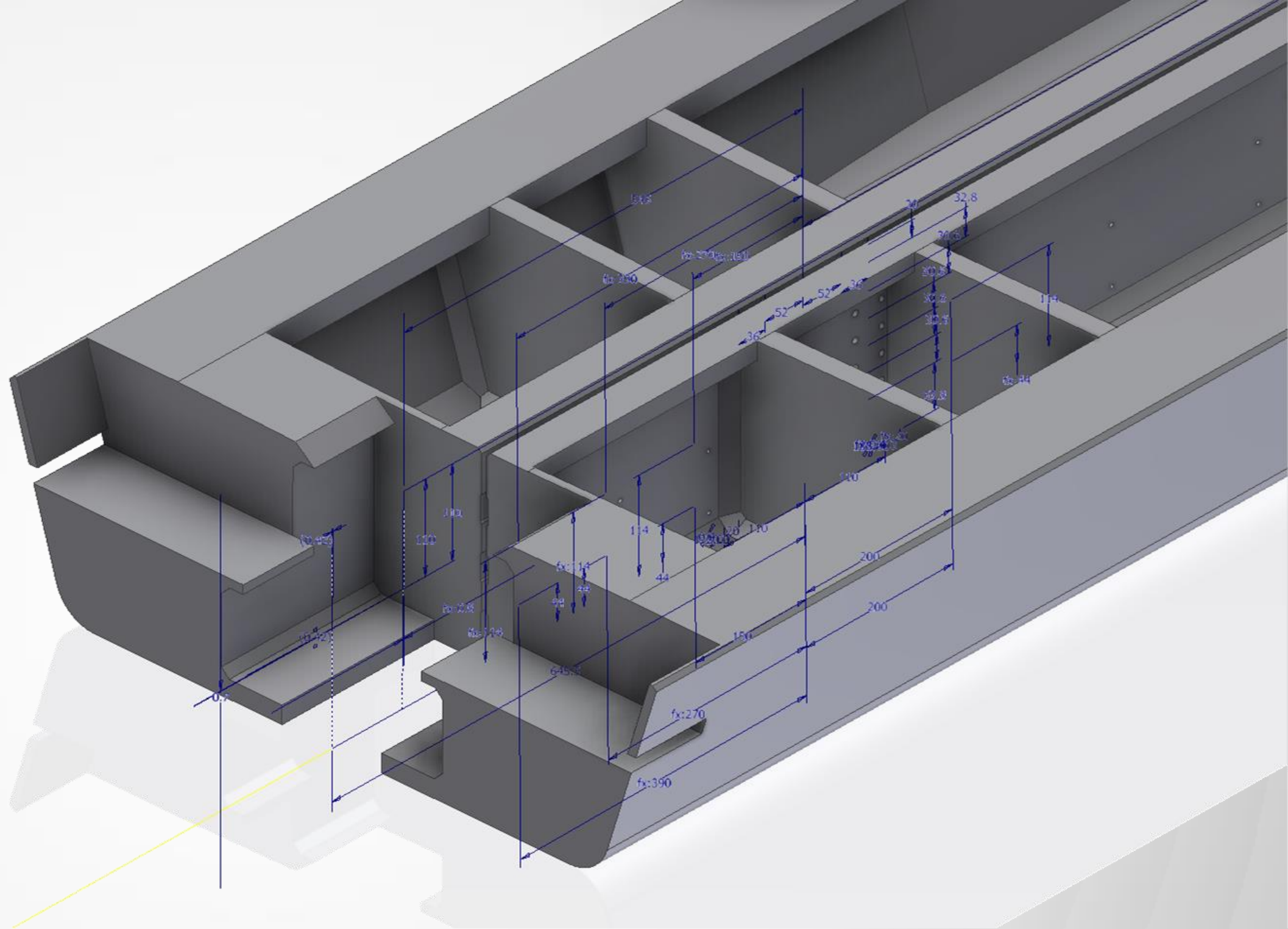
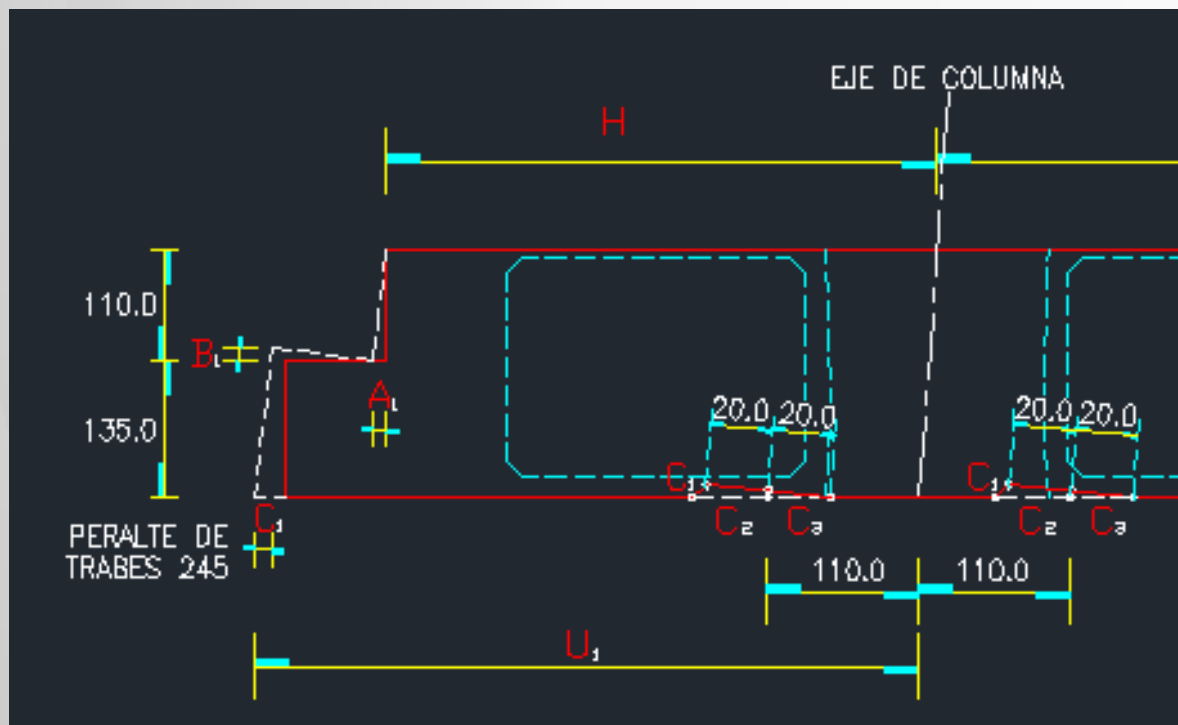
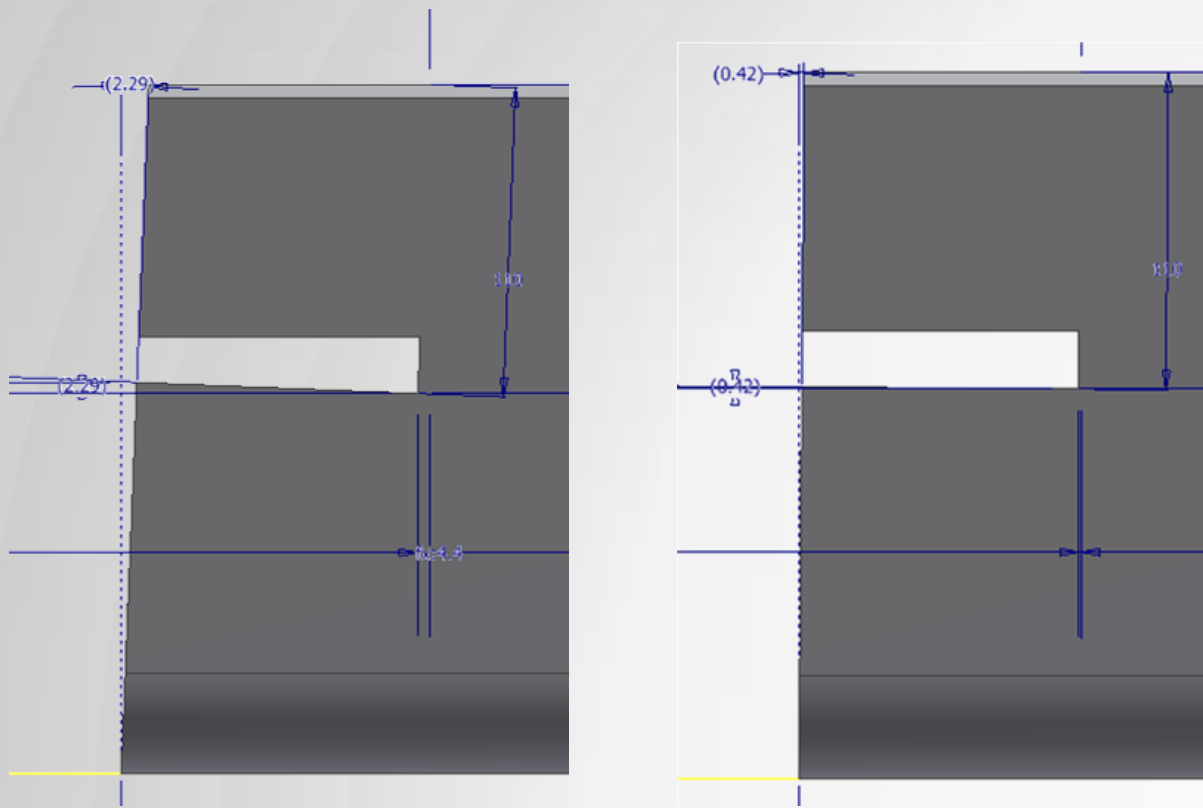
12

ASSEMBLIES MANUAL.

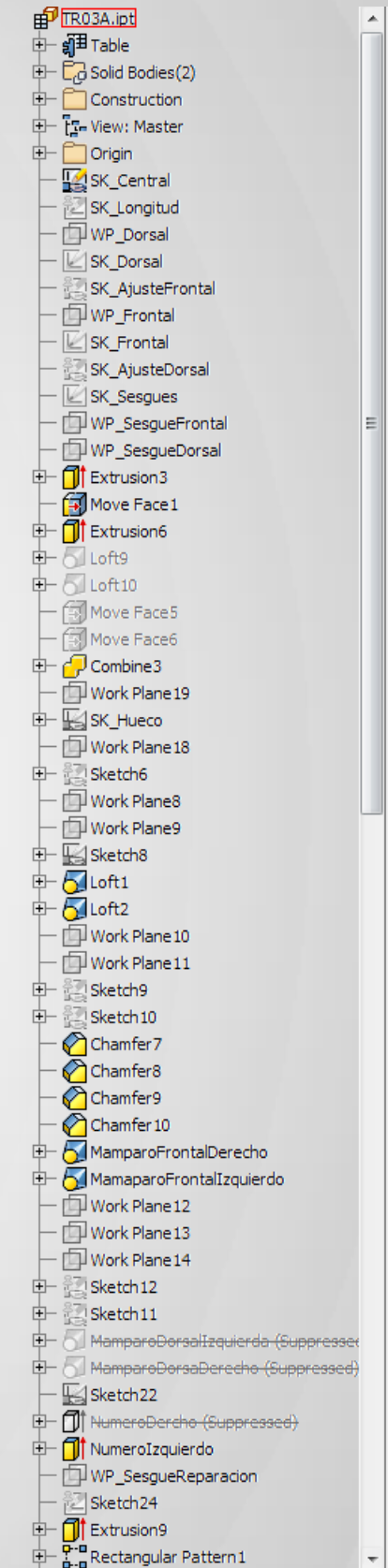
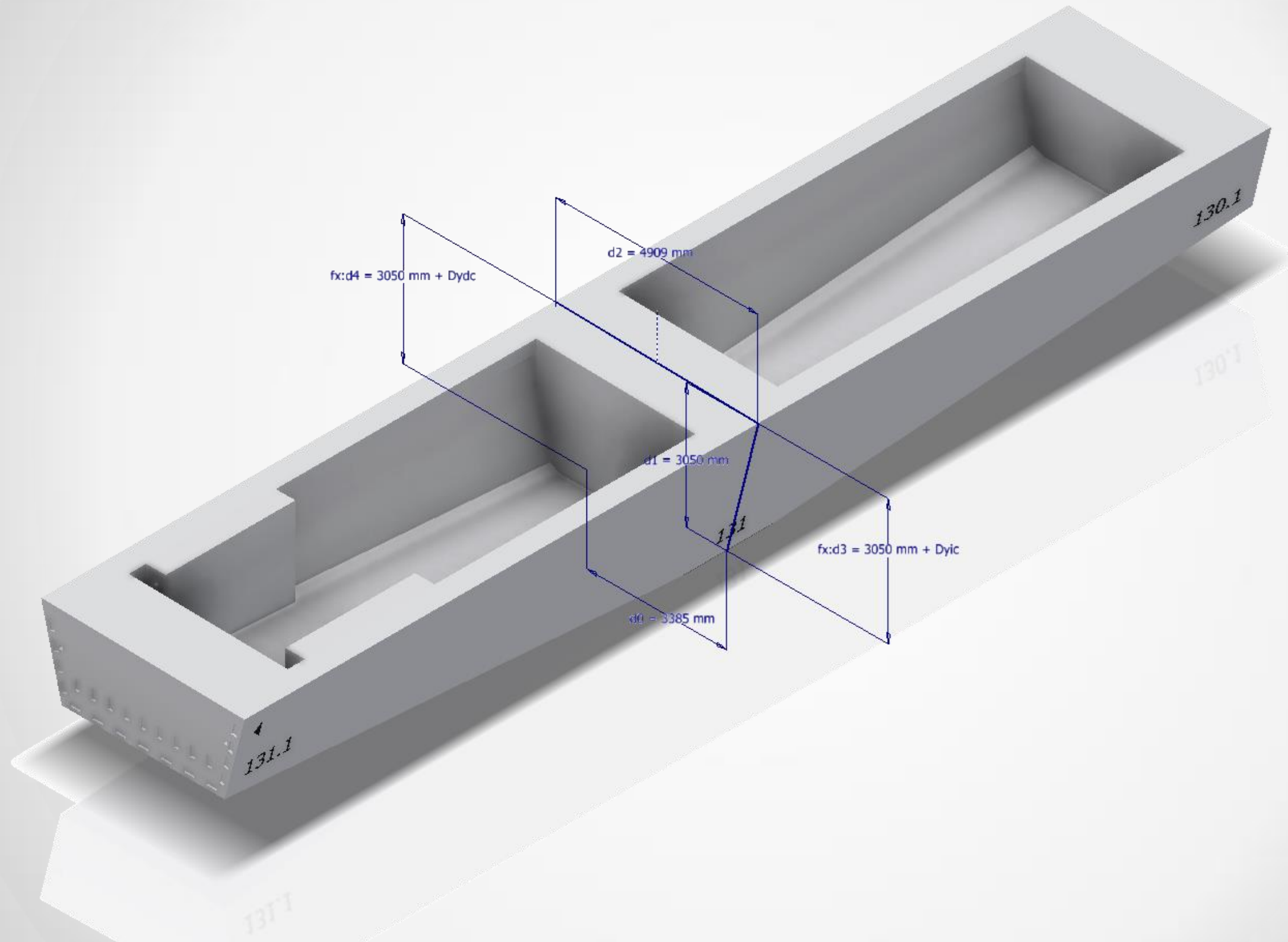


Inventor Parts and Assemblies

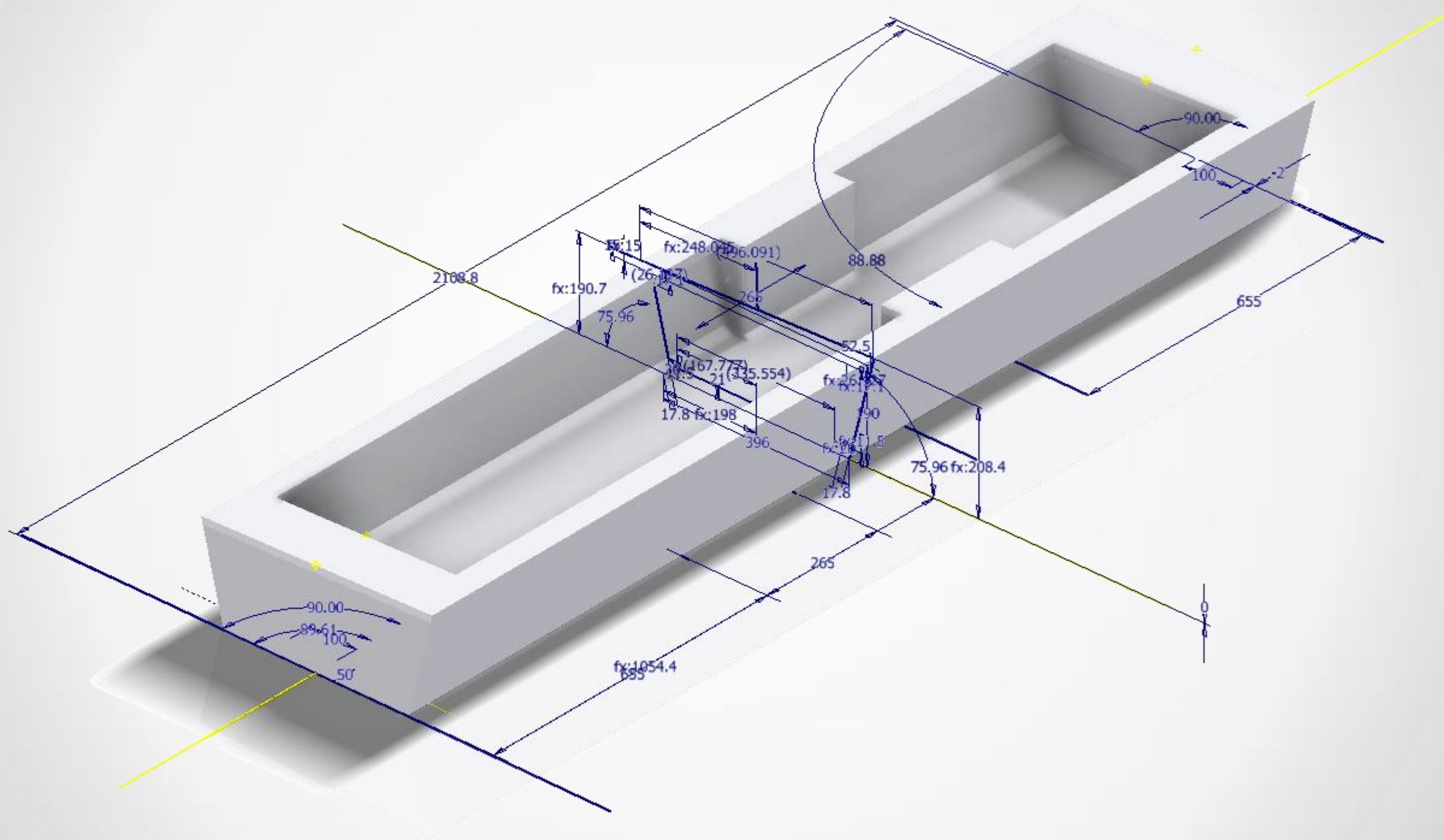




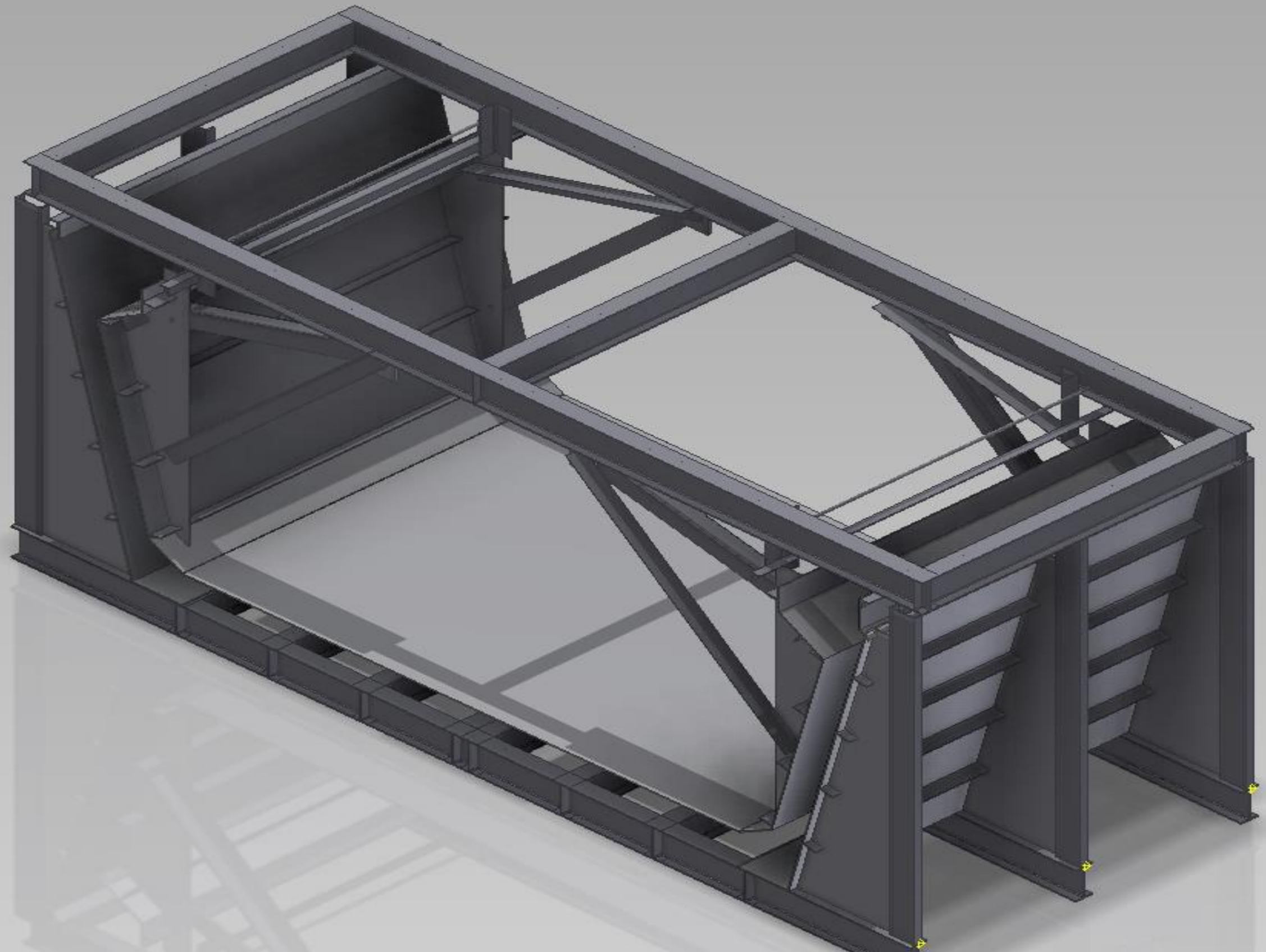
Girder

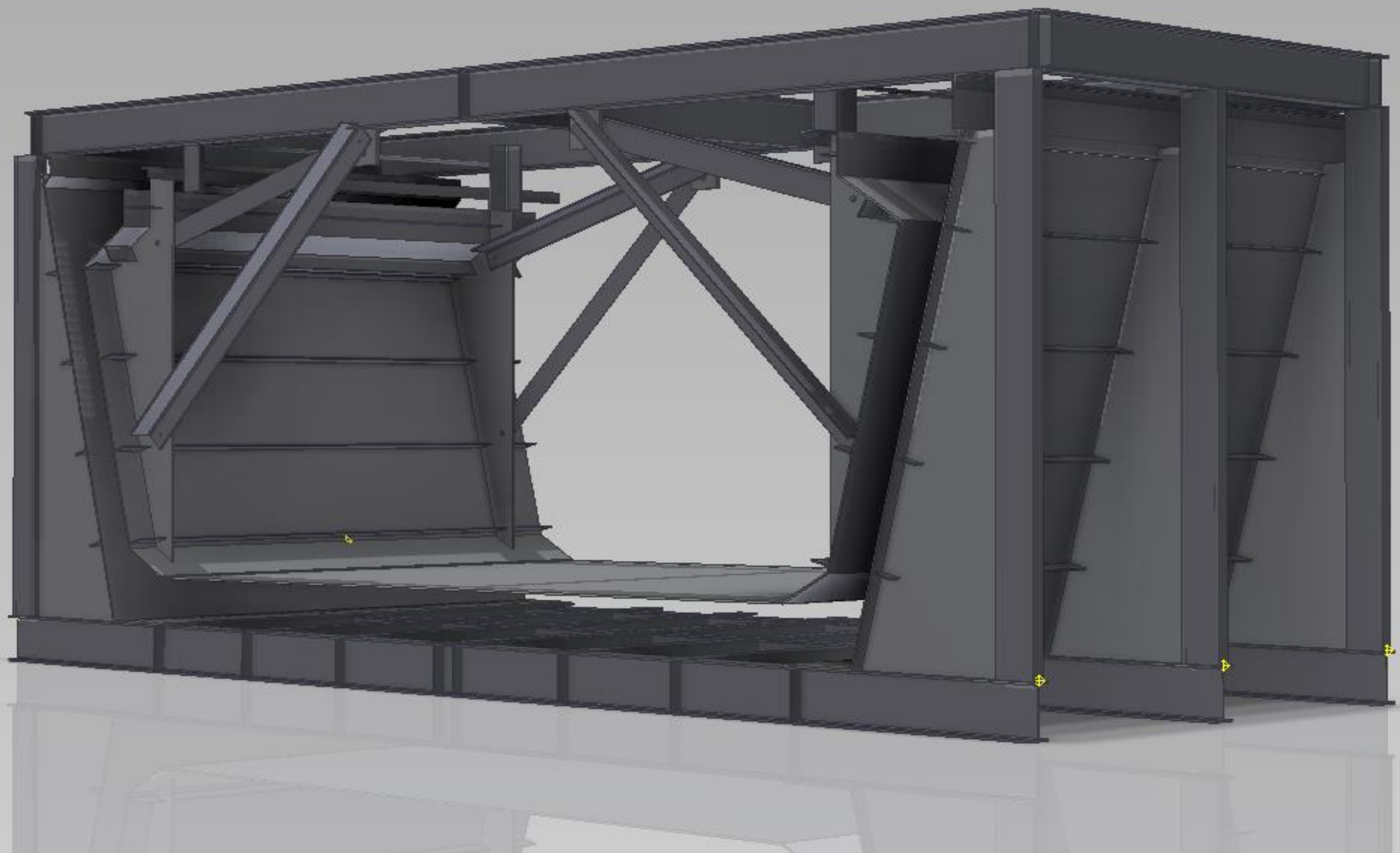


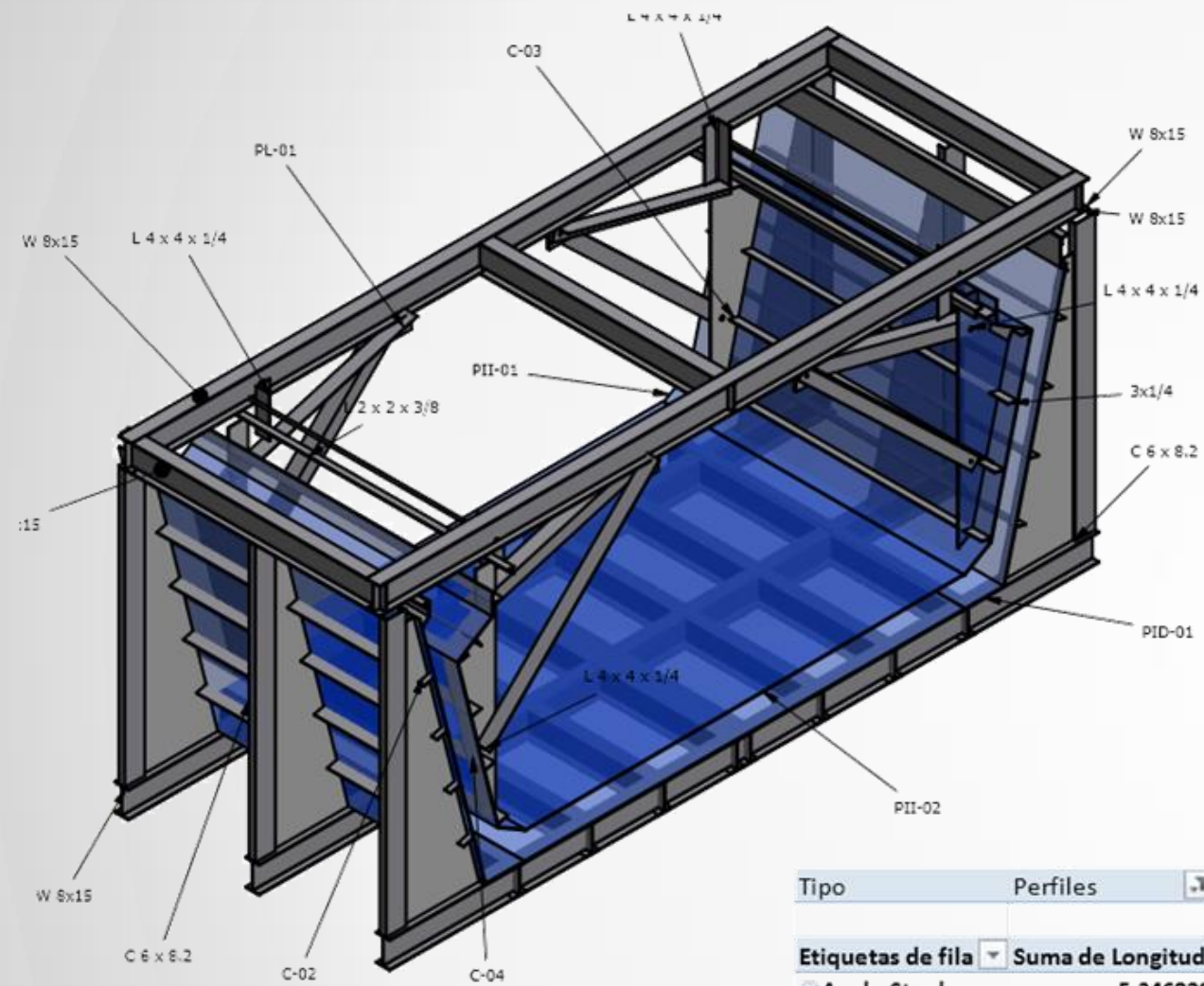
Girder



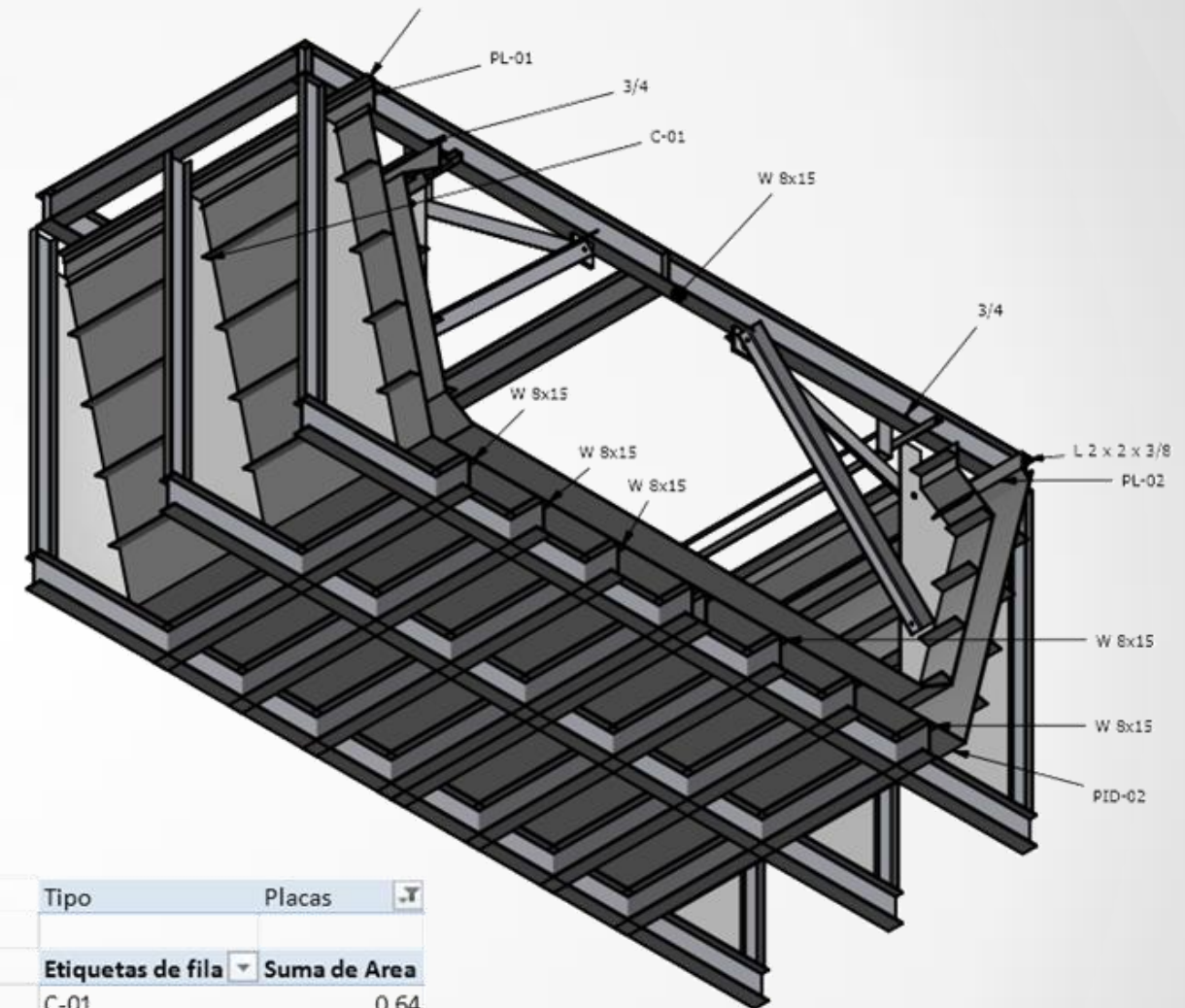
Formwork Inventor Assemblies



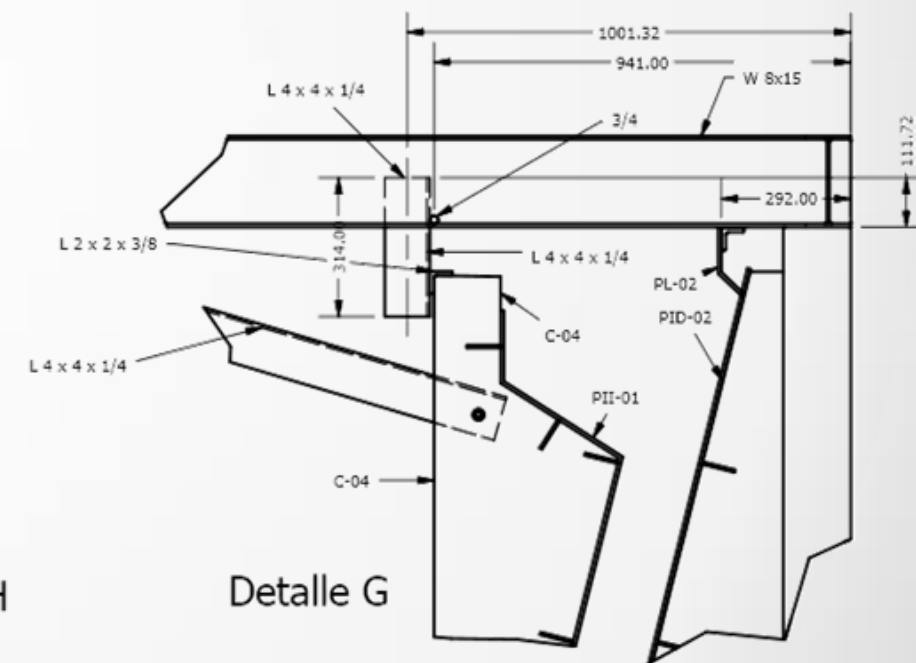
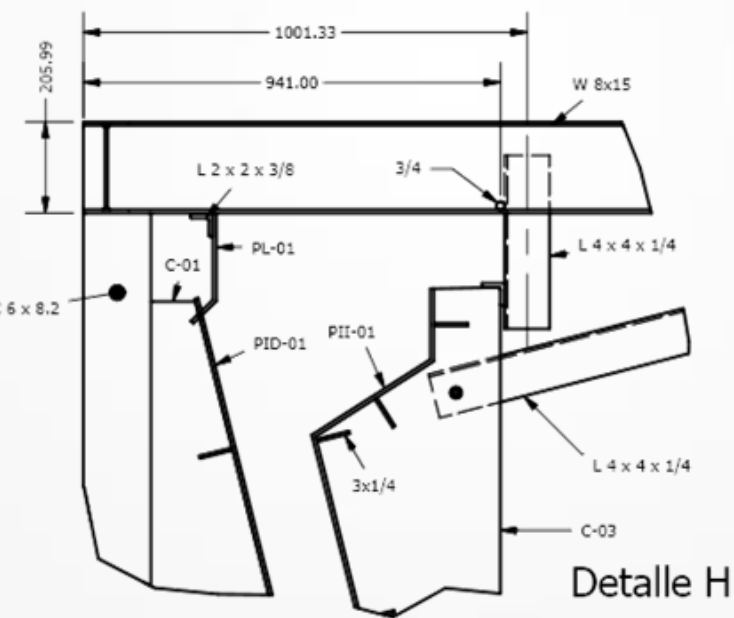
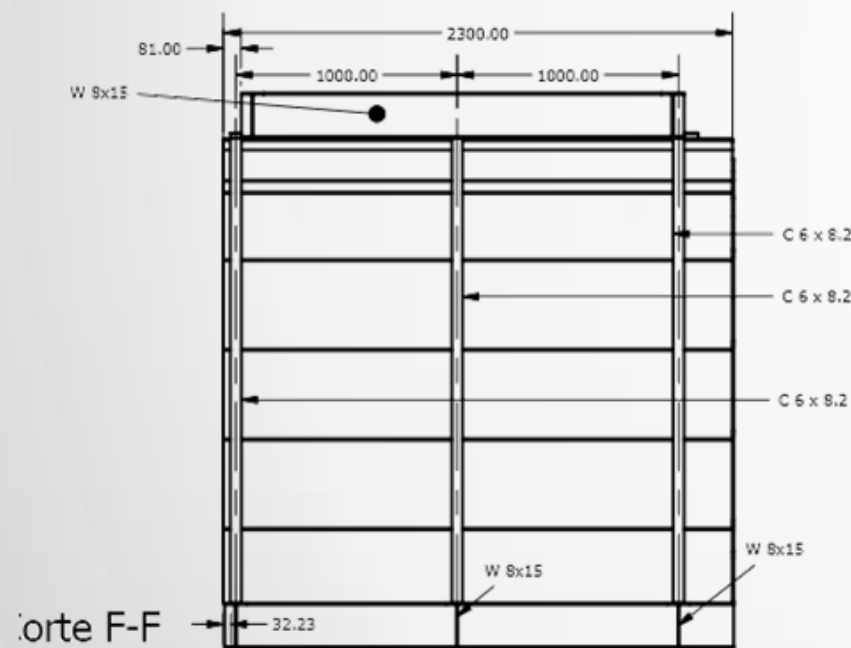
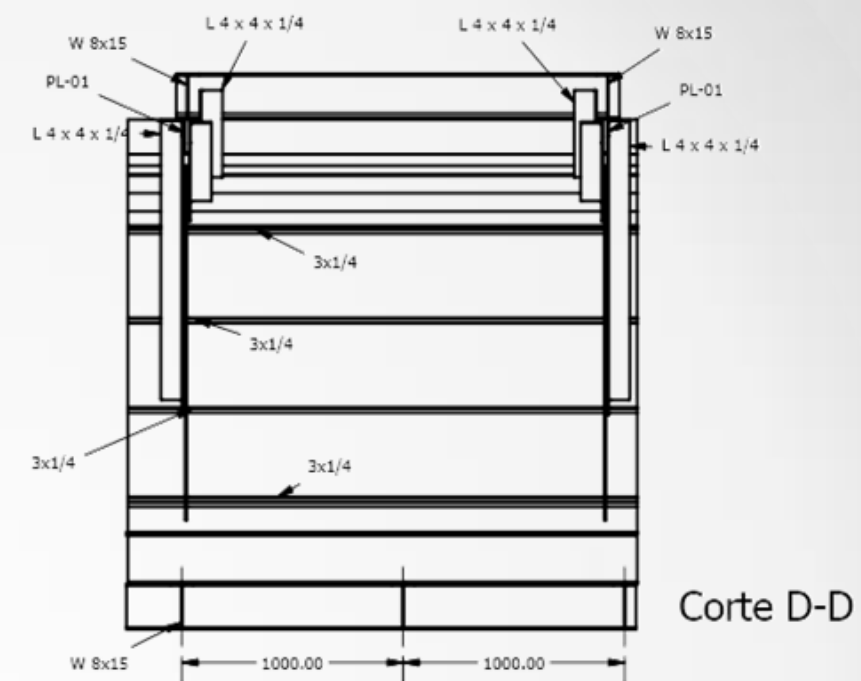
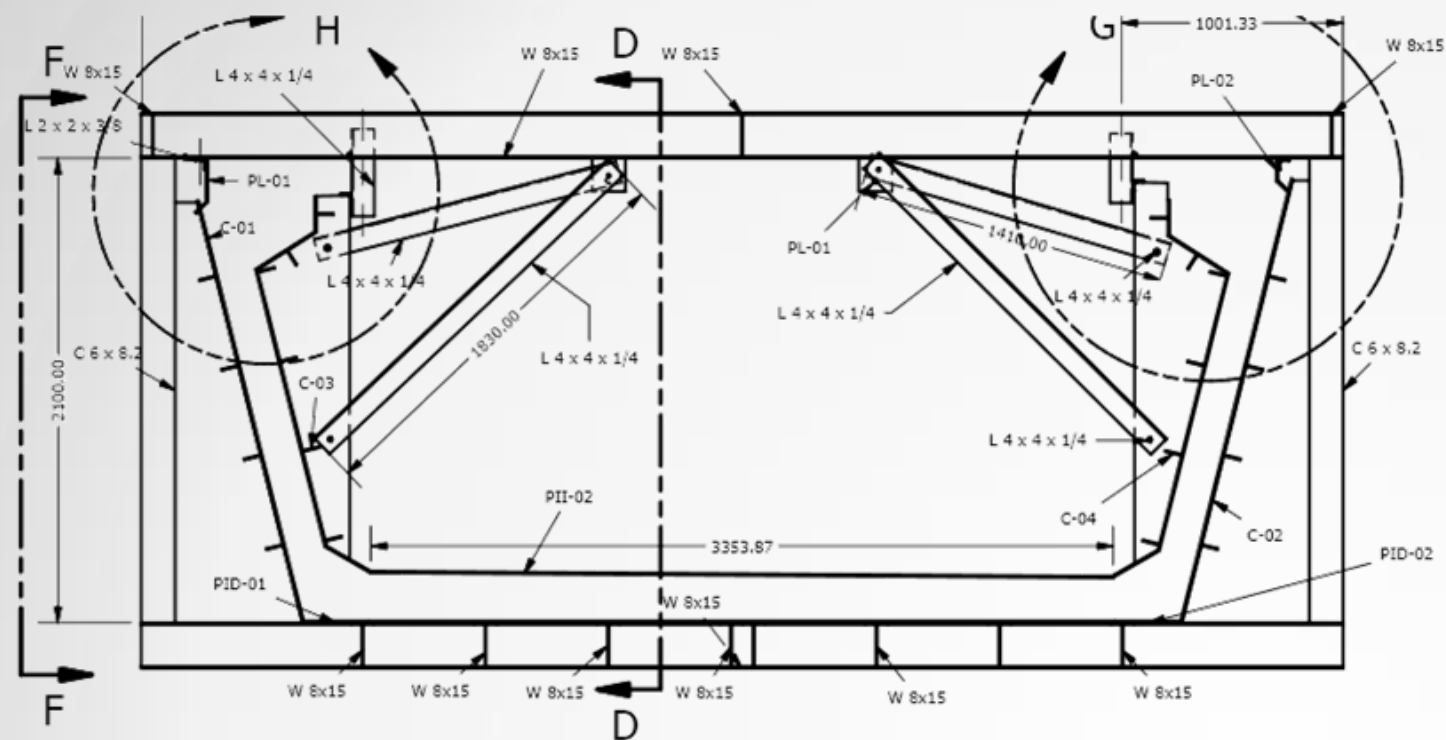




Tipo	Perfiles	
	Etiquetas de fila	Suma de Longitud
Angle Steel		5.346928
L 2 x 2 x 3/8		4.781102
L 4 x 4 x 1/4		0.565826
Flat Bar Steel		1.811024
3x1/4		1.811024
Round Bars		3.904242
3/4		3.904242
U-Shape		12.6
C 6 x 8.2		12.6
W Shape		33.135
W 8x15		33.135
Total general		56.797194



Tipo	Placas	
	Etiquetas de fila	Suma de Area
C-01		0.64
C-02		0.65
C-03		0.4
C-04		0.41
PID-01		5.2
PID-02		5.44
PII-01		4.61
PII-02		7.72
PL-01		0.68
PL-02		0.4
Total general		26.15

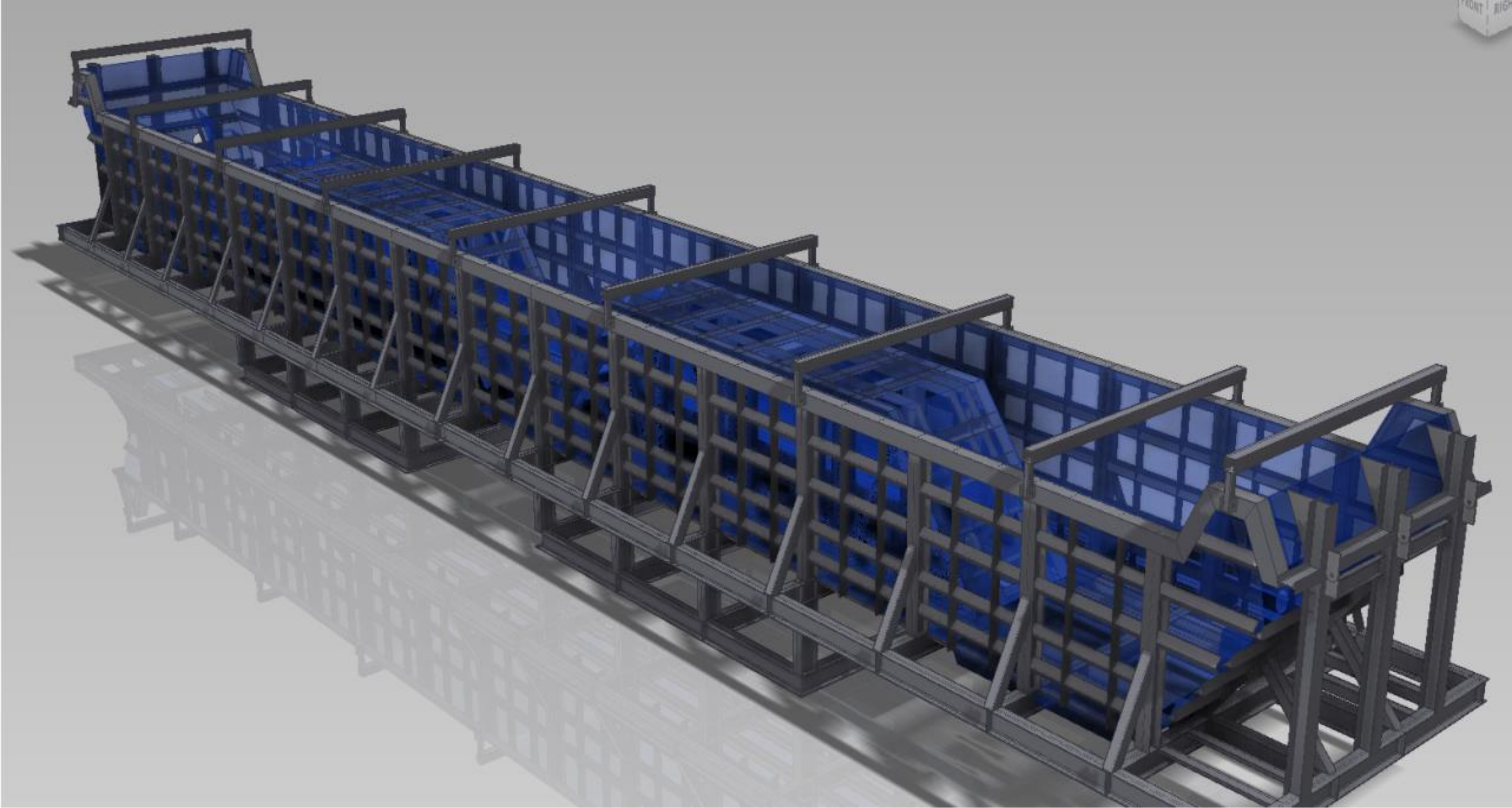


Proyecto:
Río de los Remedios
No de Pieza
Molde Trabe TC

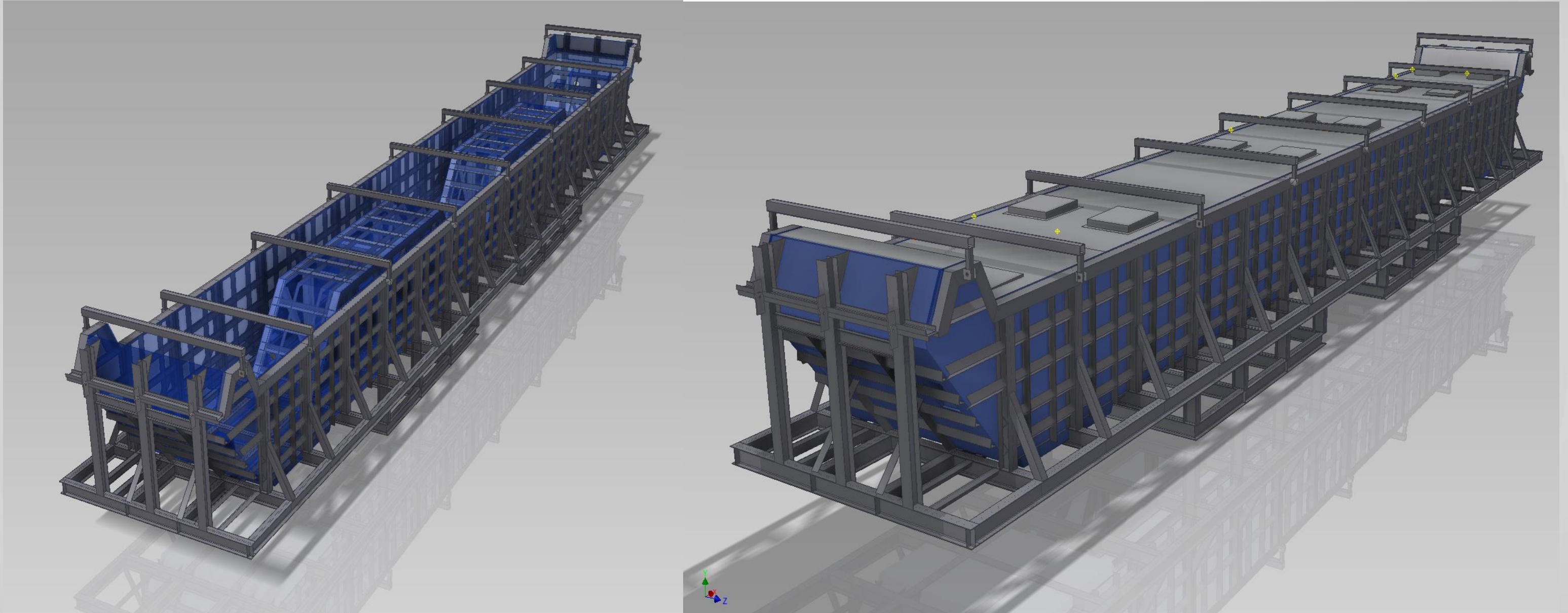
Reporte:
Detalles
Descripción:

Notas:
ICA BIM no se hace responsable del diseño, esto corre a cuenta del Cliente

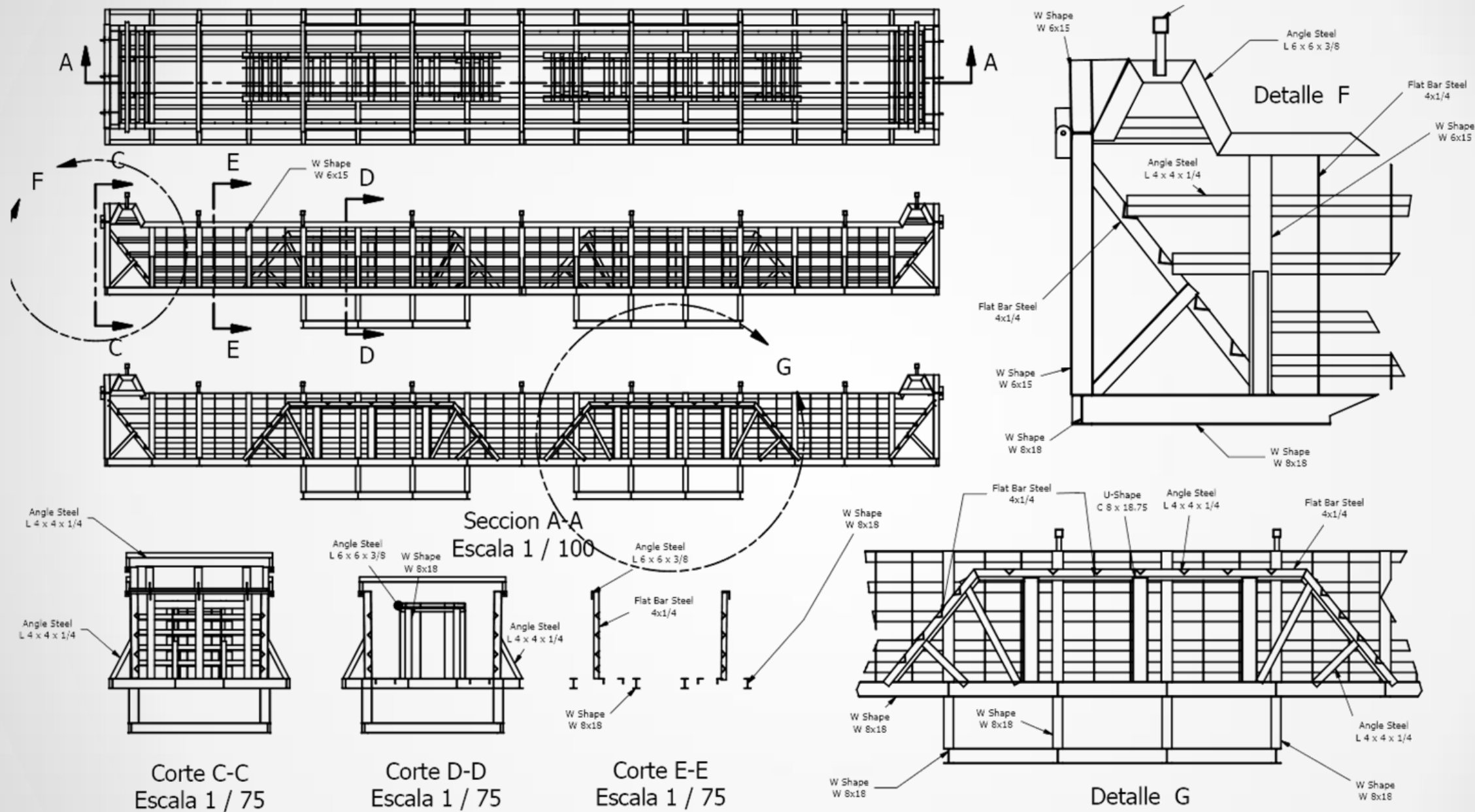
No de Pls
3 d
Fecha de entri
27/05/21
Esc



Column Cap Formwork



Formwork Drawings



Proyecto:
Rio de los Remedios

No de Pieza

Reporte:
Cortes - Molde

Descripción:

Notas:

ICA BIM no se hace responsable del diseño, esto corre a cuenta del Cliente

No de Plar
2 de
Fecha de entreg

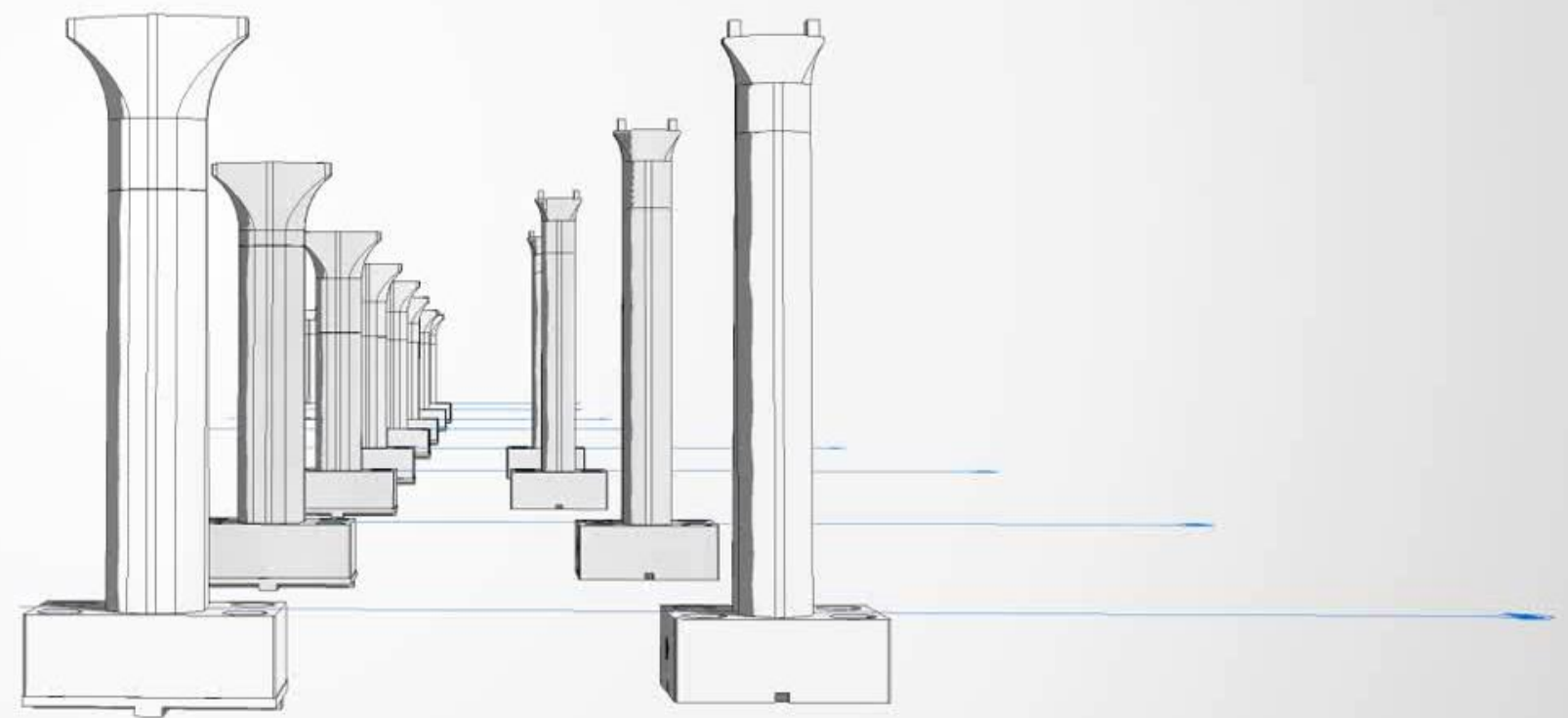


Assembly Models in Revit

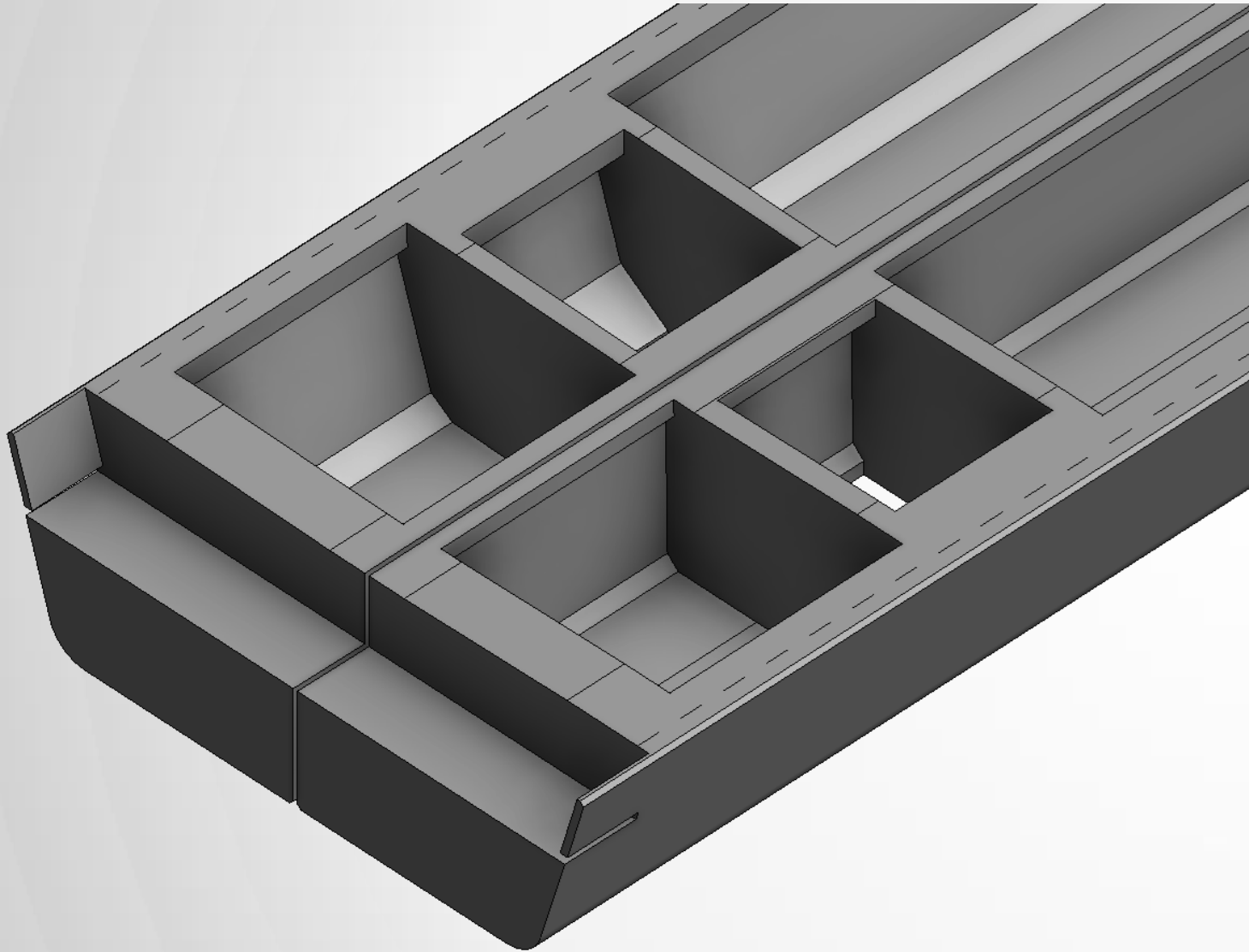


An aerial photograph of a city area with a grid-like street pattern. A thick orange line traces a route through the city, starting from the top left, curving around a large dark area (possibly a park or water body), and then extending towards the bottom right. A black rectangular box with yellow text is positioned in the upper left quadrant of the map.

LENGTH
12.70 miles

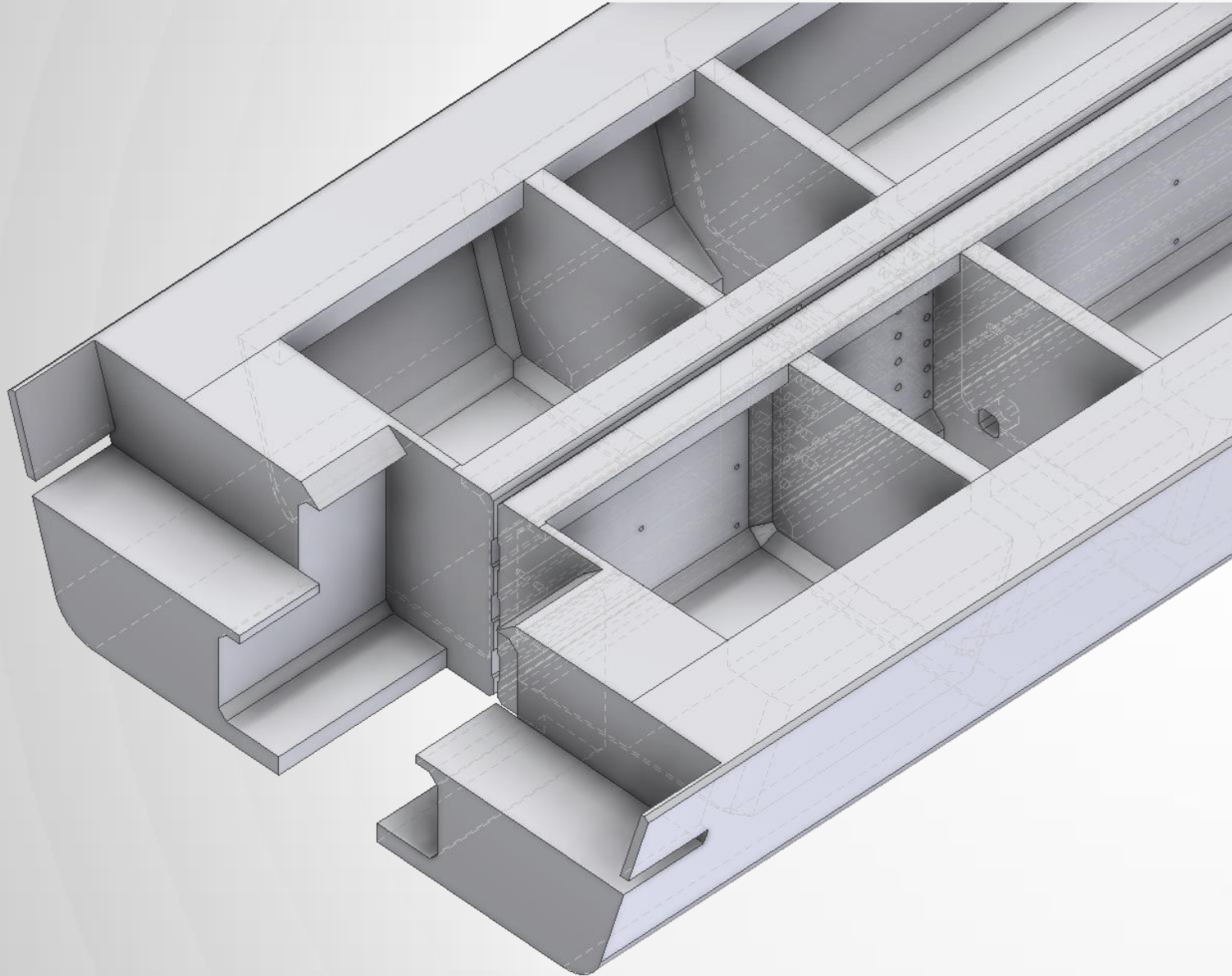


Assembly Model in Revit



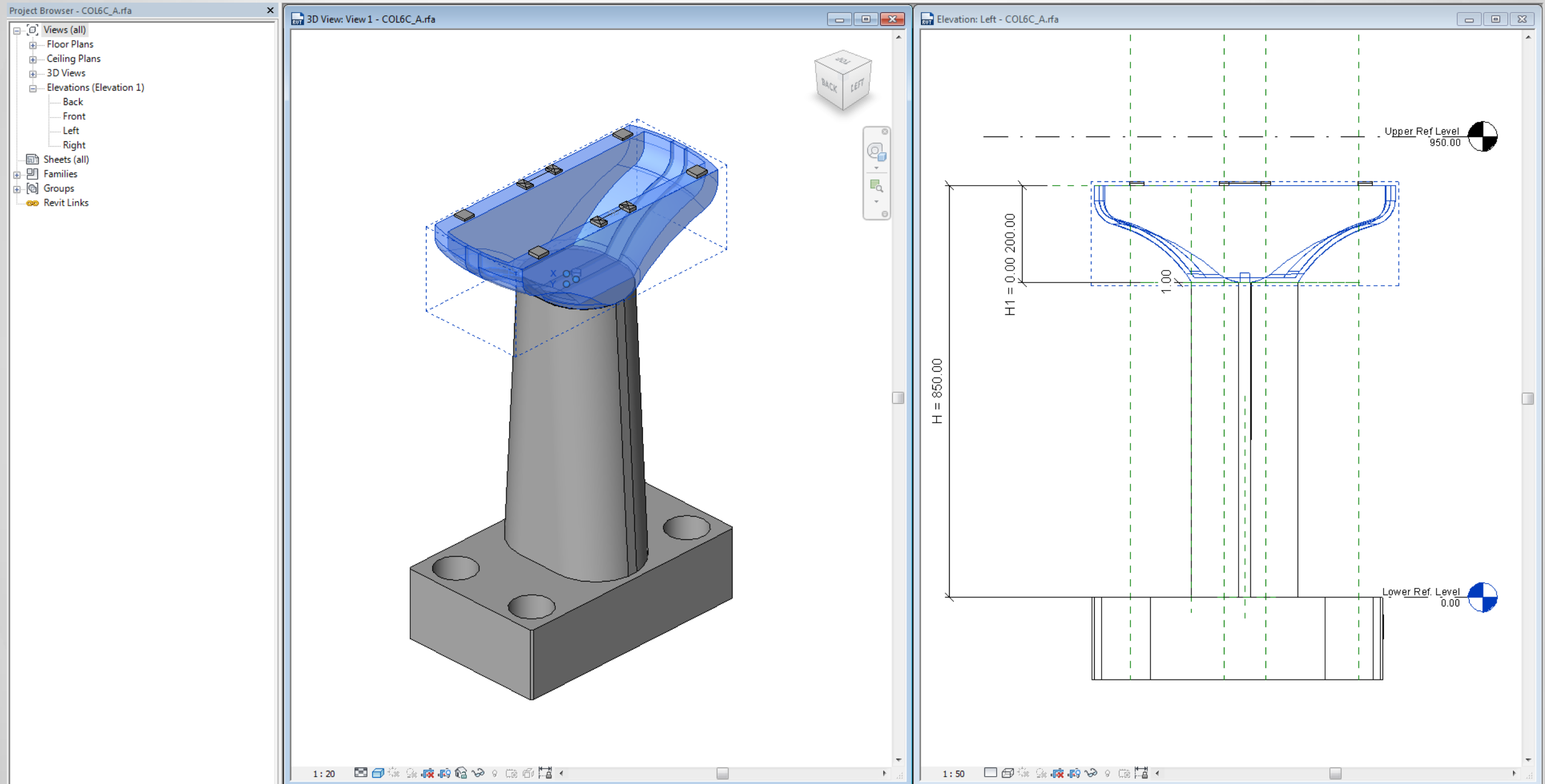
- Quantity Takeoff for elements made in-situ
- Incongruence, intersections and geometry relationships.
- Progress Report
- Simulation 4D.
- MEP Models.

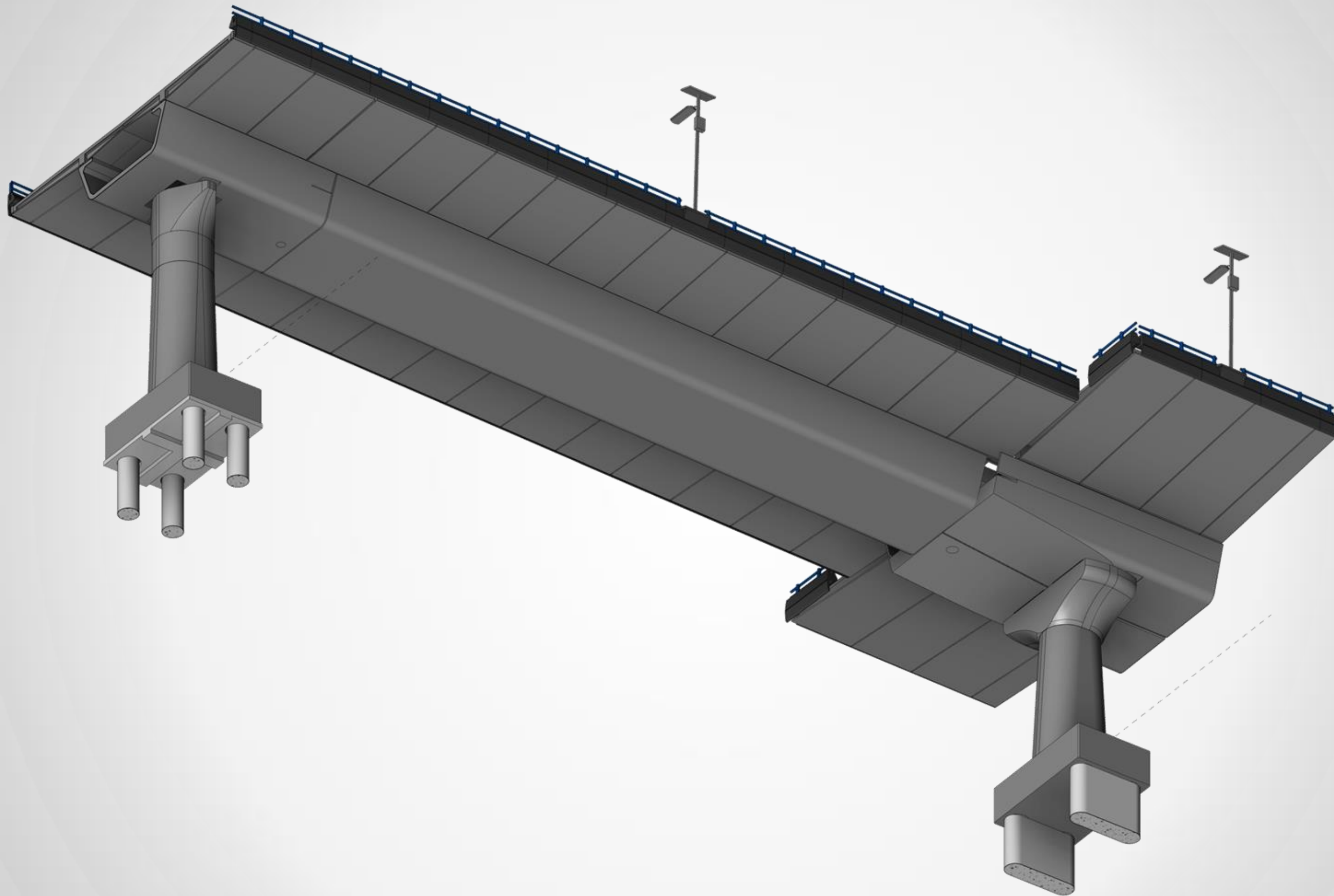
Geometry Model in Inventor and Rebar Model in Revit

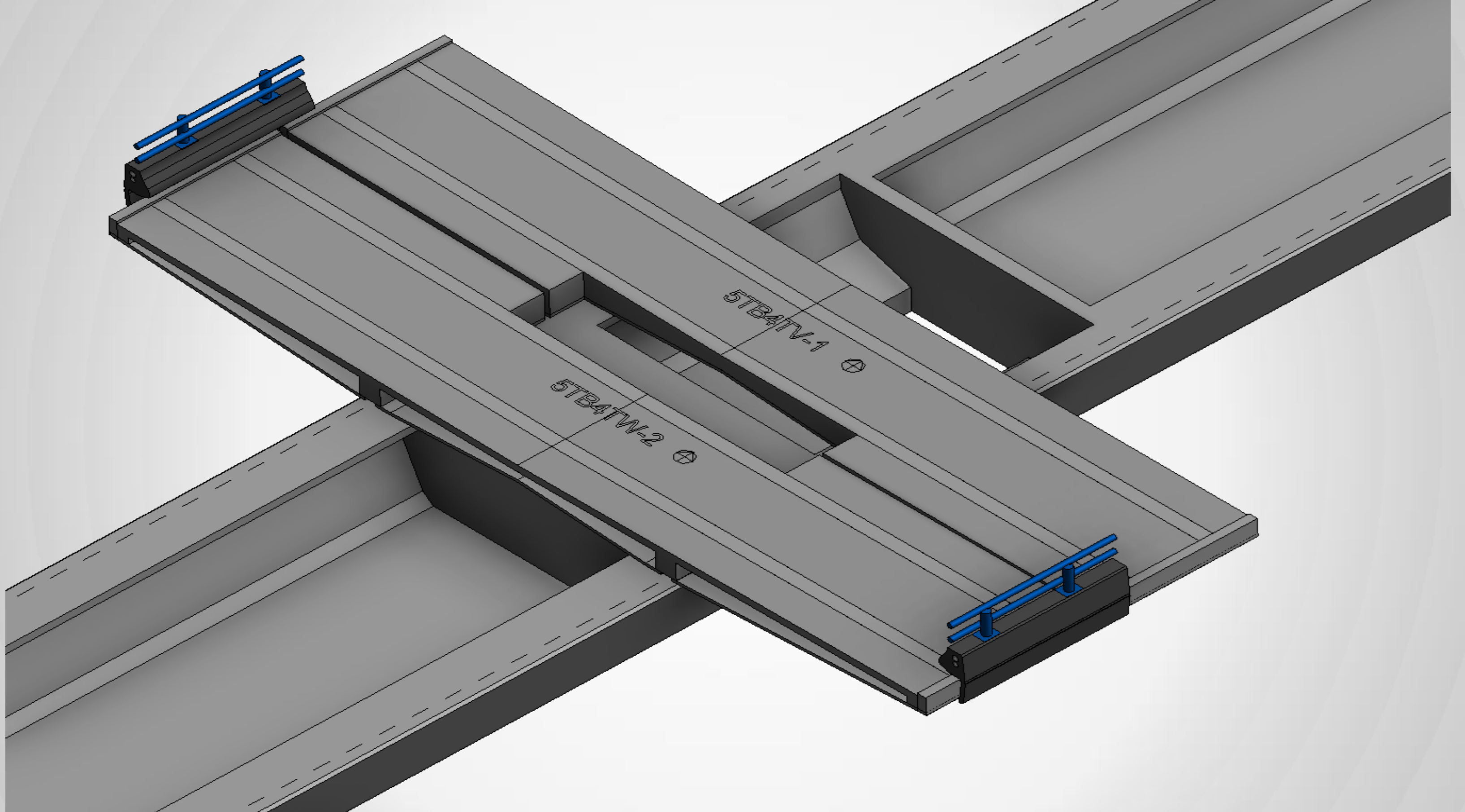


- Comparison among many elements for Formwork Optimization
- Rebar and concrete Quantity Takeoff
- Clash detection
- Shop drawings – Rebar and Tendons

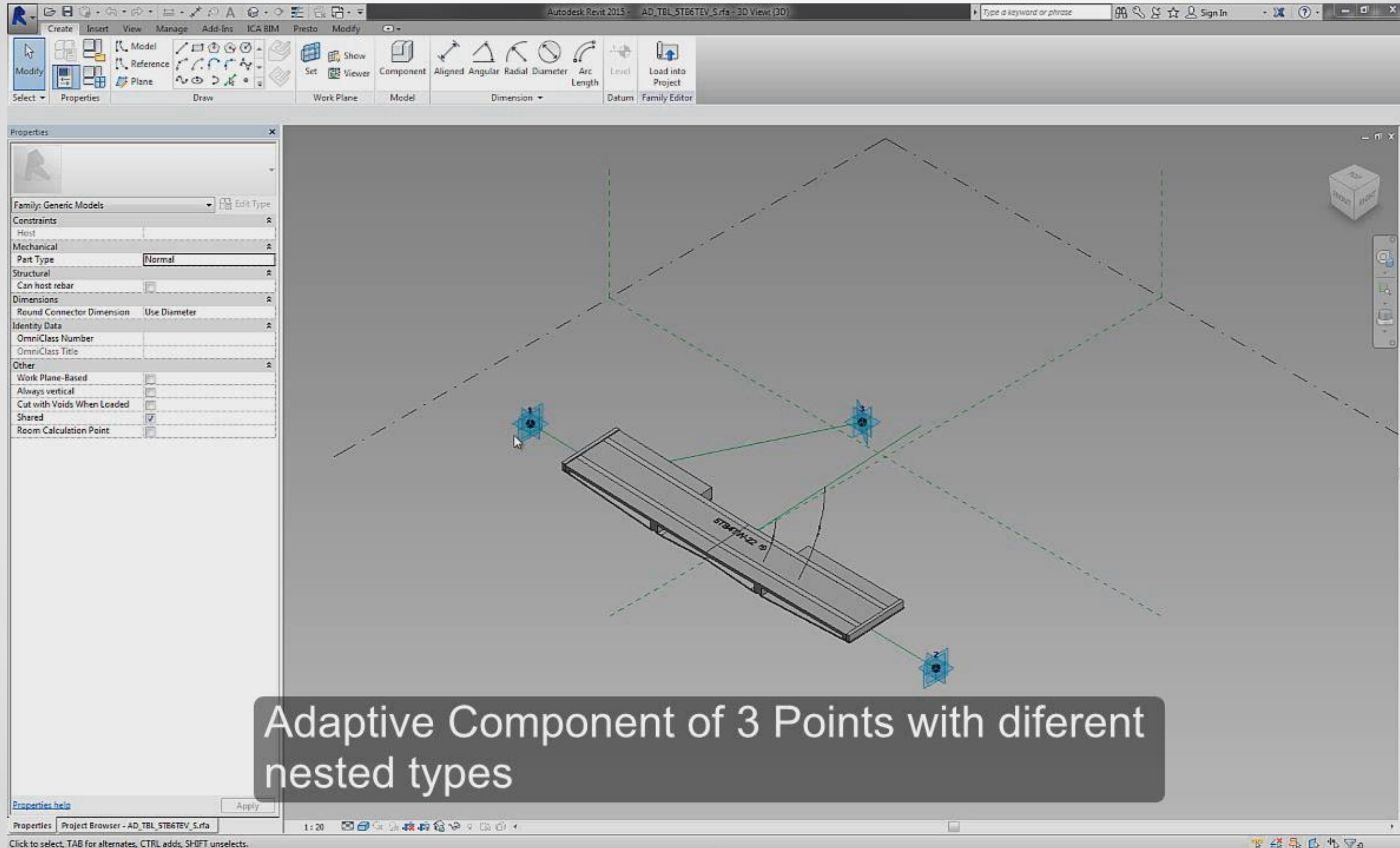
Assembly Family Column



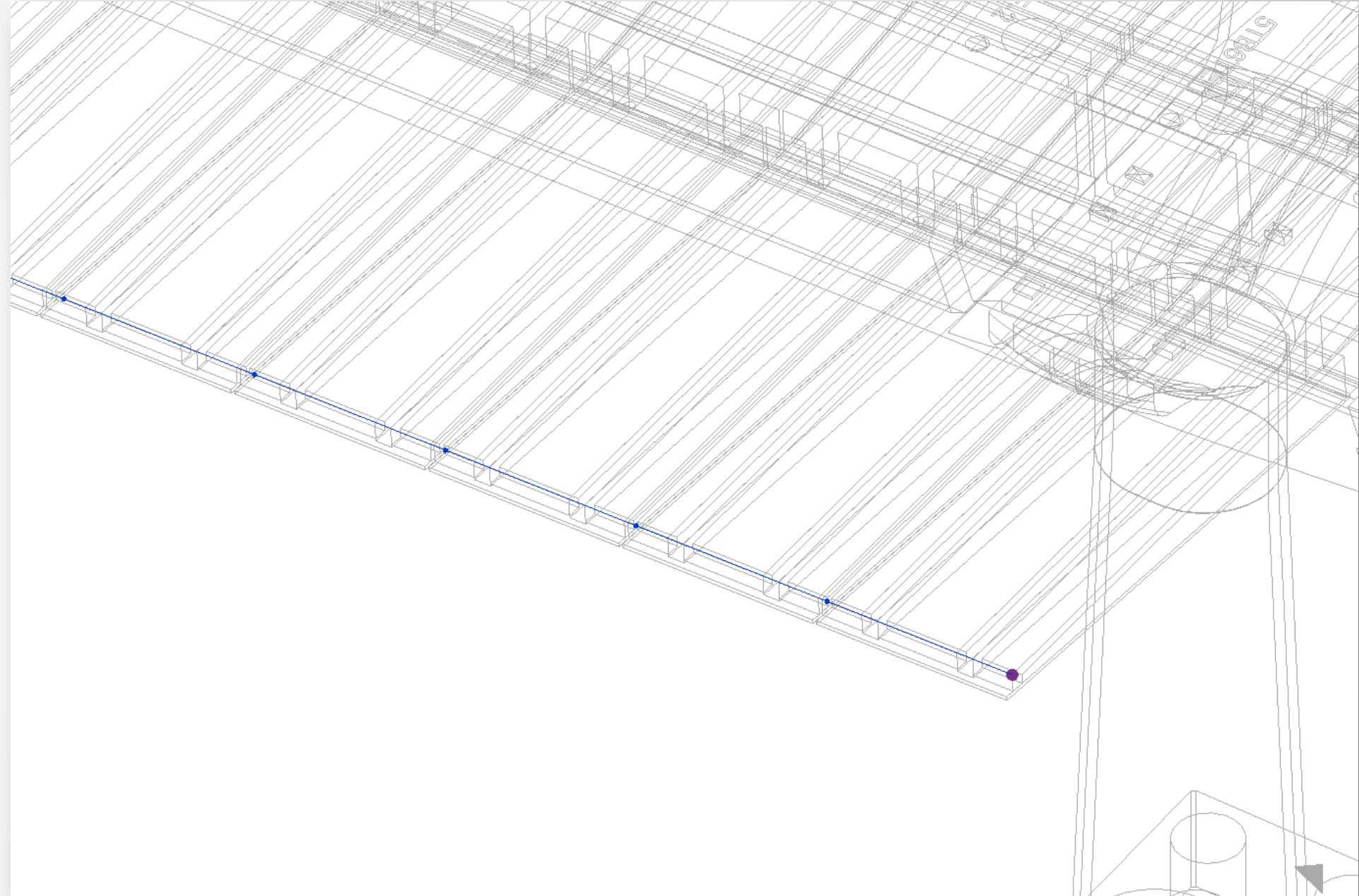
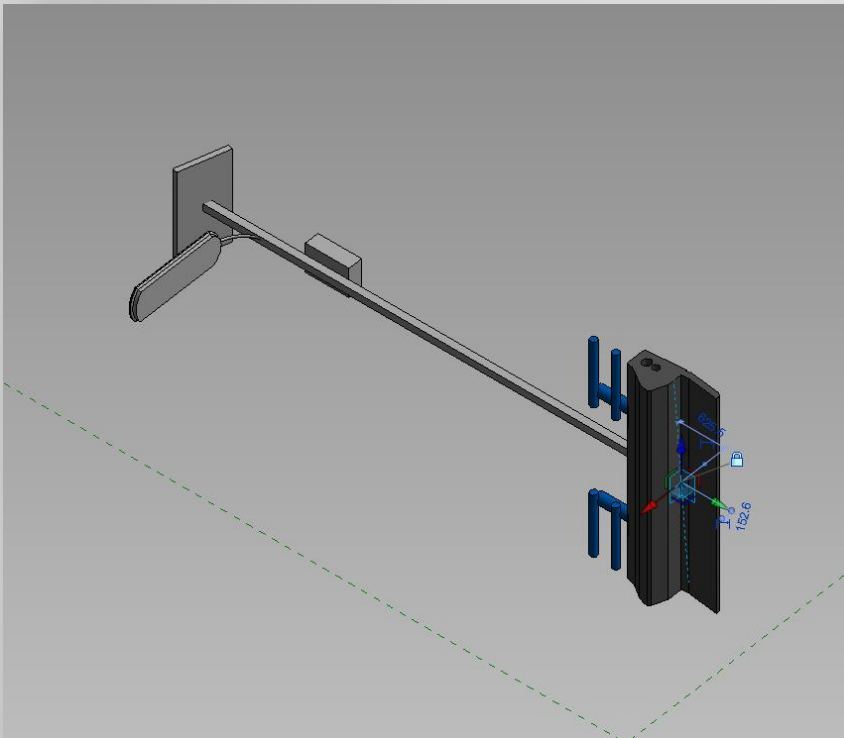
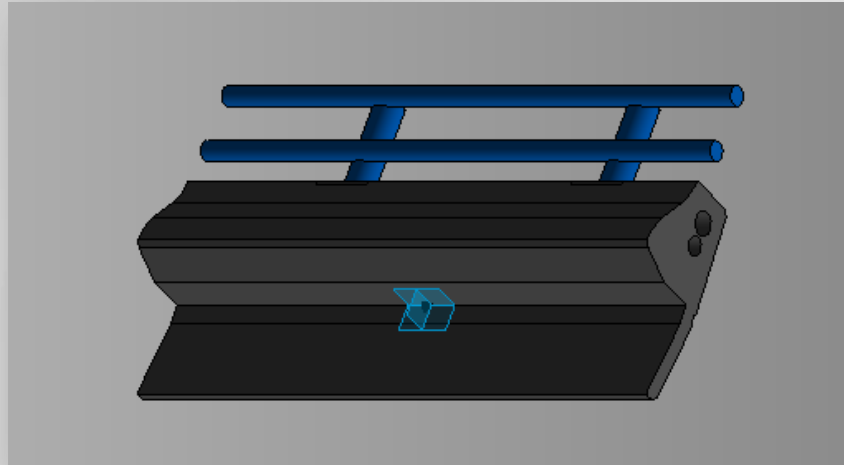




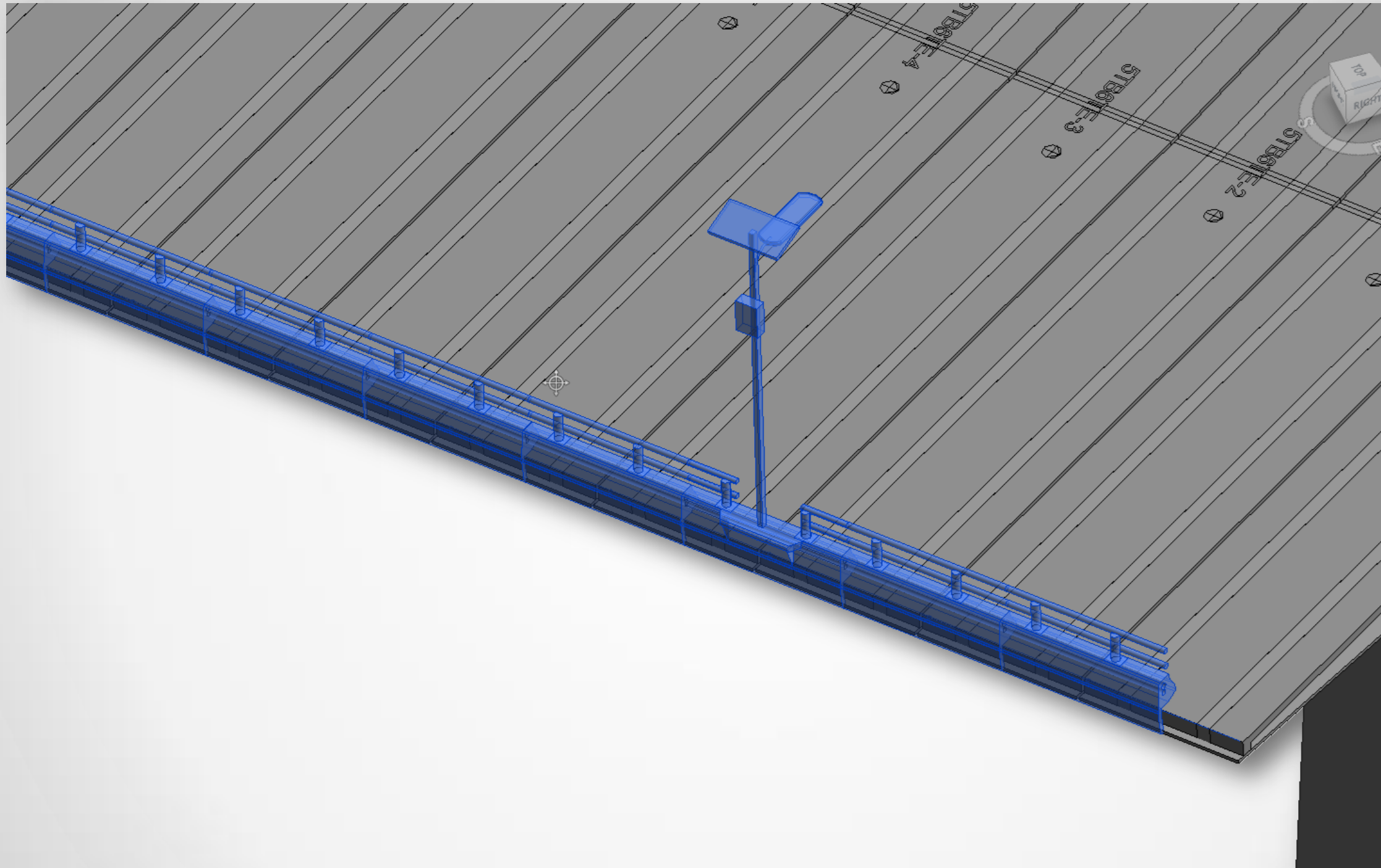
Link Prefab Slabs to Excel



Railing Families and a Division Path

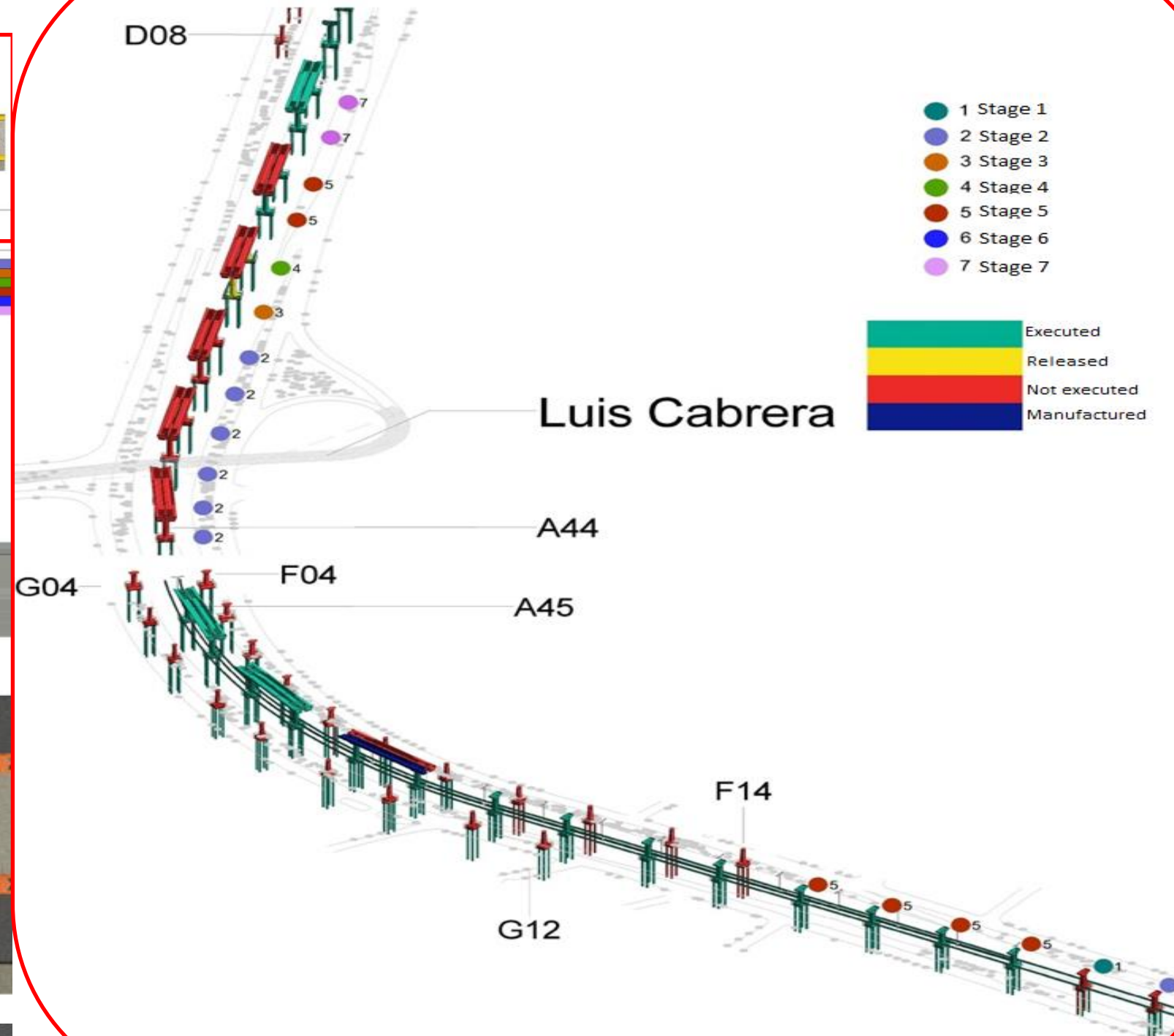
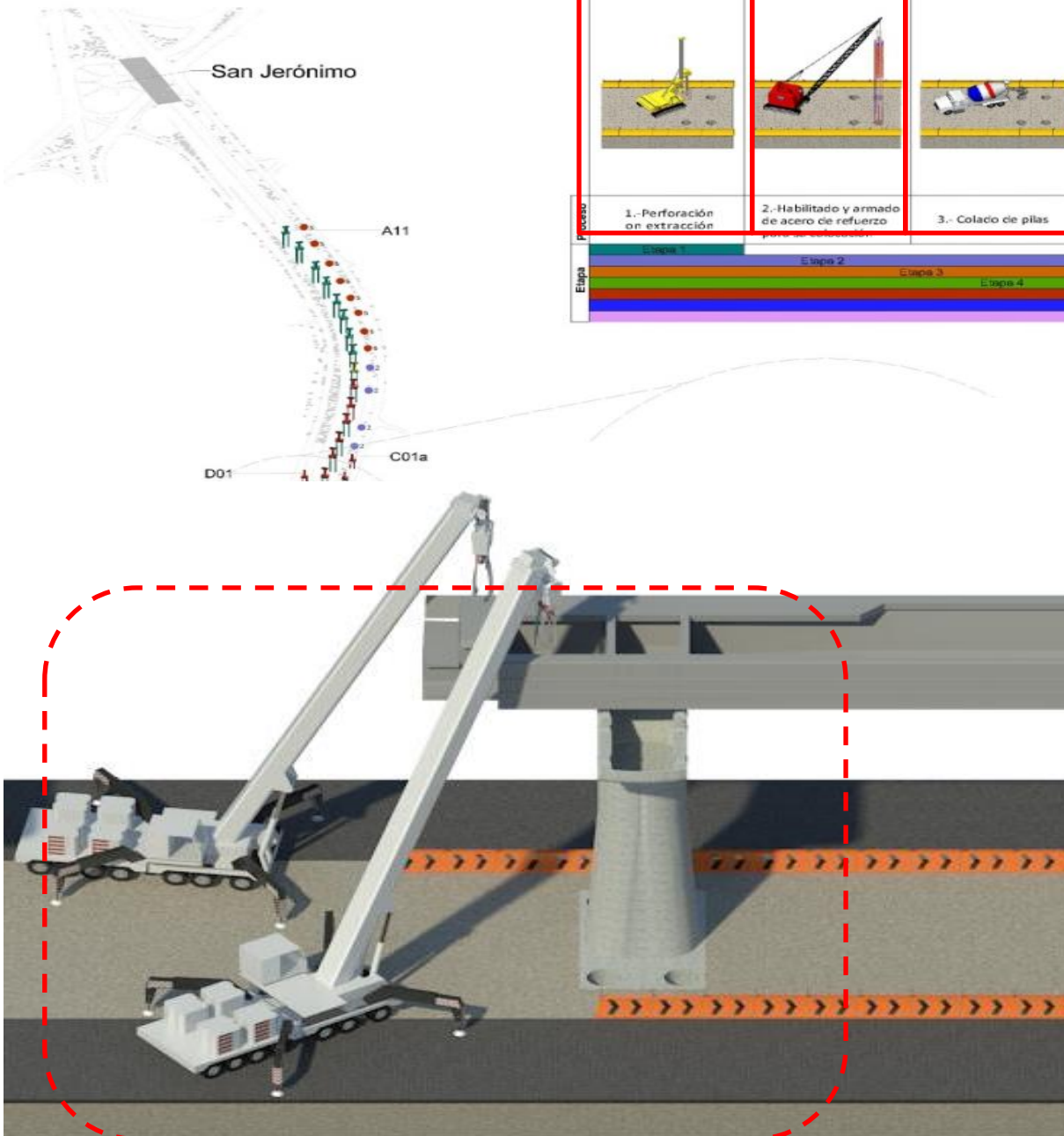
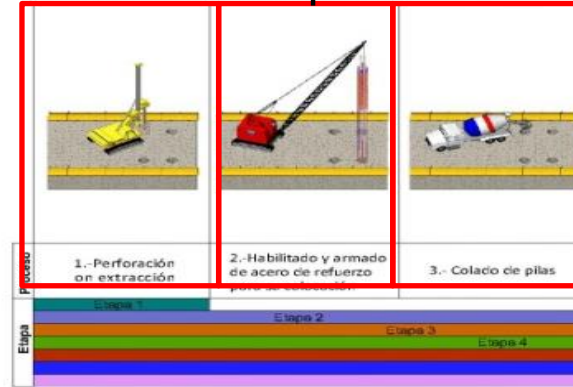


Railing Families and a Division Path



F1_T1-T3_San Jerónimo-Luis Cabrera-Camino Sta. Teresa-Picacho Ajusco

construction process



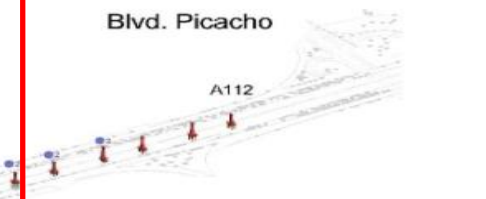
Proyecto:
ARCO SUR
No. de Proyecto:
10_009
Dirección:
Periferico Sur, San Jerónimo-Muyuguarda-Caseta a Cuernavaca.

Descripción:
F1_T1-T3_San Jerónimo-Luis Cabrera-Camino Sta. Teresa-Picacho Ajusco

Simbología:

- 1 Etapa 1
- 2 Etapa 2
- 3 Etapa 3
- 4 Etapa 4
- 5 Etapa 5
- 6 Etapa 6
- 7 Etapa 7

Ejecutado
Liberado
No ejecutado
Fabricado



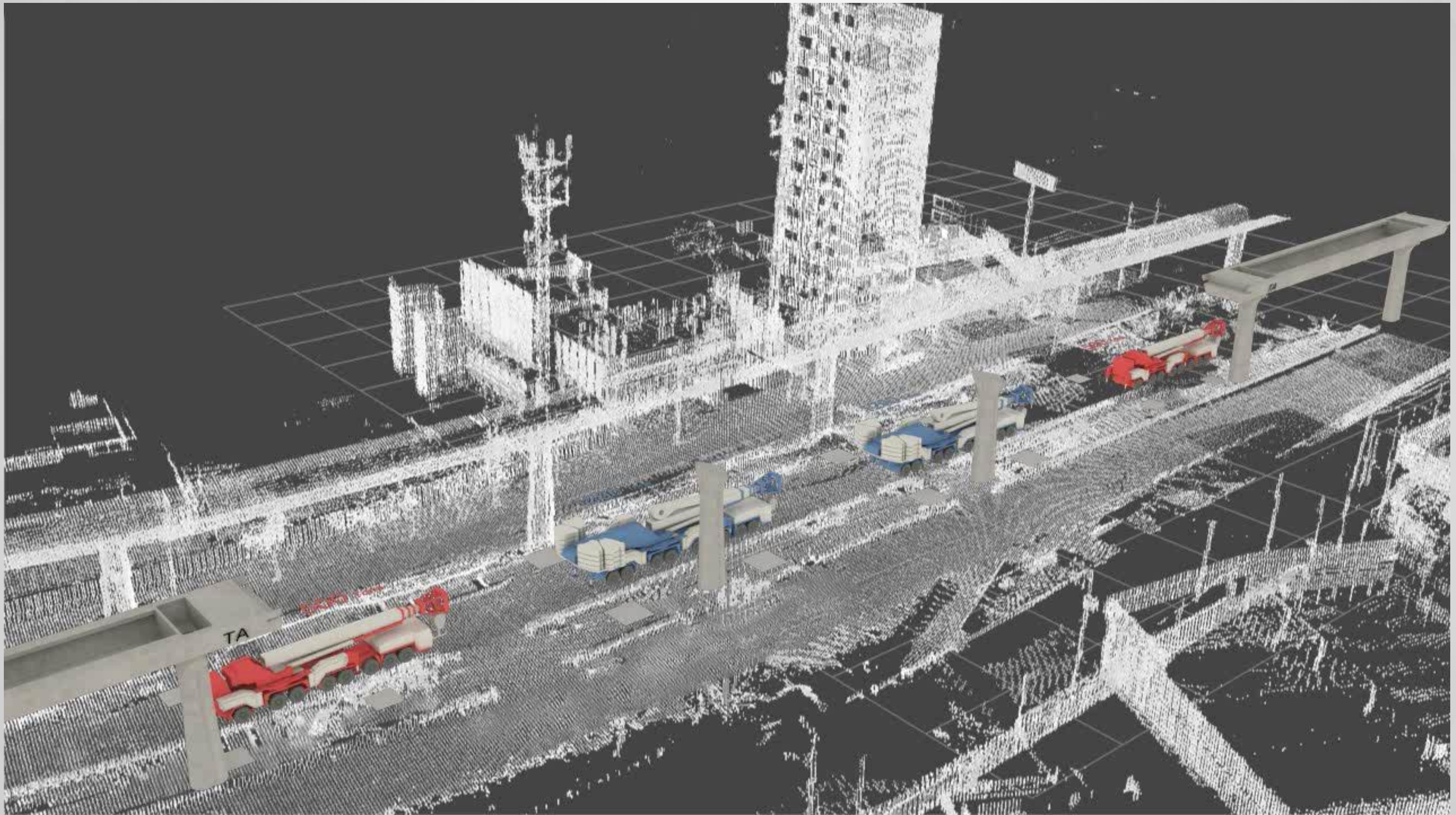
San Jerónimo

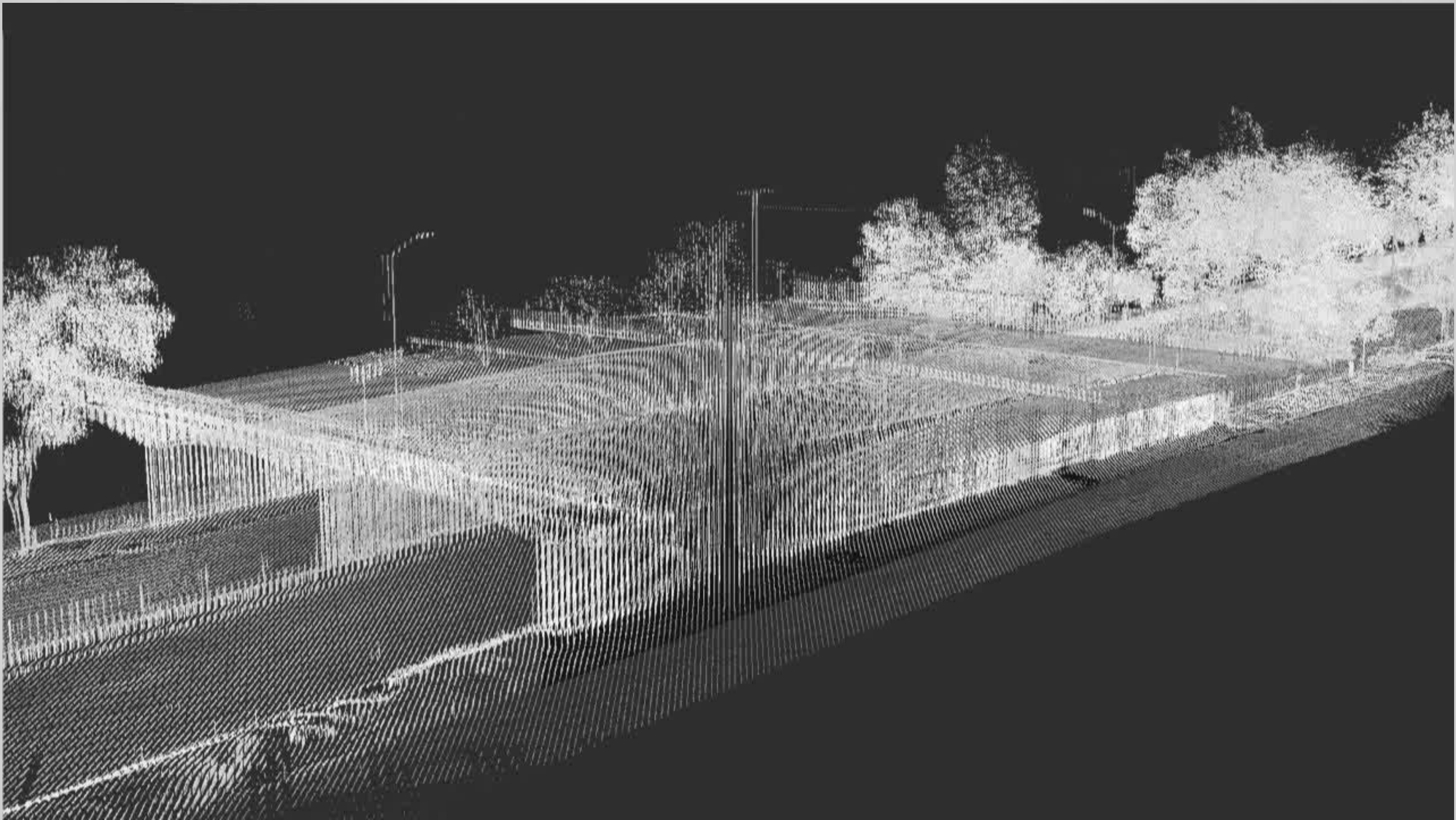
Secuencia de Montaje



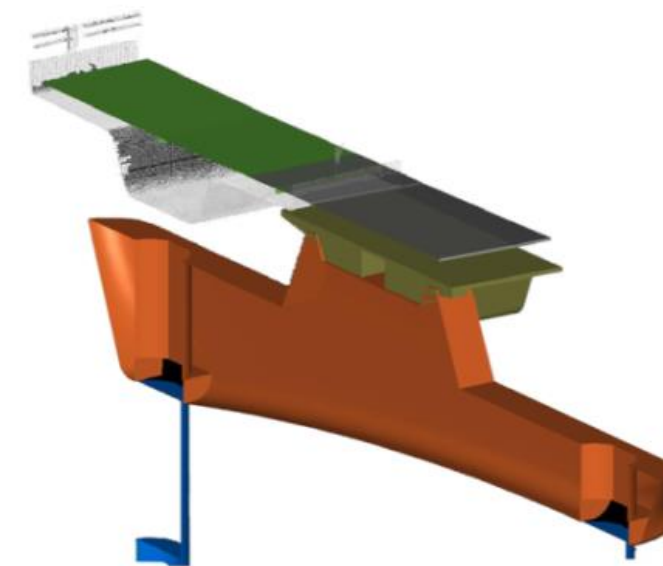
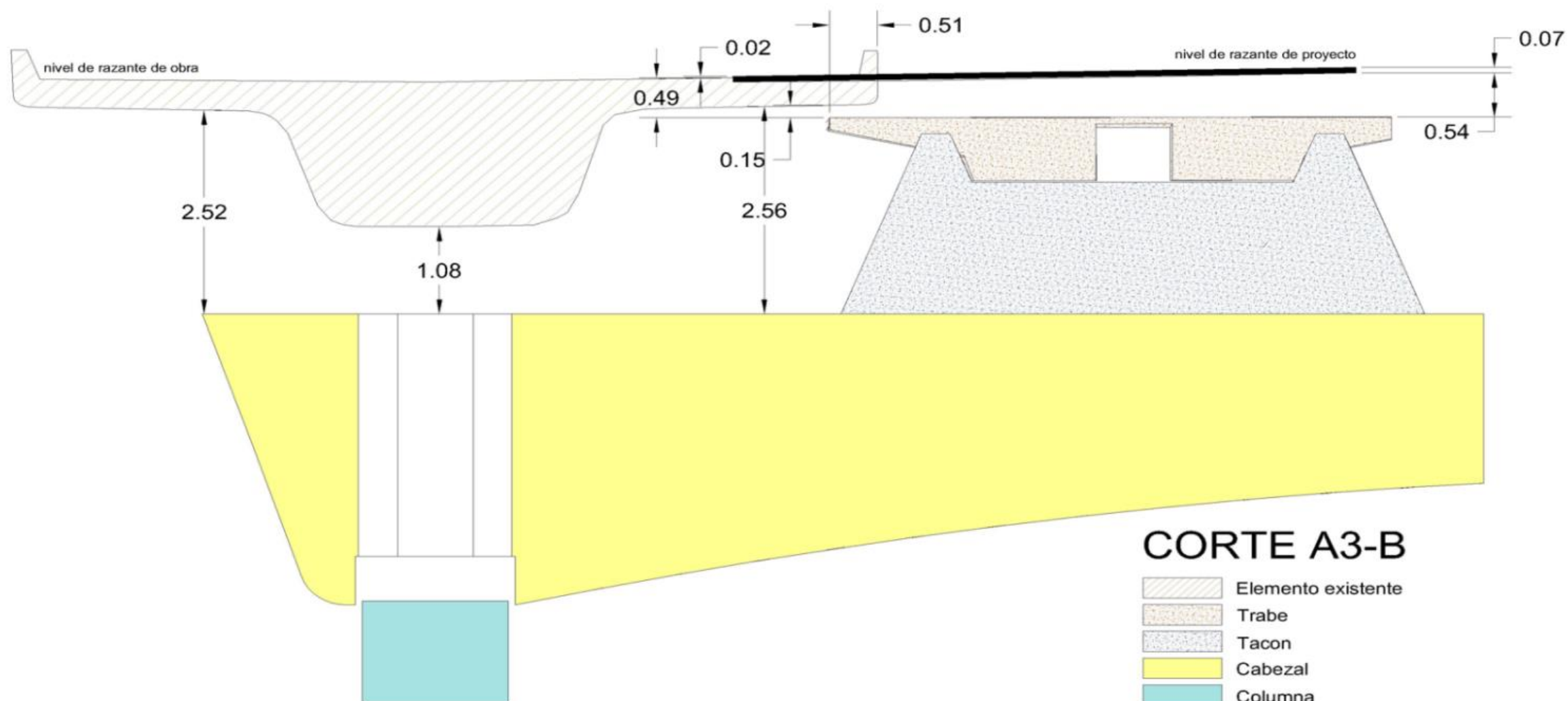
VIADUCTO TLALPAN



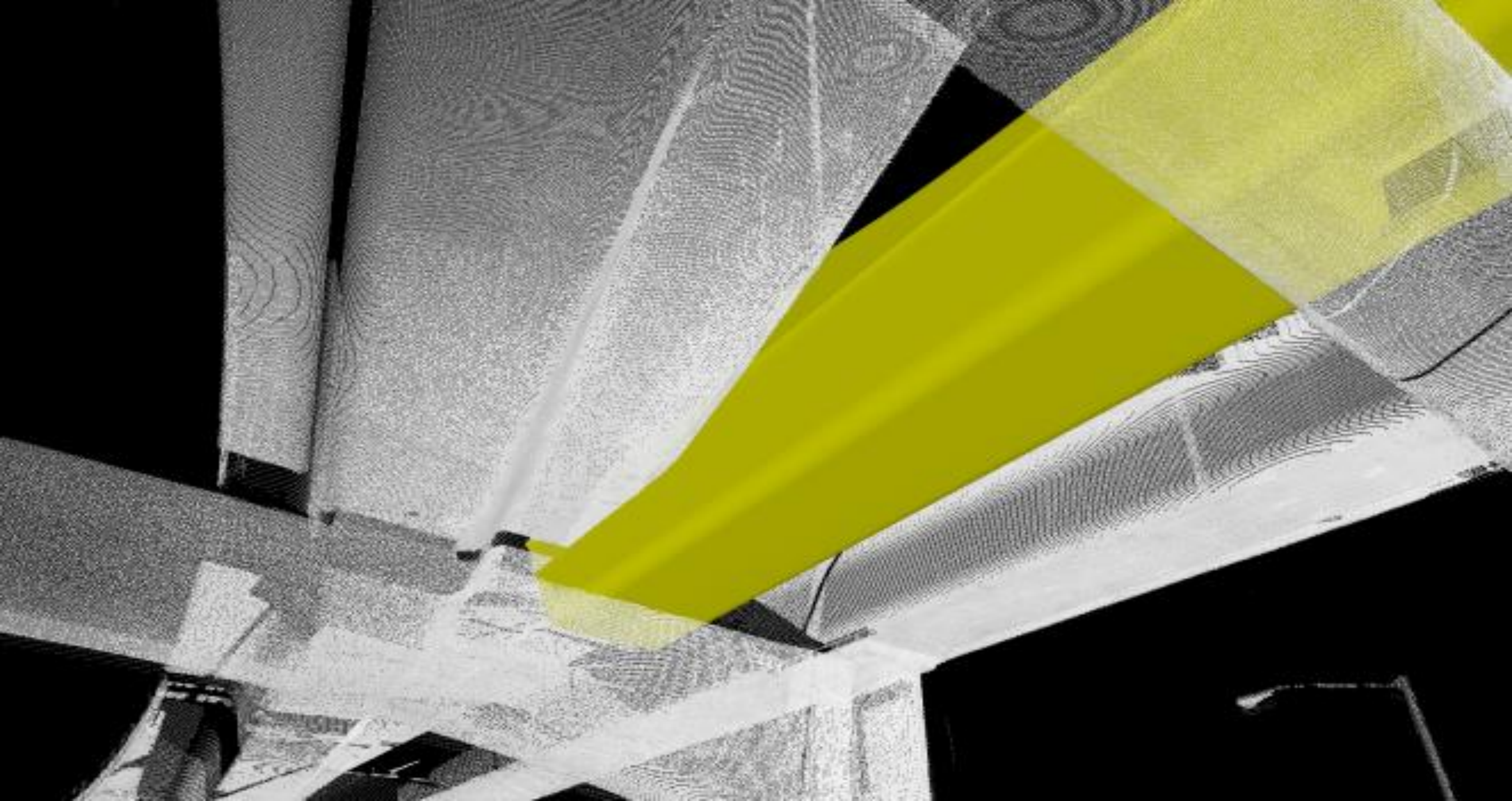




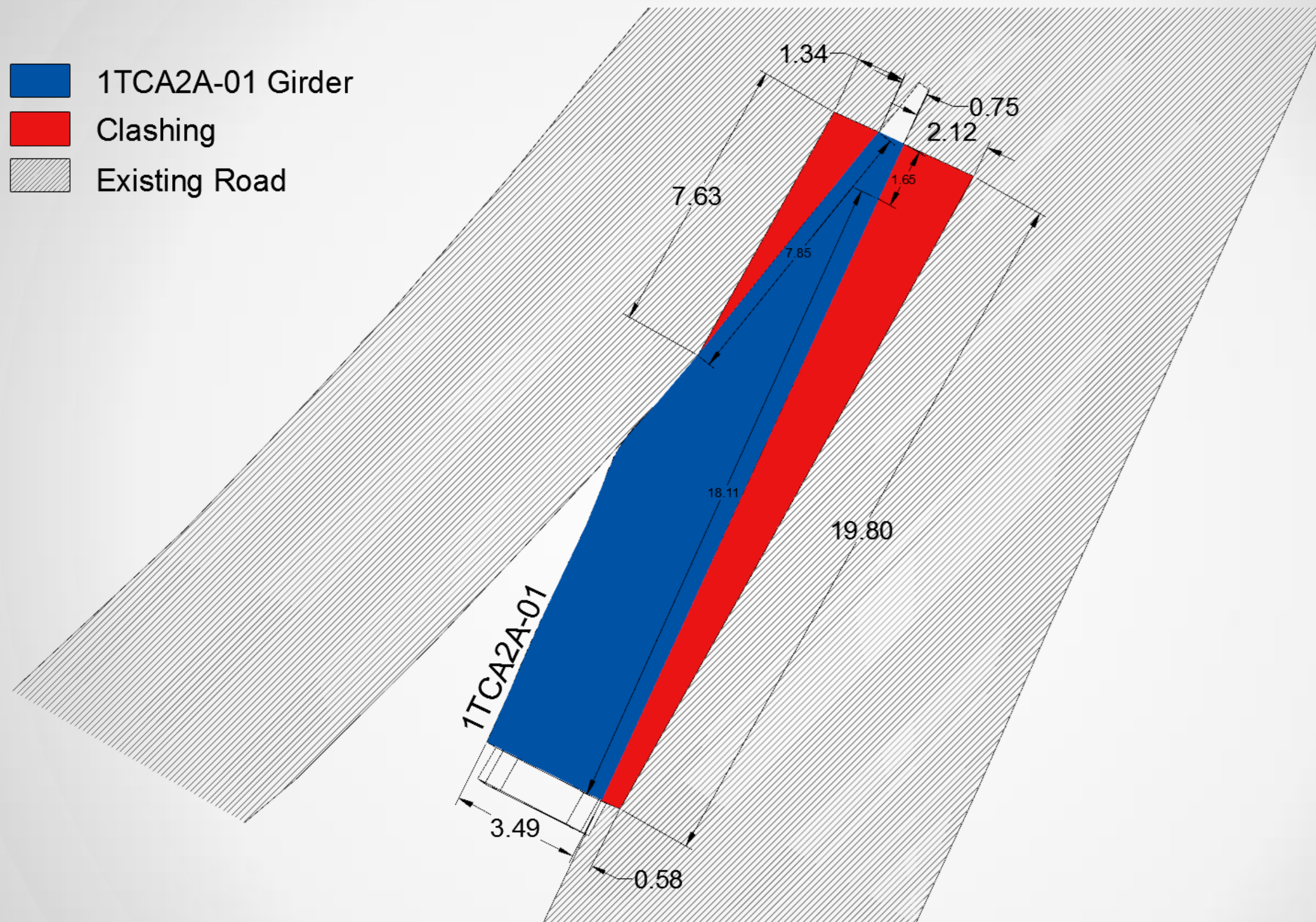




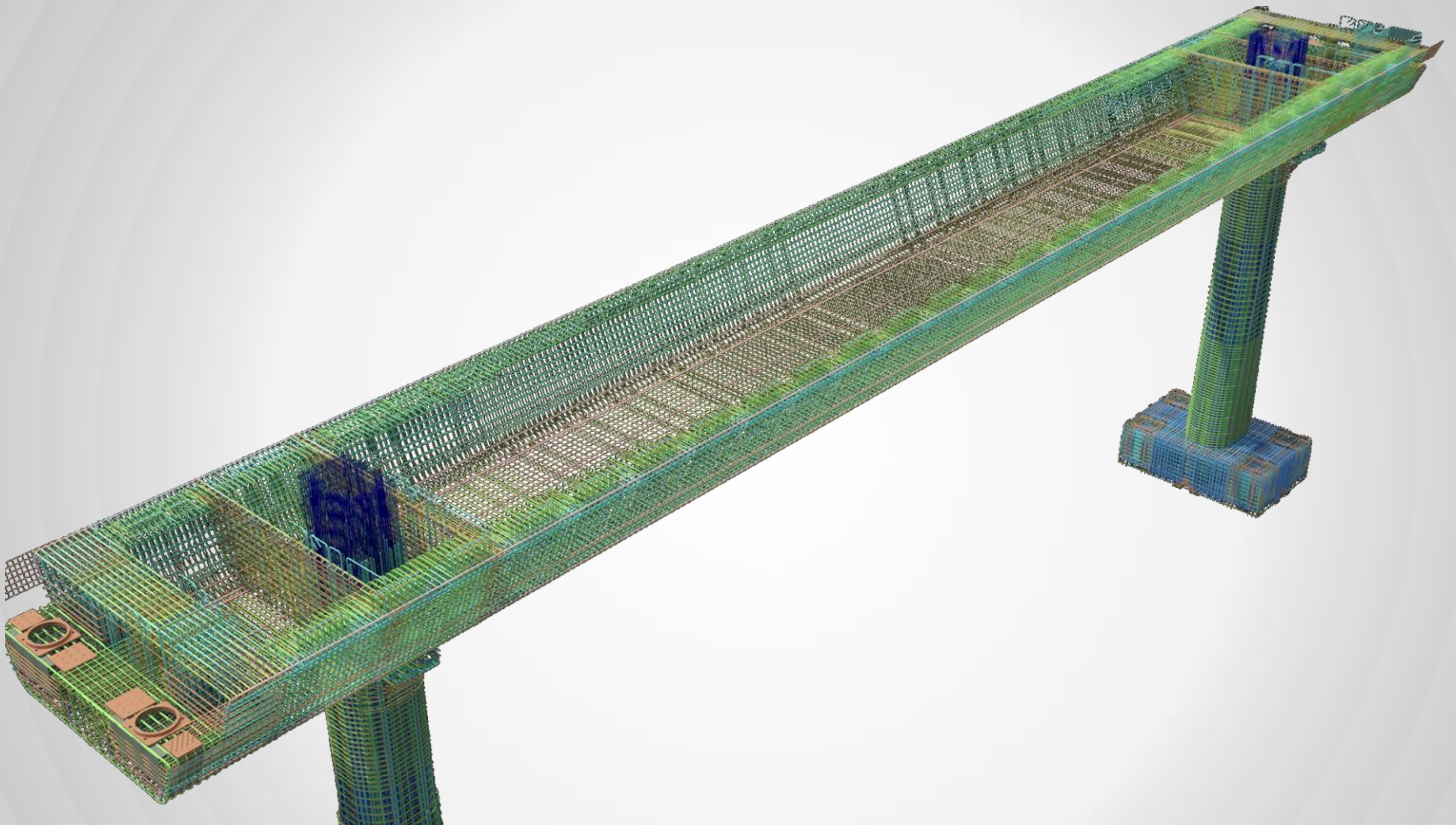
VISTA 3D

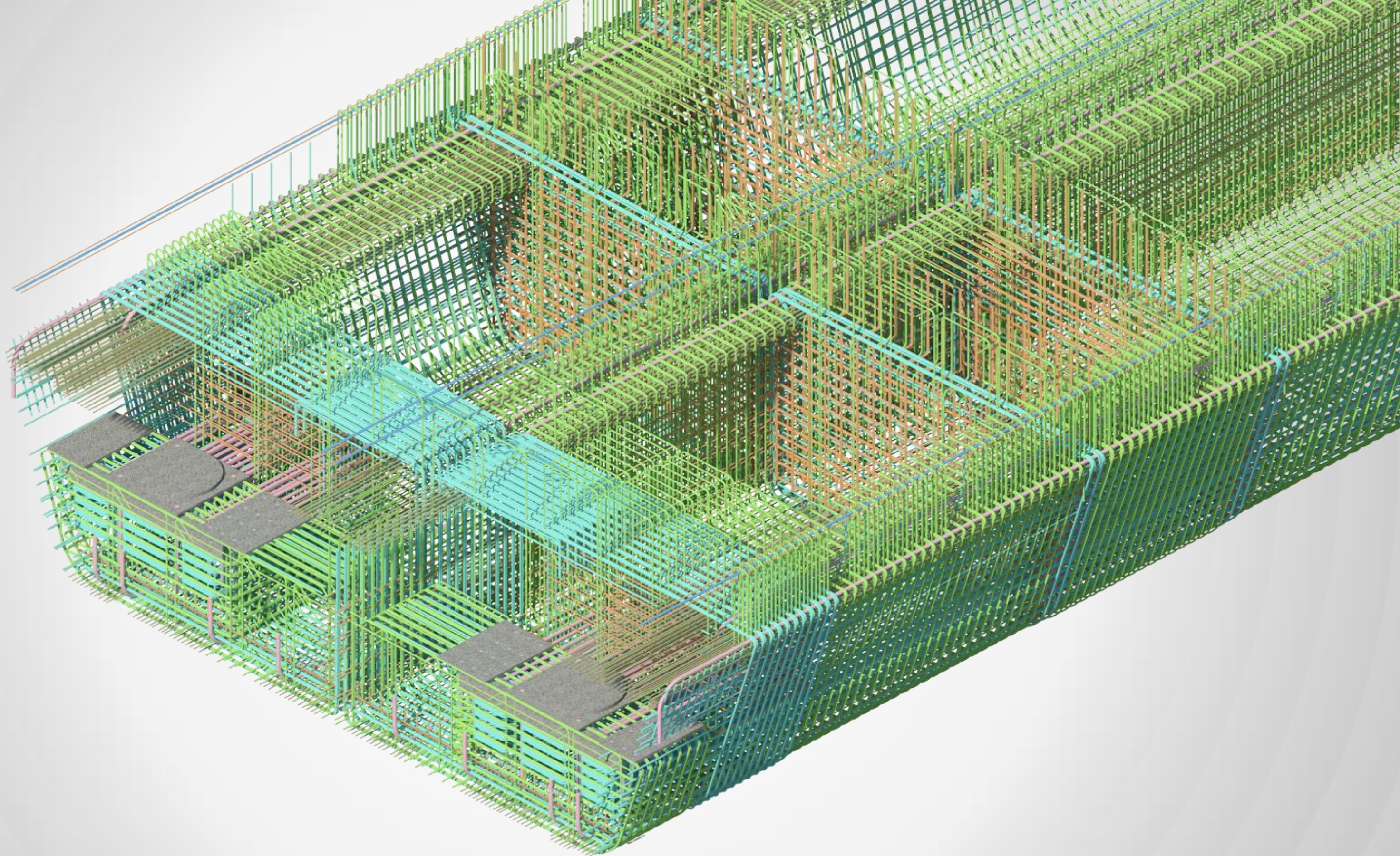


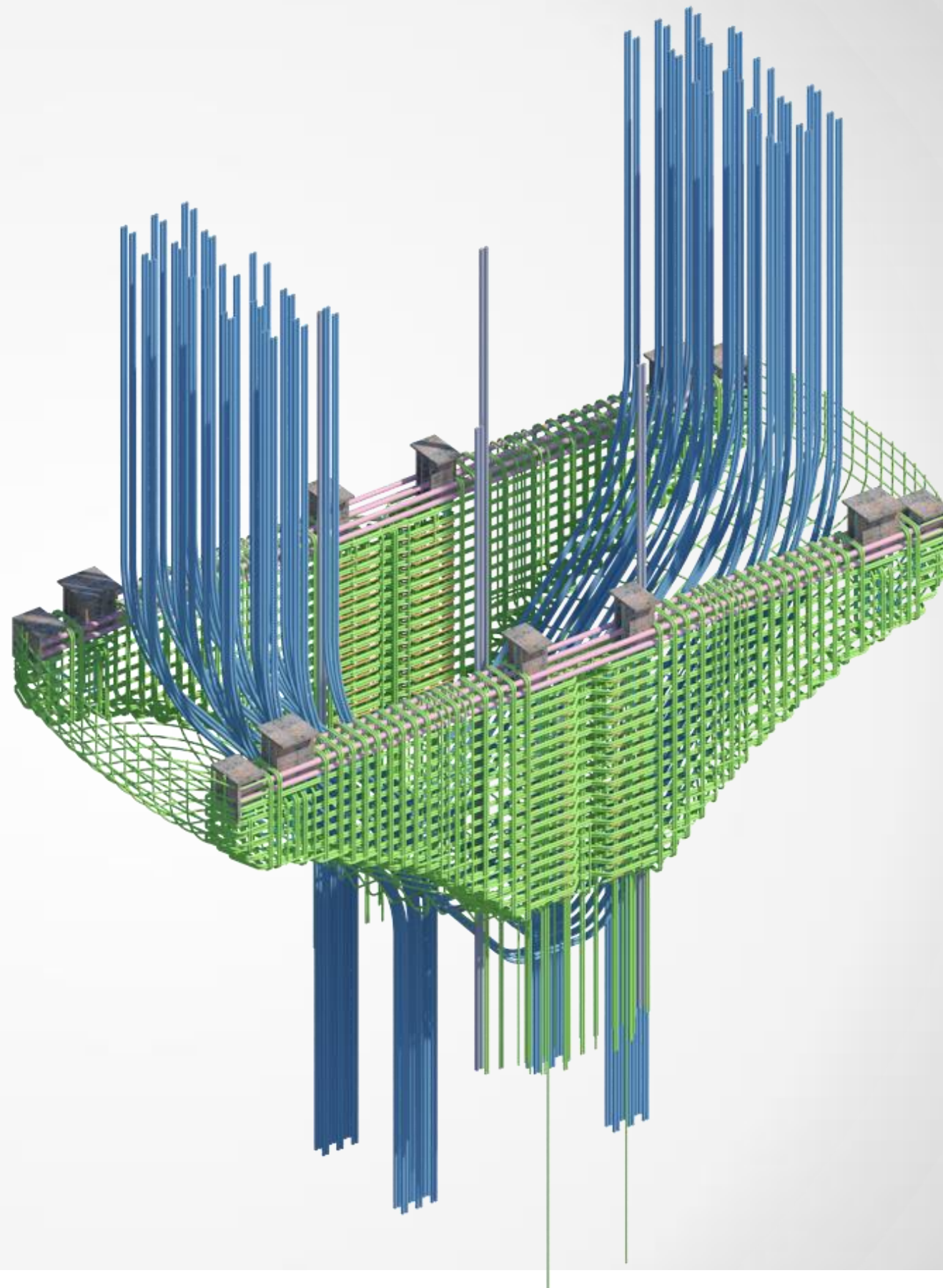
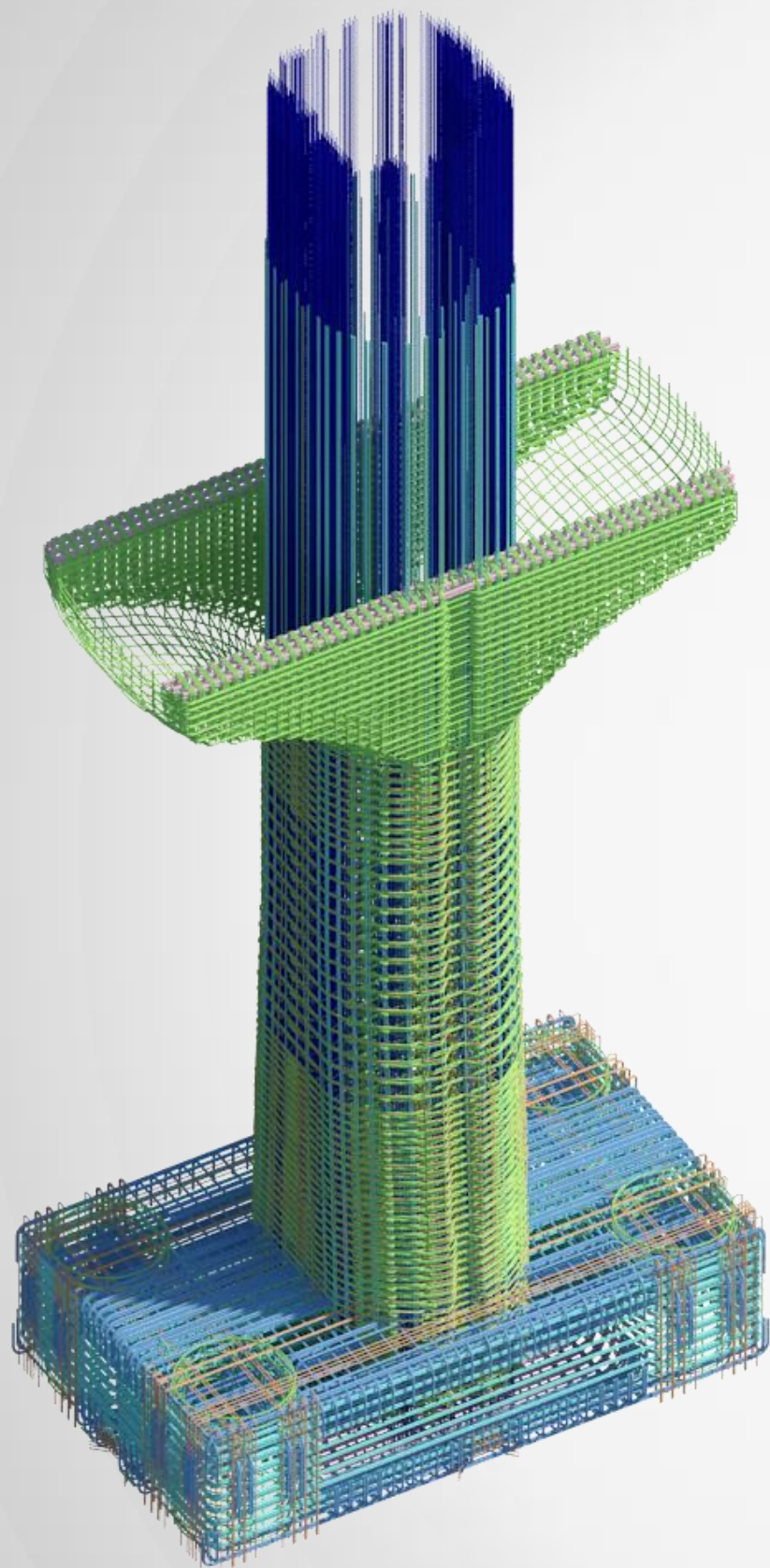
- 1TCA2A-01 Girder
- Clashing
- Existing Road

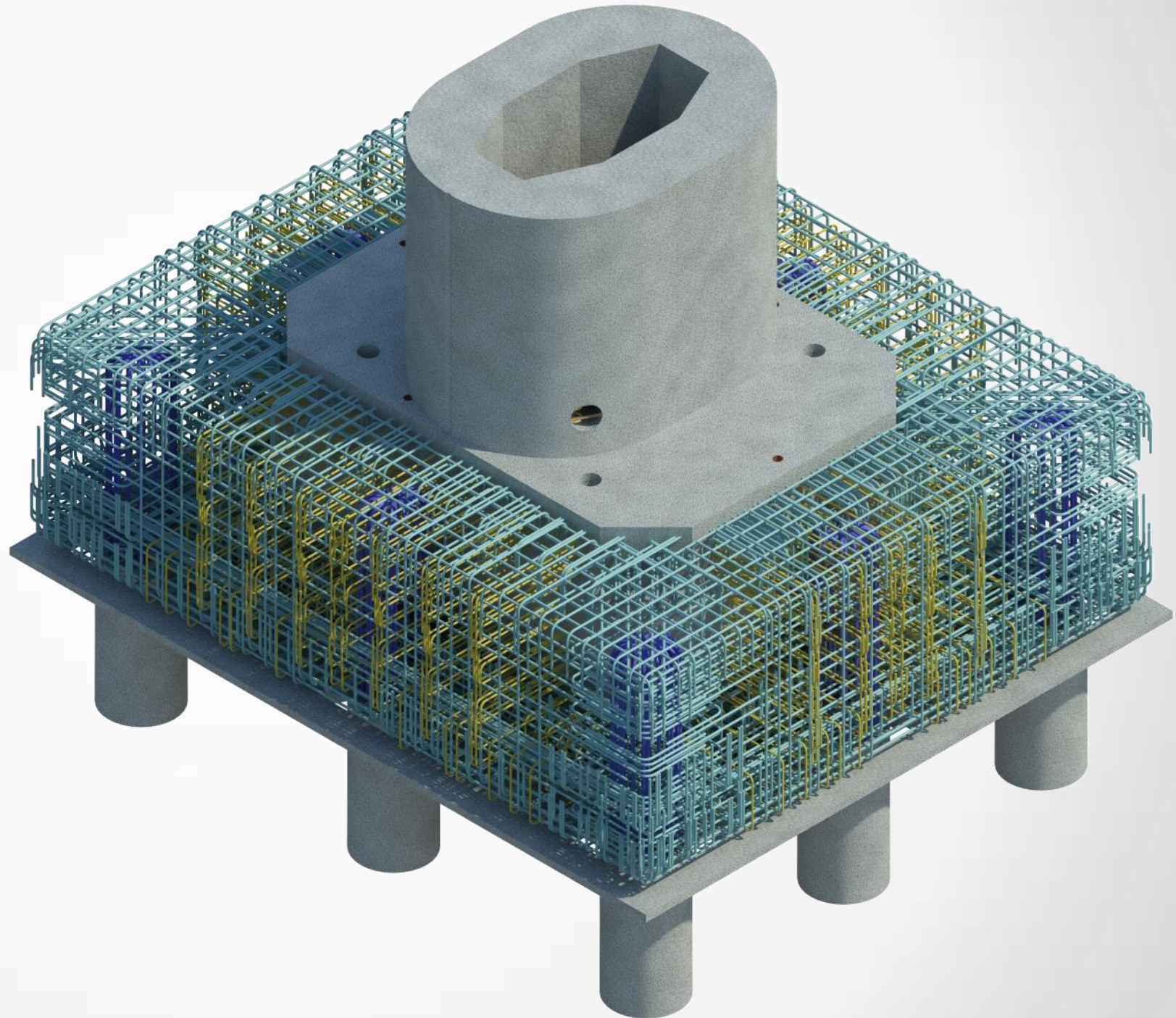


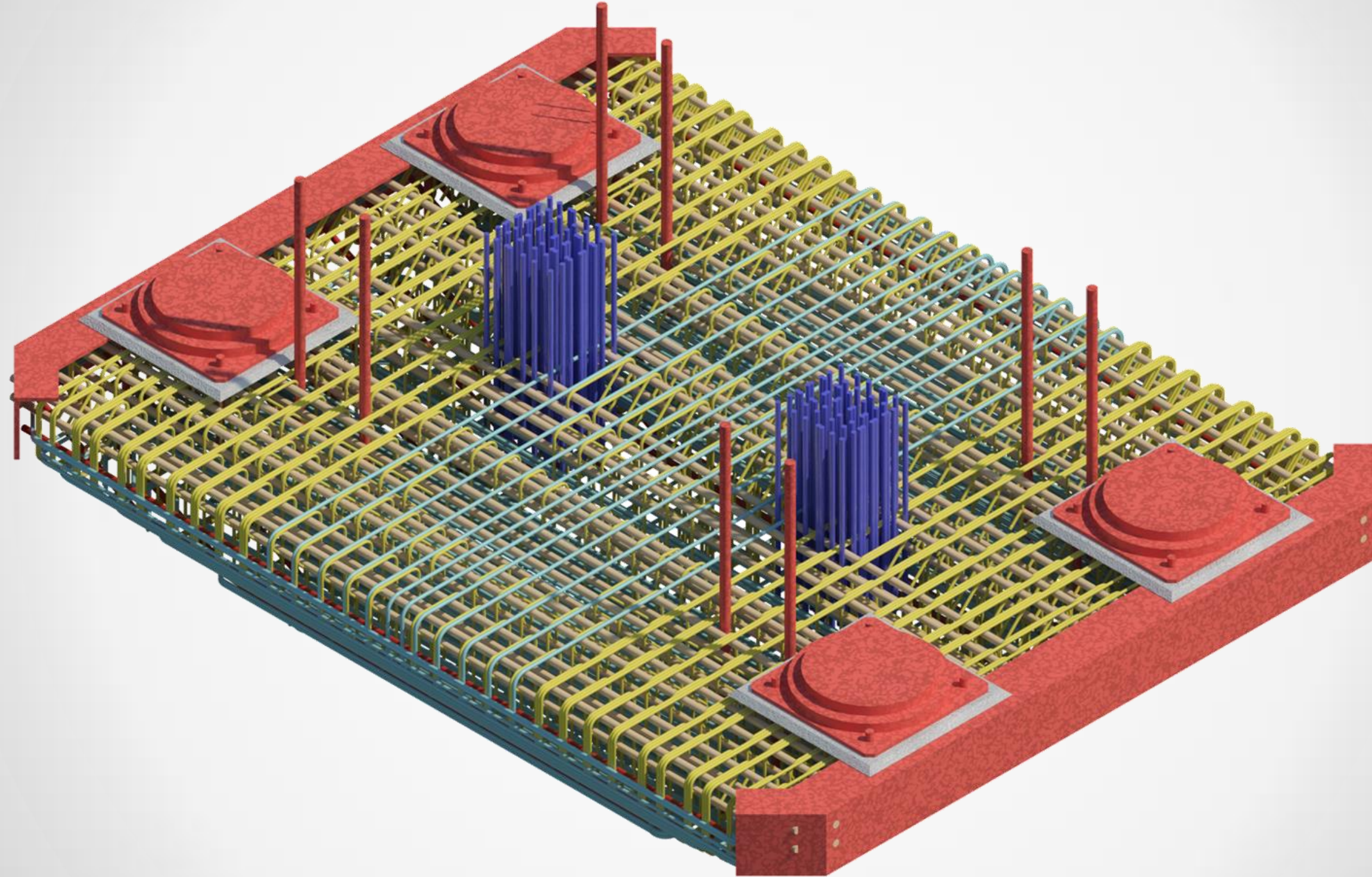
Rebar Models in Revit

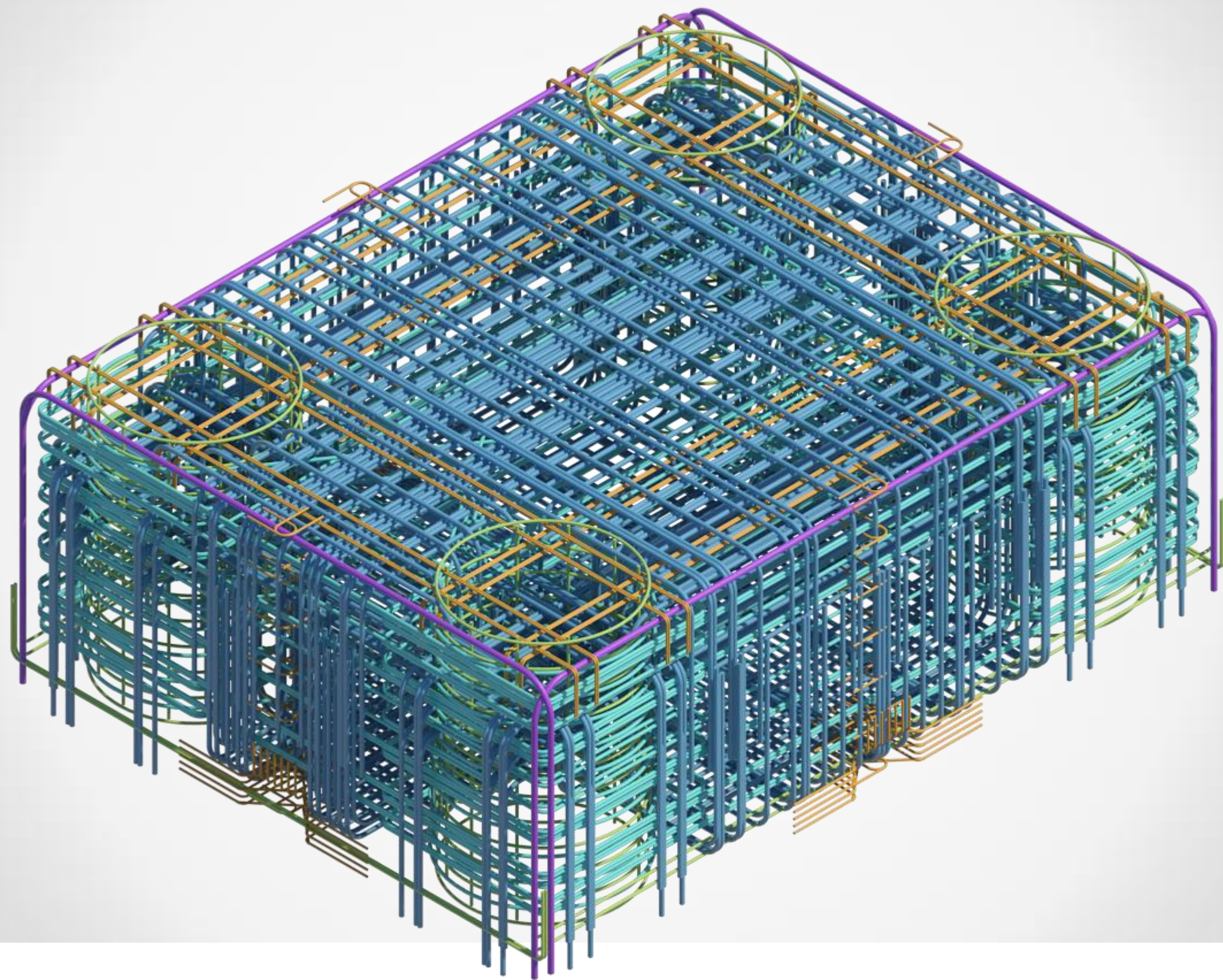


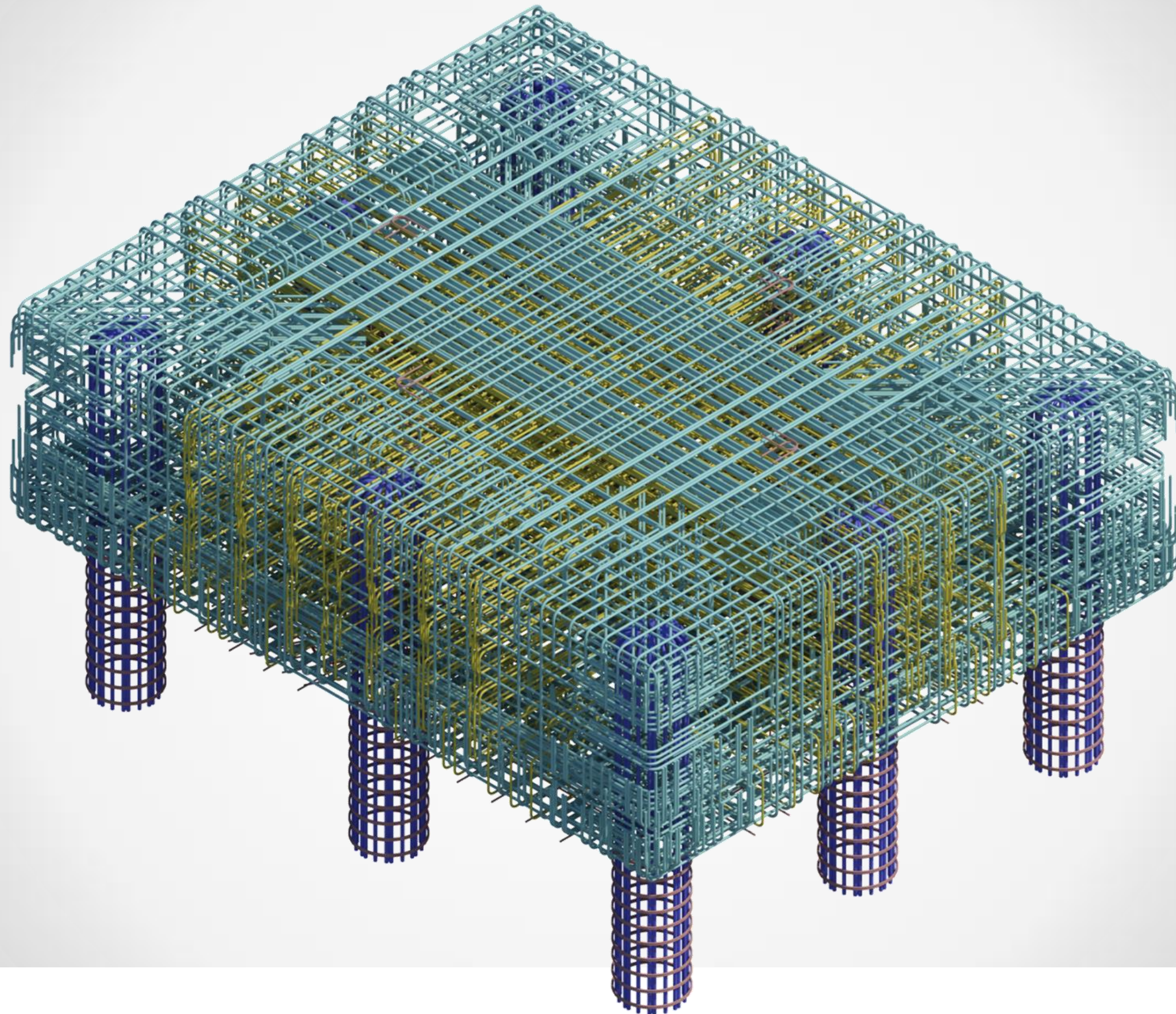












Shop Drawings



INGENIEROS CIVILES ASOCIADOS
PREFABRICADOS Y TRANSPORTES S.A. DE C.V.
BUILDING INFORMATION MODELING



ESPECIFICACIONES GENERALES:

PLANOS DE REFERENCIA:

VPE-PGE-F5-CUPRI-III-048-P-00
VPE-EST-F5-CUPRI-III-013-P-00 (05C26B11_G1 DE 2)
VPE-EST-F5-CUPRI-III-014-P-00 (05C26B11_G2 DE 2)
VPE-EST-F5-CUPRI-III-015-P-01 (05C26B11_R1 DE 3)
VPE-EST-F5-CUPRI-III-016-P-01 (05C26B11_R2 DE 3)
VPE-EST-F5-CUPRI-III-017-P-01 (05C26B11_R3 DE 3)

MATERIALES:

CONCRETO F'c= 600 kg/cm2
EDAD= 28 días
AGREGADO MAX. = 1/2" (1.27cm)
ACERO DE REFUERZO Fy= 4200 Kg/cm2
ACERO EN PLACAS, ACCESORIOS METALICOS Y
TENSORES Fy= 2530kg/cm2
ACERO DE PRESFUERZO Fy= 19000kg/cm2 (torones de 3/8")
ACERO ESTRUCTURAL = A-50
POLIESTIRENO:

ELEMENTO:
CAPITEL_B6

HOJA:
ECP01

CROQUIS:
GEOMETRIA

PIEZA A FABRICAR:
B6_8.50m

PLANO DE REFERENCIA:
ESCALA: 1 : 50

COTAS: CM

FECHA:
02/09/14

00_Resumen de Varillas por Diametro

Model	Diametro	Cantidad	Suma Longitudinal	PesoLineal	Peso Total
-------	----------	----------	-------------------	------------	------------

Capitel

#03	10 mm	84	136.87 m	0.560 kg/m	77 kg
#04	12 mm	462	1,642.26 m	0.996 kg/m	1,636 kg
#05	16 mm	112	350.22 m	1.560 kg/m	546 kg
#06	20 mm	48	372.83 m	2.250 kg/m	839 kg
#08	25 mm	40	352.94 m	3.975 kg/m	1,403 kg
#10	32 mm	16	84.57 m	6.225 kg/m	526 kg
		762	2,939.71 m		5,027 kg

02_Gancho de Izaje

CoidigoProyecto	Description	Cantidad	Suma Longitudinal	PesoLineal	Peso Total
-----------------	-------------	----------	-------------------	------------	------------

Gancho de Izaje

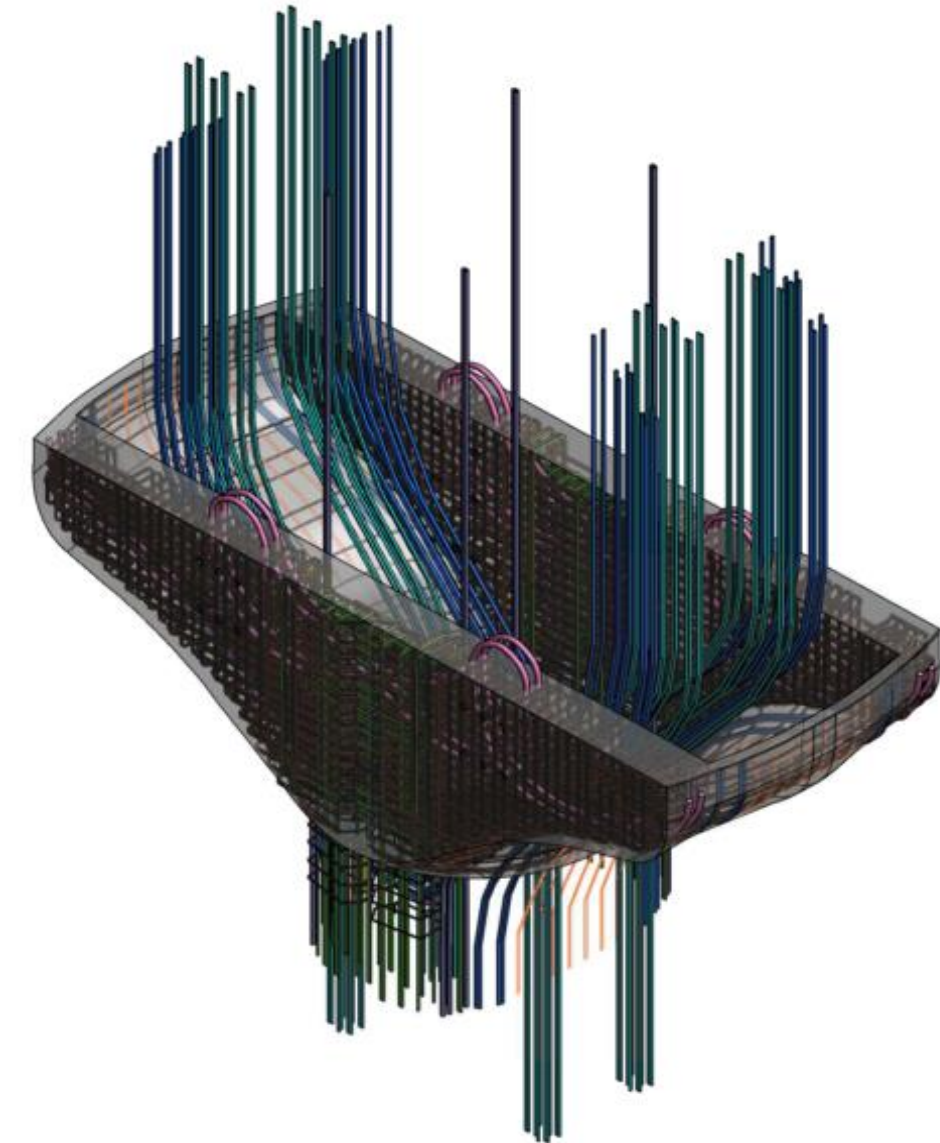
Izaje	Cable de 2" tipo Boa con alma de acero	8	54.17 m	8.938 kg/m	484 kg
		8	54.17 m		484 kg

03_Volumen de Geometria

Pertenece a	Tipo	Volumen
-------------	------	---------

Capitel

Capitel	Capitel 240x260_v3	7.624 m³
		7.624 m³



ELABORÓ:

Arq. Aarón De Marcos Estévez
MODELADOR

REVISÓ Y APROBÓ:

Ing. David Muñoz Vizuet
INGENIERIA

REVISÓ Y APROBÓ:

Ing. Fernando Mendez Galicia
SUPERINTENDENTE





INGENIEROS CIVILES ASOCIADOS
PREFABRICADOS Y TRANSPORTES S.A. DE C.V.
BUILDING INFORMATION MODELING



ESPECIFICACIONES GENERALES:

PLANOS DE REFERENCIA:

VPE-PGE-F5-CUPRI-III-048-P-00
VPE-EST-F5-CUPRI-III-013-P-00 (05CZ6B11_G1 DE 2)
VPE-EST-F5-CUPRI-III-014-P-00 (05CZ6B11_G2 DE 2)
VPE-EST-F5-CUPRI-III-015-P-01 (05CZ6B11_R1 DE 3)
VPE-EST-F5-CUPRI-III-016-P-01 (05CZ6B11_R2 DE 3)
VPE-EST-F5-CUPRI-III-017-P-01 (05CZ6B11_R3 DE 3)

MATERIALES:

CONCRETO $F'c = 600 \text{ kg/cm}^2$
EDAD=
AGREGADO MAX. = $1/2"$ (1.27cm)
ACERO DE REFUERZO $F_y = 4200 \text{ Kg/cm}^2$
ACERO EN PLACAS, ACCESORIOS METALICOS Y
TENSORES $F_y = 2530 \text{ kg/cm}^2$
ACERO DE PRESFUERZO $F_y = 19000 \text{ kg/cm}^2$ (torones de $3/8"$)
ACERO ESTRUCTURAL = A-50
POLIESTIRENO:

ELEMENTO:
CAPITEL_B6

HOJA:
ECP01

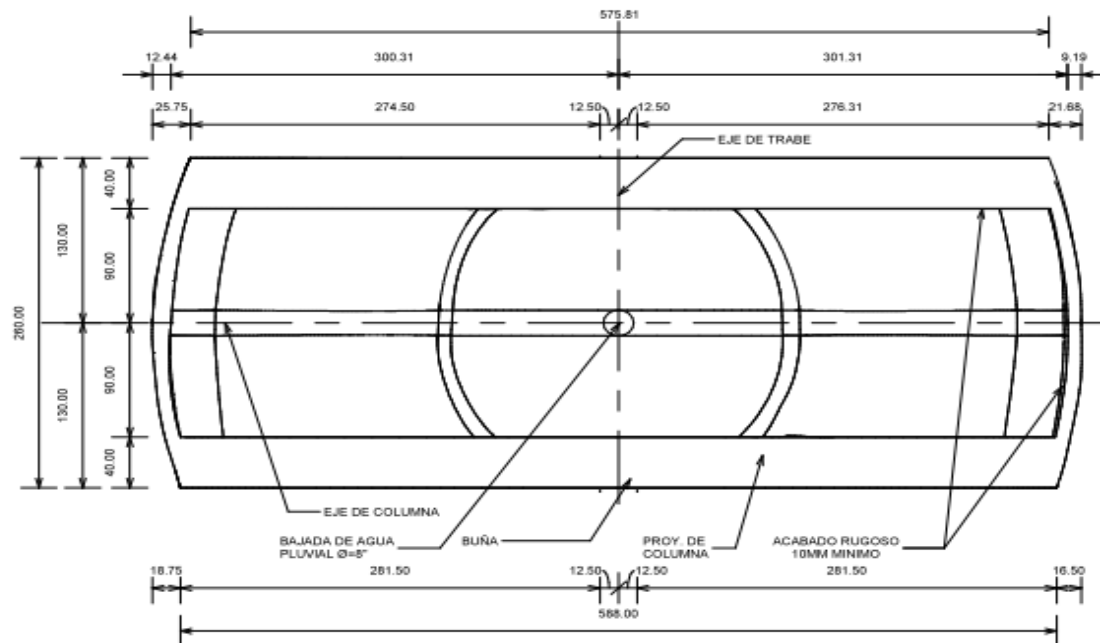
CROQUIS:
GEOMETRIA

PIEZA A FABRICAR:
B6_8.50m

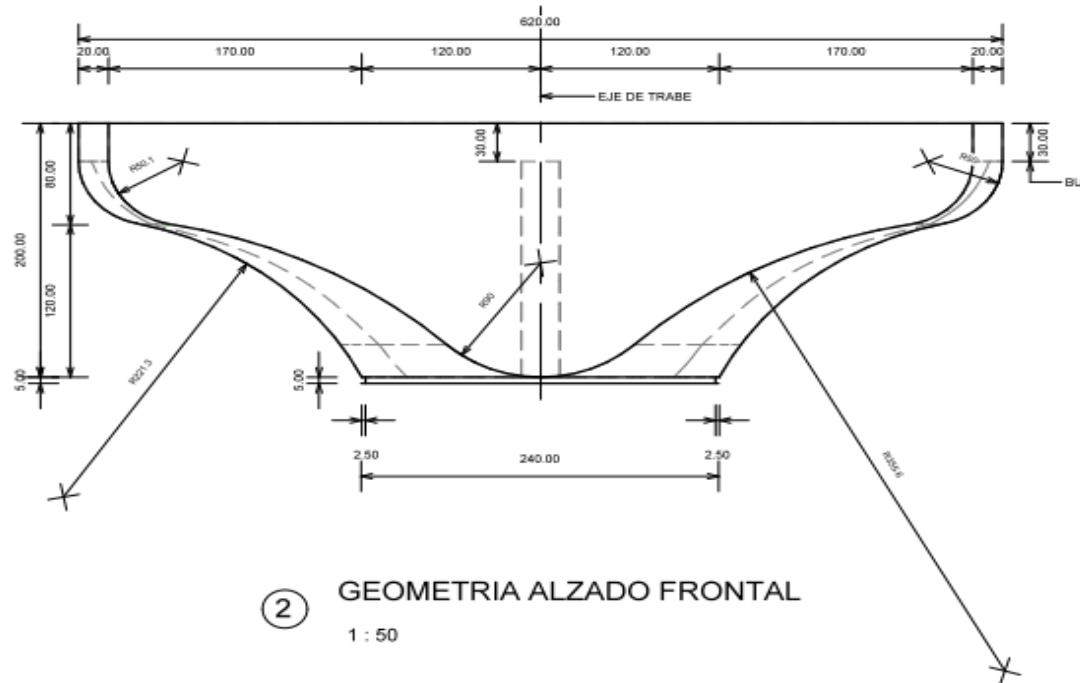
PLANO DE REFERENCIA:
ESCALA: 1 : 50

COTAS: CM

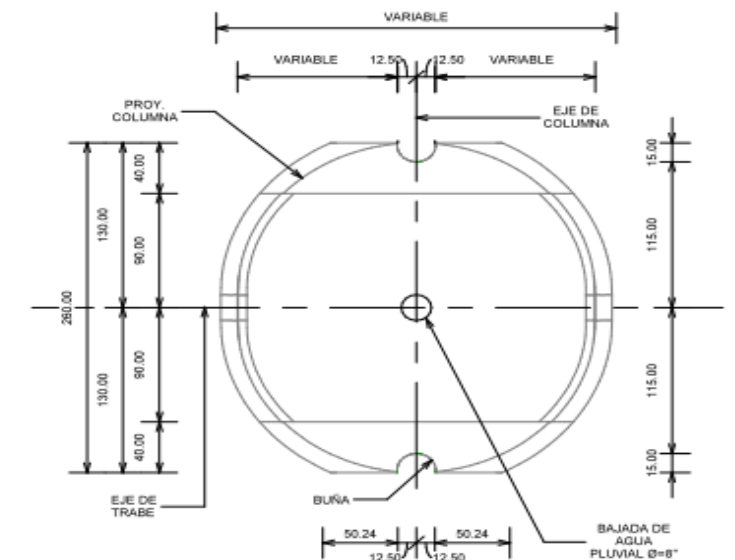
FECHA:
02/09/14



① GEOMETRIA PLANTA
1 : 50



② GEOMETRIA ALZADO FRONTAL
1 : 50



③ GEOMETRIA BASE CAPITEL
1 : 50



ELABORÓ:

Arq. Aarón De Marcos Estévez
MODELADOR

REVISÓ Y APROBÓ:

Ing. David Muñoz Vizuet
INGENIERIA

REVISÓ Y APROBÓ:

Ing. Fernando Mendez Galicia
SUPERINTENDENTE





INGENIEROS CIVILES ASOCIADOS
PREFABRICADOS Y TRANSPORTES S.A. DE C.V.
BUILDING INFORMATION MODELING



ESPECIFICACIONES GENERALES:

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VPE-EST-F5-CUPRI-III-014-P-00 (05CZ6B11_G2 DE 2)
VPE-EST-F5-CUPRI-III-015-P-01 (05CZ6B11_R1 DE 3)
VPE-EST-F5-CUPRI-III-016-P-01 (05CZ6B11_R2 DE 3)
VPE-EST-F5-CUPRI-III-017-P-01 (05CZ6B11_R3 DE 3)

MATERIALES:

CONCRETO $F'c = 600 \text{ kg/cm}^2$
EDAD =
AGREGADO MAX. = $1/2"$ (1.27cm)
ACERO DE REFUERZO $F_y = 4200 \text{ Kg/cm}^2$
ACERO EN PLACAS, ACCESORIOS METALICOS Y
TENSORES $F_y = 2530 \text{ kg/cm}^2$
ACERO DE PRESFUERZO $F_y = 19000 \text{ kg/cm}^2$ (torones de 3/8")
ACERO ESTRUCTURAL = A-50
POLIESTIRENO:

ELEMENTO:
CAPITEL_B6

HOJA:
ECP01

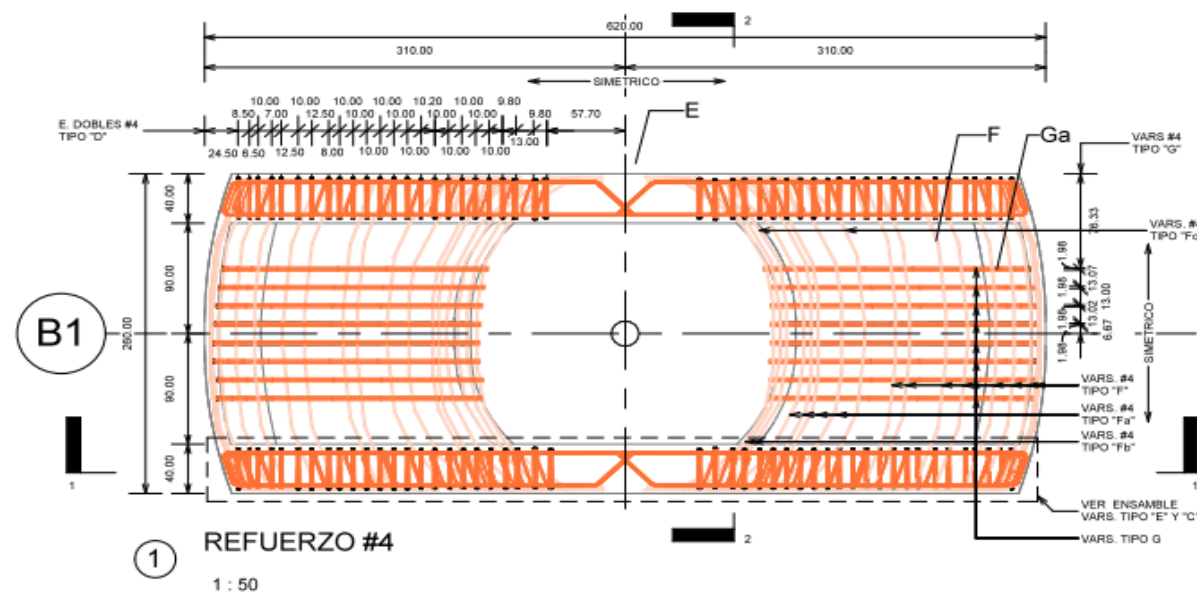
CROQUIS:
GEOMETRIA

PIEZA A FABRICAR:
B6_8.50m

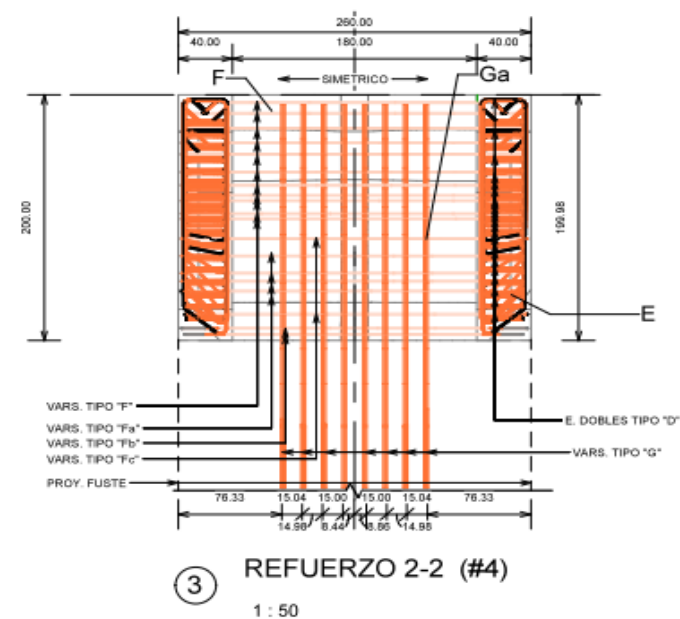
PLANO DE REFERENCIA:
ESCALA: 1 : 50

COTAS: CM

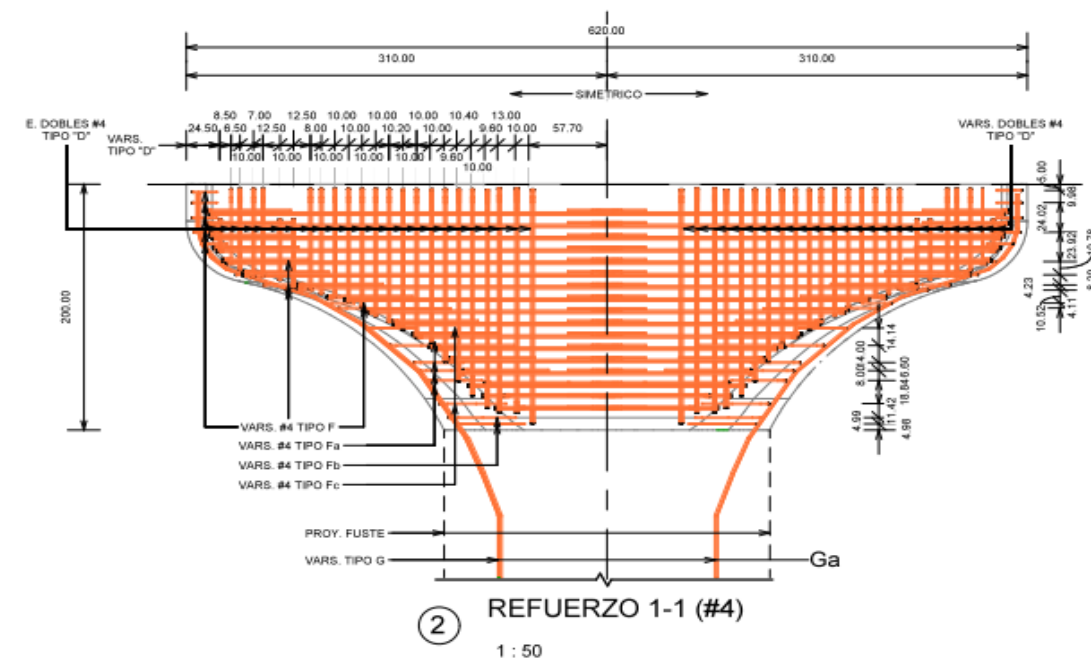
FECHA:
02/09/14



① REFUERZO #4
1 : 50



③ REFUERZO 2-2 (#4)
1 : 50



② REFUERZO 1-1 (#4)
1 : 50



ELABORÓ:
Arq. Aarón De Marcos Estévez
MODELADOR

REVISÓ Y APROBÓ:
Ing. David Muñoz Vizuet
INGENIERIA

REVISÓ Y APROBÓ:
Ing. Fernando Mendez Galicia
SUPERINTENDENTE





INGENIEROS CIVILES ASOCIADOS
PREFABRICADOS Y TRANSPORTES S.A. DE C.V.
BUILDING INFORMATION MODELING



ESPECIFICACIONES GENERALES:

PLANOS DE REFERENCIA:

VPE-PGE-F5-CUPRI-III-048-P-00
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VPE-EST-F5-CUPRI-III-014-P-00 (05C26B11_G2 DE 2)
VPE-EST-F5-CUPRI-III-015-P-01 (05C26B11_R1 DE 3)
VPE-EST-F5-CUPRI-III-016-P-01 (05C26B11_R2 DE 3)
VPE-EST-F5-CUPRI-III-017-P-01 (05C26B11_R3 DE 3)

MATERIALES:

CONCRETO F'c= 600 kg/cm2
EDAD= 28 días
AGREGADO MAX. = 1/2" (1.27cm)
ACERO DE REFUERZO Fy= 4200 Kg/cm2
ACERO EN PLACAS, ACCESORIOS METALICOS Y
TENSORES Fy= 2530kg/cm2
ACERO DE PRESFUERZO Fy= 19000kg/cm2 (torones de 3/8")
ACERO ESTRUCTURAL = A-50
POLIESTIRENO:

ELEMENTO:
CAPITEL_B6

HOJA:
ECP01

CROQUIS:
GEOMETRIA

PIEZA A FABRICAR:
B6_8.50m

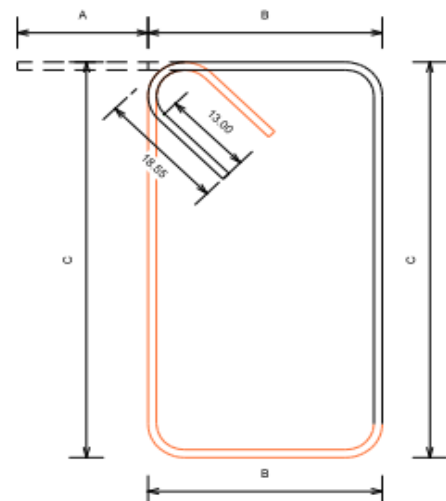
PLANO DE REFERENCIA:
ESCALA: 1 : 50

COTAS: CM

FECHA:
02/09/14

03_Varilla Tipo D

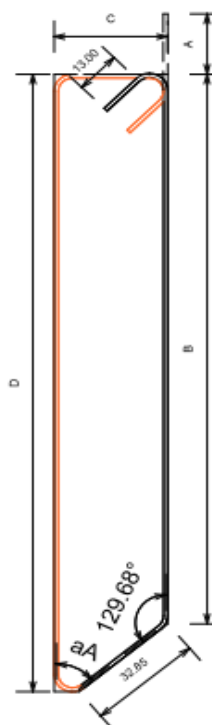
Codigo Proyecto	Clave	Modelo	Cantidad	Longitud Unitaria	Suma Longitudinal	A	B	C
D	D11	#04	8	312.59	25.01 m	18.64	33.42	109.00
D	D12	#04	8	302.59	24.21 m	18.64	33.42	104.00
D	D13	#04	8	287.76	23.02 m	18.64	33.42	96.59
D	D14	#04	8	279.53	22.36 m	18.64	33.42	92.47
D	D15	#04	8	272.33	21.79 m	18.64	33.42	88.87
D	D16	#04	8	266.33	21.31 m	18.64	33.42	85.87
D	D17	#04	8	258.50	20.68 m	18.64	33.42	81.96
D	D18	#04	8	205.94	16.48 m	18.64	33.42	55.68
D	D19	#04	8	194.59	15.57 m	18.64	33.42	50.00
D	D20	#04	8	238.64	19.09 m	18.64	33.42	72.03
D	D21	#04	8	238.64	19.09 m	18.64	33.42	72.03
D	D22	#04	8	227.32	18.19 m	18.64	33.42	66.37
D	D23	#04	4	216.88	8.68 m	18.64	33.42	61.15
D	D24	#04	8	146.59	11.73 m	18.64	33.42	26.00



① Varilla Tipo D
1 : 10

03_Varilla Tipo D_1

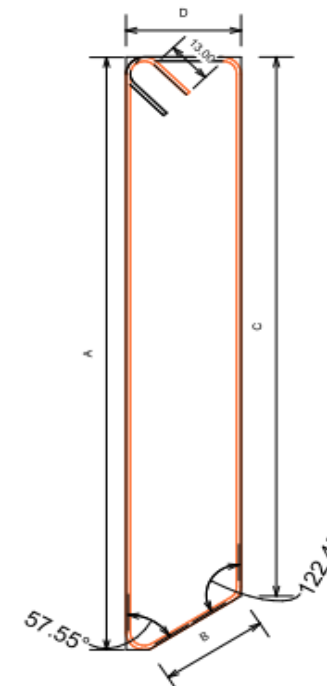
Codigo Proyecto	Clave	Modelo	Cantidad	Longitud Unitaria	Suma Longitudinal	A	B	C	D	aA
D	D01	#04	8	463.08	37.05 m	18.64	170.23	32.38	191.00	50.32°
D	D03	#04	8	445.10	35.61 m	18.64	154.47	32.61	183.00	41.34°
D	D05	#04	8	415.06	33.21 m	18.64	135.97	32.30	168.68	36.87°
D	D07	#04	8	363.63	29.09 m	18.64	123.04	32.30	141.00	55.23°
D	D09	#04	8	329.60	26.37 m	18.64	114.48	32.42	120.85	76.78°



② Varilla Tipo D01
1 : 20

03_Varilla Tipo D_2

Codigo Proyecto	Clave	Modelo	Cantidad	Longitud Unitaria	Suma Longitudinal	A	B	C	D
D	D02	#04	8	449.63	35.97 m	183.20	34.54	166.71	32.74



③ Varilla Tipo D02
1 : 20



ELABORÓ:

Arq. Aarón De Marcos Estévez
MODELADOR

REVISÓ Y APROBÓ:

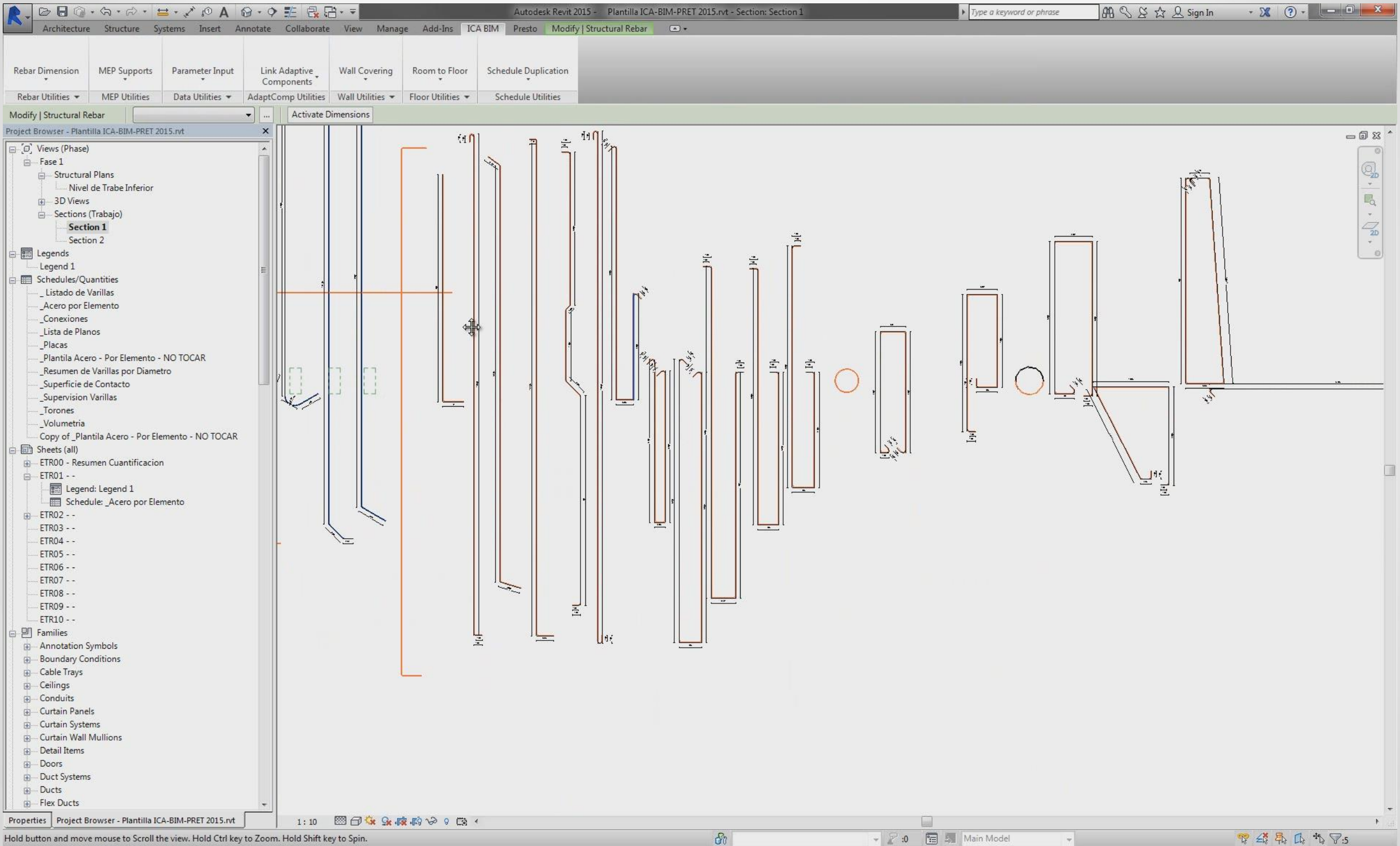
Ing. David Muñoz Vizuet
INGENIERIA

REVISÓ Y APROBÓ:

Ing. Fernando Mendez Galicia
SUPERINTENDENTE



Rebar Dimension App





INGENIEROS CIVILES ASOCIADOS
PREFABRICADOS Y TRANSPORTES S.A. DE C.V.
BUILDING INFORMATION MODELING



ESPECIFICACIONES GENERALES:

PLANOS DE REFERENCIA:

VPE-PGE-F5-CUPRI-III-048-P-00
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VPE-EST-F5-CUPRI-III-014-P-00 (05CZ6B11_G2 DE 2)
VPE-EST-F5-CUPRI-III-015-P-01 (05CZ6B11_R1 DE 3)
VPE-EST-F5-CUPRI-III-016-P-01 (05CZ6B11_R2 DE 3)
VPE-EST-F5-CUPRI-III-017-P-01 (05CZ6B11_R3 DE 3)

MATERIALES:

CONCRETO F'c= 600 kg/cm2
EDAD= 28 días
AGREGADO MAX. = 1/2" (1.27cm)
ACERO DE REFUERZO Fy= 4200 Kg/cm2
ACERO EN PLACAS, ACCESORIOS METALICOS Y
TENSORES Fy= 2530kg/cm2
ACERO DE PRESFUERZO Fy= 19000kg/cm2 (torones de 3/8")
ACERO ESTRUCTURAL = A-50
POLIESTIRENO:

ELEMENTO:
CAPITEL_B6

HOJA:
ECP01

CROQUIS:
GEOMETRIA

PIEZA A FABRICAR:
B6_8.50m

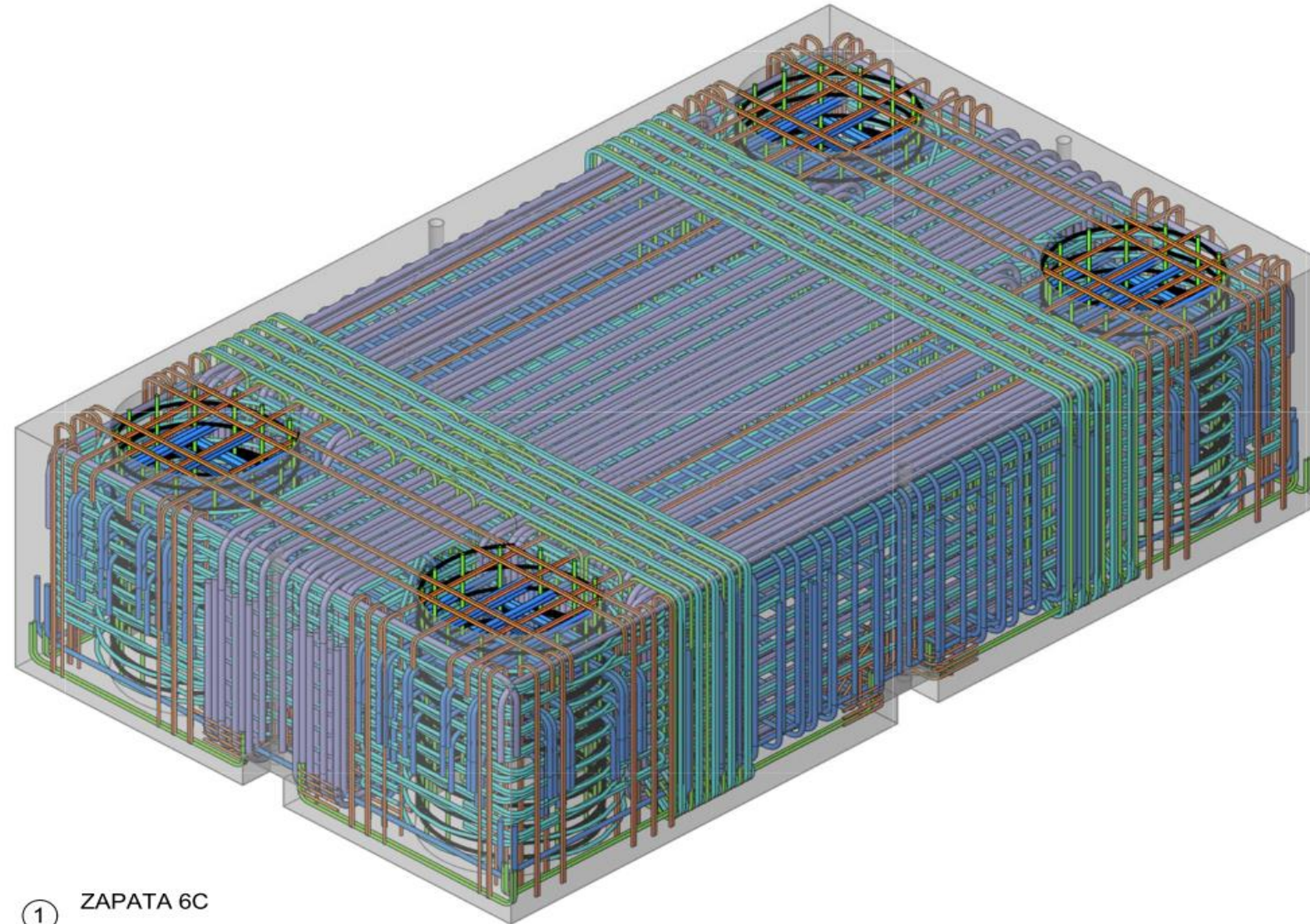
PLANO DE REFERENCIA:

ESCALA: 1 : 50

COTAS: CM

FECHA:
02/09/14

00_ Portada	
No de Plano	Título de Plano
EZPT00	ZAPATA DE 6 CARRILES ZPT 6C
EZPT01	GEOMETRIA
EZPT02	ACERO DE REFUERZO #4
EZPT03	ACERO DE REFUERZO #5
EZPT04	ACERO DE REFUERZO #6
EZPT05	ACERO DE REFUERZO #8
EZPT06	ACERO DE REFUERZO #10
EZPT07	DESPIECE
EZPT08	DESPIECE
EZPT09	ENSAMBLE 1/2
EZPT10	ENSAMBLE 2/2



ELABORÓ:

Arq. Aarón De Marcos Estévez
MODELADOR

REVISÓ Y APROBÓ:

Ing. David Muñoz Vizuet
INGENIERIA

REVISÓ Y APROBÓ:

Ing. Fernando Mendez Galicia
SUPERINTENDENTE





INGENIEROS CIVILES ASOCIADOS
PREFABRICADOS Y TRANSPORTES S.A. DE C.V.
BUILDING INFORMATION MODELING

ARCO SUR INTERTRAMO 5



ESPECIFICACIONES GENERALES:

PLANOS DE REFERENCIA:

VPE-PGE-F5-CUPRI-III-048-P-00
VPE-EST-F5-CUPRI-III-013-P-00 (05CZ6B11_G1 DE 2)
VPE-EST-F5-CUPRI-III-014-P-00 (05CZ6B11_G2 DE 2)
VPE-EST-F5-CUPRI-III-015-P-01 (05CZ6B11_R1 DE 3)
VPE-EST-F5-CUPRI-III-016-P-01 (05CZ6B11_R2 DE 3)
VPE-EST-F5-CUPRI-III-017-P-01 (05CZ6B11_R3 DE 3)

MATERIALES:

CONCRETO F'c= 600 kg/cm2
EDAD=
AGREGADO MAX. = 1/2" (1.27cm)
ACERO DE REFUERZO Fy= 4200 Kg/cm2
ACERO EN PLACAS, ACCESORIOS METALICOS Y
TENSORES Fy= 2530kg/cm2
ACERO DE PRESFUERZO Fy= 19000kg/cm2 (torones de 3/8")
ACERO ESTRUCTURAL = A-50
POLIESTIRENO:

ELEMENTO:
CAPITEL_B6

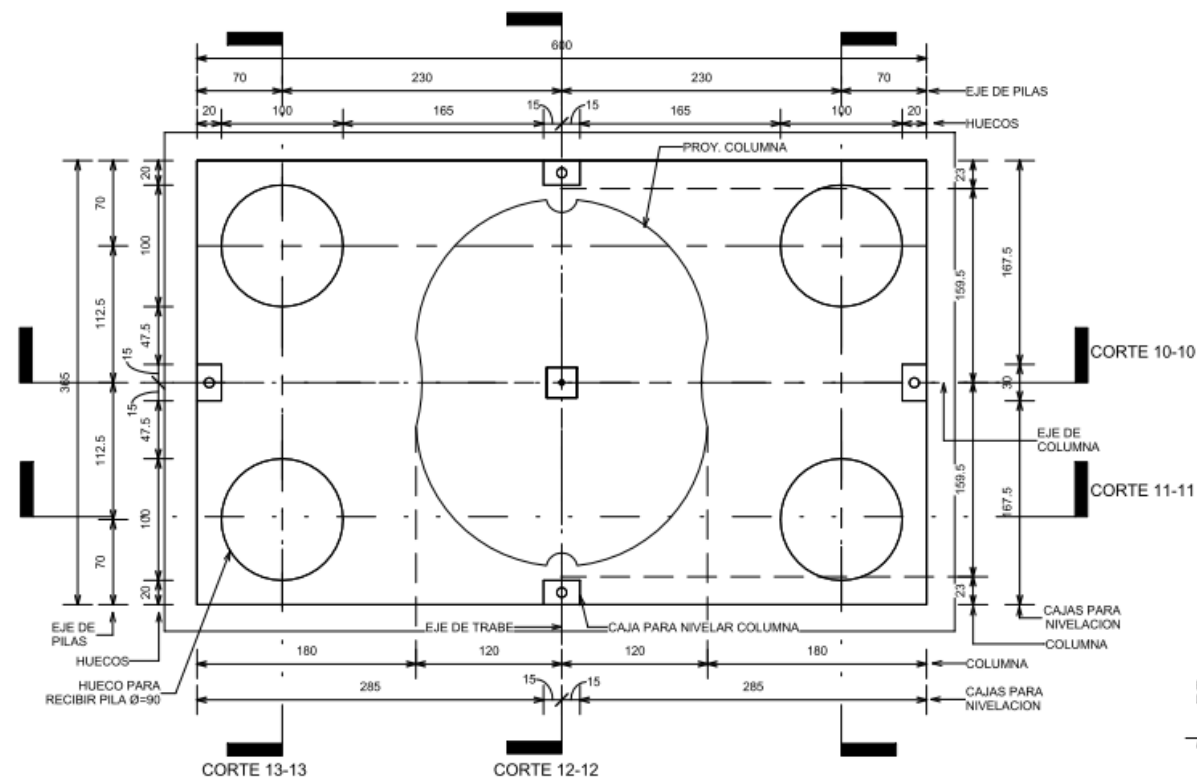
HOJA:
ECP01

CROQUIS:
GEOMETRIA

PIEZA A FABRICAR:
B6_8.50m

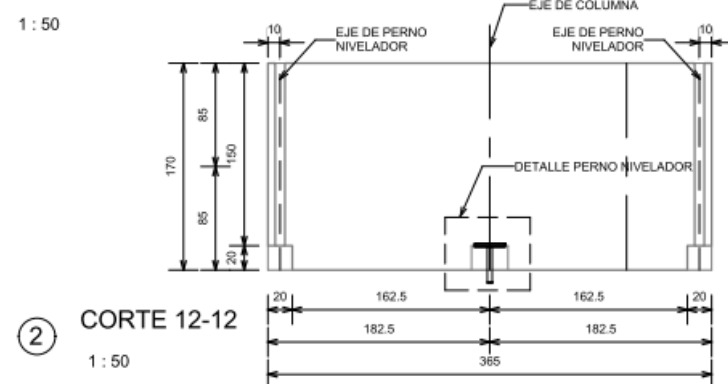
PLANO DE REFERENCIA:
ESCALA: 1 : 50 COTAS: CM

FECHA:
02/09/14



① PLANTA ZAPATA

1 : 50

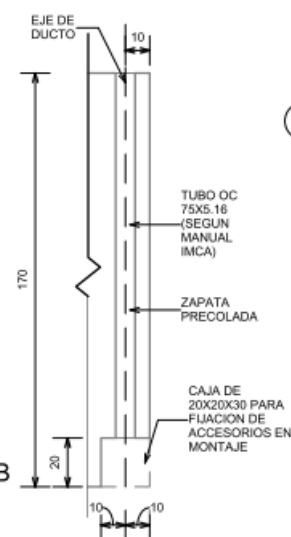


② CORTE 12-12

1 : 50

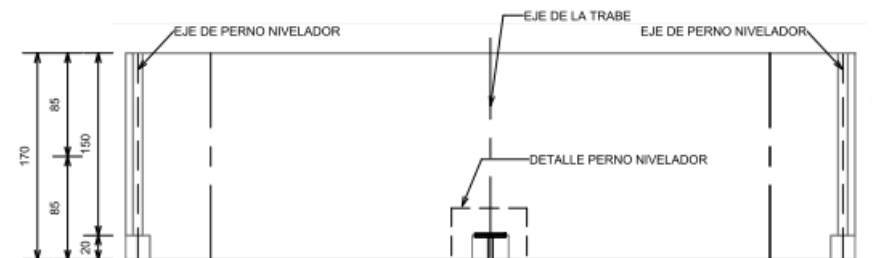
(1) PERNO NIVELADOR
RECUPERABLE L=190cm
Ø=1 1/4", fy=2530 kg/cm2
CON CABEZA
HEXAGONAL Y ROSCA
ESTANDAR

(2) COLOCAR ADITIVO QUE
GARANTICE LA RUGOSIDAD
DESDE EL COLADO



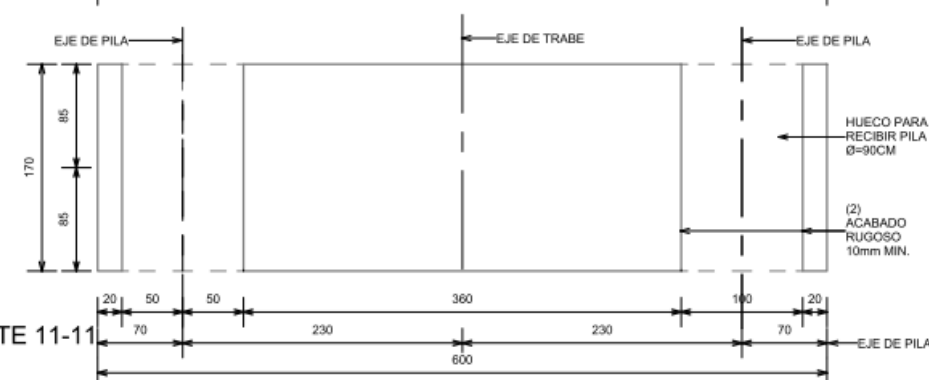
④ DETALLE-B

1 : 25



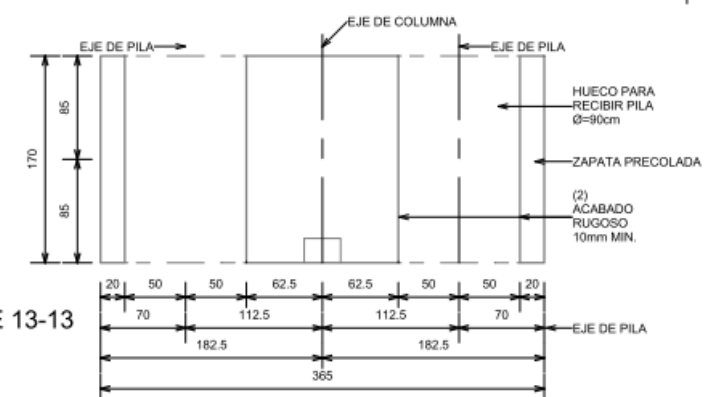
③ CORTE 10-10

1 : 50



⑤ CORTE 11-11

1 : 50



⑥ CORTE 13-13

1 : 50



ELABORÓ:
Arq. Aarón De Marcos Estévez
MODELADOR

REVISÓ Y APROBÓ:
Ing. David Muñoz Vizuet
INGENIERIA

REVISÓ Y APROBÓ:
Ing. Fernando Mendez Galicia
SUPERINTENDENTE



AUTODESK UNIVERSITY 2014





INGENIEROS CIVILES ASOCIADOS

PREFABRICADOS Y TRANSPORTES S.A. DE C.V.

BUILDING INFORMATION MODELING



ESPECIFICACIONES GENERALES:

PLANOS DE REFERENCIA:

VPE-PGE-F5-CUPRI-III-048-P-00
VPE-EST-F5-CUPRI-III-013-P-00 (05C26B11_G1 DE 2)
VPE-EST-F5-CUPRI-III-014-P-00 (05C26B11_G2 DE 2)
VPE-EST-F5-CUPRI-III-015-P-01 (05C26B11_R1 DE 3)
VPE-EST-F5-CUPRI-III-016-P-01 (05C26B11_R2 DE 3)
VPE-EST-F5-CUPRI-III-017-P-01 (05C26B11_R3 DE 3)

MATERIALES:

CONCRETO F'c= 600 kg/cm2
EDAD= 28 días
AGREGADO MAX. = 1/2" (1.27cm)
ACERO DE REFUERZO Fy= 4200 Kg/cm2
ACERO EN PLACAS, ACCESORIOS METALICOS Y
TENSORES Fy= 2530kg/cm2
ACERO DE PRESFUERZO Fy= 19000kg/cm2 (torones de 3/8")
ACERO ESTRUCTURAL = A-50
POLIESTIRENO:

ELEMENTO:
CAPITEL_B6

HOJA:
ECP01

CROQUIS:
GEOMETRIA

PIEZA A FABRICAR:
B6_8.50m

PLANO DE REFERENCIA:

ESCALA: 1 : 50

COTAS: CM

FECHA:
02/09/14

07_Varilla Tipo G

Codigo Proyecto	Shape	Clave	Modelo	Cantidad	Longitud Unitaria	Suma Longitudinal	B	C	D
G	E_17	G01	#05	8	201.10	16.09 m	26.00	158.25	26.00
G	E_17	G02	#05	8	314.99	25.20 m	26.00	272.14	26.00
G	E_17	G03	#06	32	396.82	126.98 m	32.20	343.40	32.20
G	E_17	G04	#06	32	397.62	127.24 m	32.60	343.40	32.60
G	E_17	G05	#06	34	627.49	213.35 m	32.20	574.07	32.20
G	E_17	G06	#06	66	631.82	417.00 m	32.20	578.40	32.20
G	E_17	G07	#06	17	632.38	107.50 m	32.48	578.40	32.48
G	E_17	G08	#06	17	632.62	107.55 m	32.60	578.40	32.60
G	E_17	G09	#08	4	332.35	13.29 m	120.25	106.00	120.25
G	E_17	G10	#08	8	336.35	26.91 m	120.25	110.00	120.25
G	E_17	G11	#08	8	342.35	27.39 m	118.25	120.00	118.25
G	E_17	G13	#08	2	358.35	7.17 m	118.25	136.00	118.25
G	E_17	G14	#08	2	362.35	7.25 m	120.25	136.00	120.25
G	E_17	G15	#08	2	360.35	7.21 m	120.25	134.00	120.25
G	E_17	G16	#08	4	358.35	14.33 m	118.25	136.00	118.25
G	E_17	G18	#08	2	362.35	7.25 m	118.25	140.00	118.25
G	E_17	G19	#08	2	382.35	7.65 m	118.25	160.00	118.25
G	E_17	G20	#08	2	391.35	7.83 m	120.25	165.00	120.25
G	E_17	G21	#08	2	382.35	7.65 m	118.25	160.00	118.25
G	E_17	G22	#08	16	411.85	65.90 m	42.00	342.00	42.00
G	E_17	G23	#08	16	424.85	67.98 m	42.00	355.00	42.00
G	E_17	G24	#08	8	417.85	33.43 m	42.00	348.00	42.00
G	E_17	G25	#08	2	425.29	8.51 m	42.25	354.94	42.25
G	E_17	G26	#08	16	424.85	67.98 m	42.00	355.00	42.00
G	E_17	G27	#08	2	522.85	10.46 m	108.50	320.00	108.50
G	E_17	G28	#08	1	533.63	5.34 m	108.50	330.78	108.50
G	E_17	G29	#08	10	545.88	54.59 m	105.50	349.03	105.50
G	E_17	G30	#08	3	586.35	17.59 m	122.75	355.00	122.75
G	E_17	G31	#08	22	557.85	122.73 m	108.50	355.00	108.50
G	E_17	G32	#08	22	586.35	129.00 m	122.75	355.00	122.75
G	E_17	G33	#08	14	647.85	90.70 m	42.00	578.00	42.00
G	E_17	G34	#08	14	659.85	92.38 m	42.00	590.00	42.00
G	E_17	G35	#08	14	653.35	91.47 m	42.25	583.00	42.25
G	E_17	G36	#08	2	656.92	13.14 m	42.00	587.07	42.00
G	E_17	G37	#08	14	660.35	92.45 m	42.25	590.00	42.25
G	E_17	G38	#10	2	431.89	8.64 m	52.00	346.56	52.00
G	E_17	G39	#10	2	541.90	10.84 m	110.99	338.60	110.99
G	E_17	G40	#10	2	542.16	10.84 m	112.42	336.00	112.42
G	E_17	G41	#10	2	546.16	10.92 m	112.42	340.00	112.42
G	E_17	G41	#10	6	554.47	33.27 m	121.57	330.00	121.57
G	E_17	G42	#10	4	540.16	21.61 m	112.42	334.00	112.42
G	E_17	G43a	#10	4	541.90	21.68 m	110.99	338.60	110.99
G	E_17	G43b	#10	4	551.90	22.08 m	110.99	348.60	110.99
G	E_17	G44	#10	2	559.30	11.19 m	110.99	356.00	110.99
G	E_17	G45	#10	2	560.47	11.21 m	121.57	336.00	121.57
G	E_17	G46	#10	4	573.47	22.94 m	126.07	340.00	126.07
G	E_17	G47	#10	2	579.46	11.59 m	126.07	346.00	126.07
G	E_17	G48	#10	2	562.16	11.24 m	112.42	356.00	112.42
G	E_17	G49	#10	4	571.59	22.86 m	122.13	346.00	122.13
G	E_17	G50	#10	4	570.20	22.81 m	122.44	344.00	122.44
G	E_17	G51	#10	2	572.76	11.46 m	112.42	366.60	112.42
G	E_17	G52a	#10	4	573.80	22.95 m	122.44	347.60	122.44
G	E_17	G52b	#10	4	592.20	23.69 m	122.44	366.00	122.44
G	E_17	G53	#10	2	581.93	11.64 m	122.30	356.00	122.30
G	E_17	G54	#10	2	591.59	11.83 m	122.13	366.00	122.13

07_Varilla Tipo G

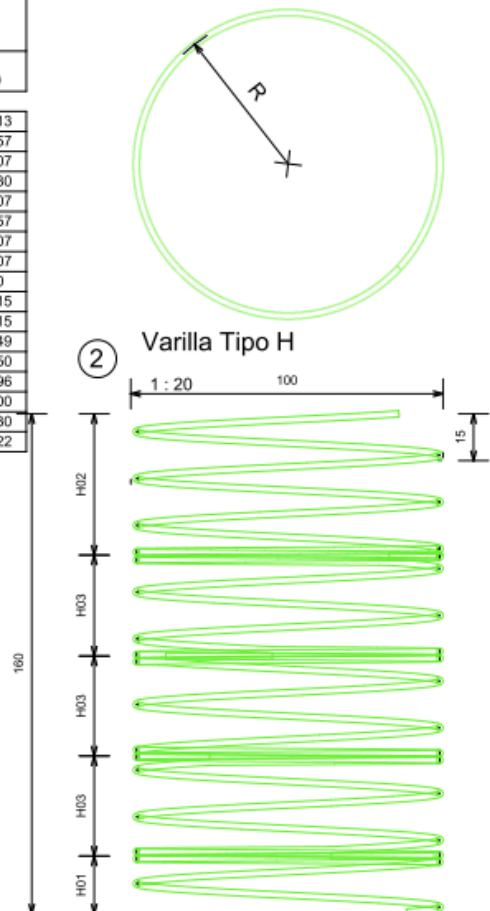
Codigo Proyecto	Shape	Clave	Modelo	Cantidad	Longitud Unitaria	Suma Longitudinal	B	C	D
G	E_17	G55	#10	2	601.19	12.02 m	122.13	375.60	122.13
G	E_17	G56	#10	2	592.47	11.85 m	121.57	368.00	121.57
G	E_17	G57	#10	2	598.46	11.97 m	126.07	365.00	126.07
G	E_17	G58	#10	4	600.93	24.04 m	122.30	375.00	122.30
G	E_17	G59	#10	2	611.47	12.23 m	126.07	378.00	126.07
G	E_17	G60	#10	2	614.47	12.29 m	121.57	390.00	121.57
G	E_17	G61	#10	2	618.46	12.37 m	126.07	385.00	126.07
G	E_17	G62	#10	2	633.47	12.67 m	126.07	400.00	126.07
G	E_17	G63	#10	2	670.74	13.41 m	52.00	585.41	52.00
G	E_17	G64	#10	3	723.63	21.71 m	105.15	532.00	105.15
G	E_17	G65	#10	8	761.63	60.93 m	105.15	570.00	105.15
G	E_17	G66	#10	3	748.52	22.46 m	113.49	540.23	113.49
G	E_17	G67	#10	8	788.33	63.07 m	113.50	580.00	113.50
G	E_17	G68	#10	3	767.24	23.02 m	117.96	550.00	117.96
G	E_17	G69	#10	8	807.33	64.59 m	118.00	590.00	118.00
G	E_17	G70	#10	12	803.93	96.47 m	126.30	570.00	126.30
G	E_17	G71	#10	6	831.77	49.91 m	130.22	590.00	130.22



1 Varilla Tipo G
1 : 20

08_Varilla Tipo H

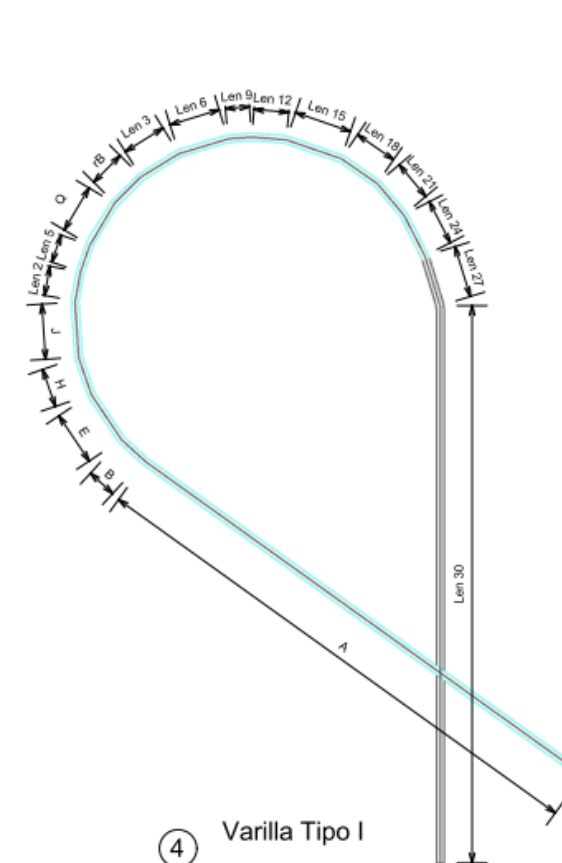
Codigo Proyecto	Clave	Modelo	Cantidad	Longitud Unitaria	Suma Longitudinal	R
H	H01	#05	4	628.81	25.15 m	49.60
H	H03	#05	12	1178.74	141.45 m	49.60
H	H02	#05	4	1182.97	47.32 m	49.60



3 Varilla Tipo H_Zuncho
1 : 20

09_Varilla Tipo I

Codigo Proyecto	Clave	Modelo	Cantidad	Longitud Unitaria	Suma Longitudinal	A	B	C	E	R	H	J	Len 2	Q	rB	Len 3	Len 6	Len 9	Len 12	Len 15	Len 18	Len 21	Len 24	Len 27	Len 30
I	I01	#06	40	581.57	232.63 m	172.57	10.30	10.22	17.71	0.91	12.82	18.34	9.13	14.44	12.22	3.92	16.22	7.81	11.45	19.41	14.40	13.52	15.32	16.64	178.42



4 Varilla Tipo I
1 : 20

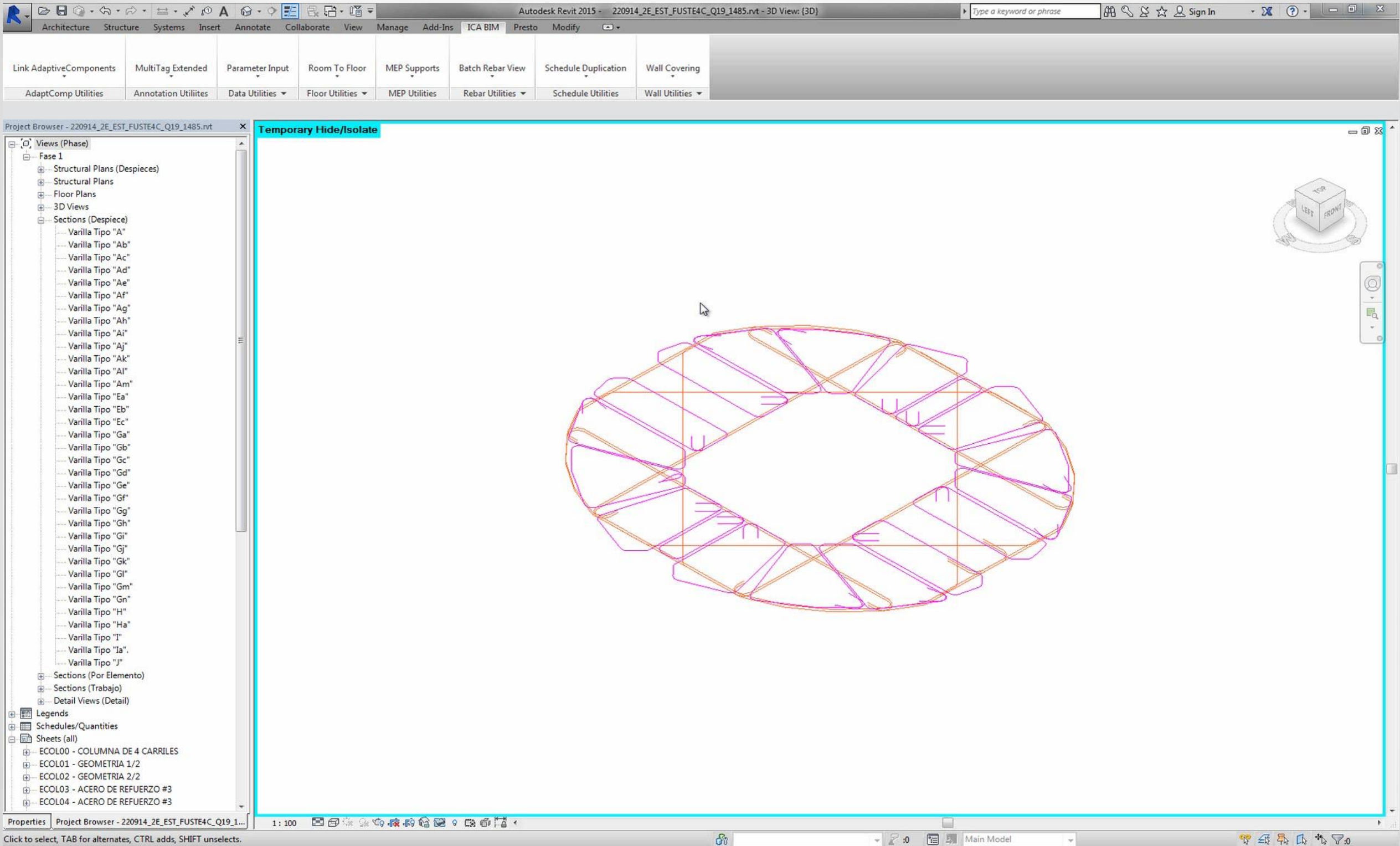


ELABORÓ:
Arq. Aarón De Marcos Estévez
MODELADOR

REVISÓ Y APROBÓ:
Ing. David Muñoz Vizuet
INGENIERIA

REVISÓ Y APROBÓ:
Ing. Fernando Mendez Galicia
SUPERINTENDENTE

Automatic View + Rebar Dimension App





INGENIEROS CIVILES ASOCIADOS

PREFABRICADOS Y TRANSPORTES S.A. DE C.V.

BUILDING INFORMATION MODELING



ESPECIFICACIONES GENERALES:

PLANOS DE REFERENCIA:

VPE-PGE-F5-CUPRI-III-048-P-00
VPE-EST-F5-CUPRI-III-013-P-00 (05CZ6B11_G1 DE 2)
VPE-EST-F5-CUPRI-III-014-P-00 (05CZ6B11_G2 DE 2)
VPE-EST-F5-CUPRI-III-015-P-01 (05CZ6B11_R1 DE 3)
VPE-EST-F5-CUPRI-III-016-P-01 (05CZ6B11_R2 DE 3)
VPE-EST-F5-CUPRI-III-017-P-01 (05CZ6B11_R3 DE 3)

MATERIALES:

CONCRETO F'c= 600 kg/cm2
EDAD= 28 días
AGREGADO MAX. = 1/2" (1.27cm)
ACERO DE REFUERZO Fy= 4200 Kg/cm2
ACERO EN PLACAS, ACCESORIOS METALICOS Y
TENSORES Fy= 2530kg/cm2
ACERO DE PRESFUERZO Fy= 19000kg/cm2 (torones de 3/8")
ACERO ESTRUCTURAL = A-50
POLIESTIRENO:

ELEMENTO:

CAPITEL_B6

HOJA:

ECP01

CROQUIS:

GEOMETRIA

PIEZA A FABRICAR:

B6_8.50m

PLANO DE REFERENCIA:

ESCALA: 1 : 50

COTAS: CM

FECHA:

02/09/14

13_Varilla Tipo F

Cold Igo Proy ecto	Clave	Modelo	Cant idad	Longitu d Unitaria	Suma Longitu dinal																												
						A	B	E	H		J	L2	L3	L3	P	rA	L2	L5	L6	L9	L12	L15	L18	L19	L22	L25	L28	L31					
F	M1_F11	#05	2	908.72	18.17 m	64.01	25.06	22.91	23.83	25.45	126.73	22.17	25.89	23.83	22.91	25.83	99.78	25.83	22.91	23.83	25.45	127.14	25.45	23.83	22.91	24.71	64.45						
F	M1_F12	#05	2	907.16	18.14 m	63.98	25.07	23.03	23.74	25.42	125.88	22.16	25.97	23.74	23.03	25.82	99.75	25.82	23.03	23.74	25.42	126.40	25.42	23.74	23.03	24.73	64.41						
F	M1_F13	#05	2	905.60	18.11 m	63.96	25.08	23.15	23.65	25.39	125.14	22.16	25.94	23.65	23.15	25.82	99.73	25.82	23.15	23.65	25.39	125.65	25.39	23.65	23.15	24.75	64.38						
F	M1_F14	#05	2	904.04	18.08 m	63.94	25.08	23.27	23.57	25.35	124.39	22.15	25.90	23.57	23.27	25.81	99.70	25.81	23.27	23.57	25.36	124.90	25.36	23.57	23.27	24.77	64.35						
F	M1_F15	#05	2	902.47	18.05 m	63.92	25.09	23.38	23.48	25.32	123.64	22.14	25.87	23.48	23.38	25.81	99.68	25.81	23.38	23.48	25.32	124.15	25.32	23.48	23.38	24.78	64.31						
F	M1_F16	#05	2	900.91	18.02 m	63.89	25.10	23.50	23.39	25.29	122.90	22.13	25.83	23.39	23.50	25.80	99.65	25.80	23.50	23.39	25.29	123.40	25.29	23.39	23.50	24.80	64.28						
F	M1_F17	#05	2	899.34	17.99 m	63.87	25.11	23.62	23.30	25.26	122.15	22.13	25.80	23.30	23.62	25.79	99.62	25.79	23.62	23.30	25.26	122.65	25.26	23.30	23.62	24.82	64.25						
F	M1_F18	#05	2	897.78	17.96 m	63.85	25.12	23.74	23.22	25.23	121.40	22.12	25.77	23.22	23.74	25.79	99.60	25.79	23.74	23.22	25.23	121.90	25.23	23.22	23.74	24.83	64.21						
F	M1_F19	#05	2	896.22	17.92 m	63.83	25.13	23.86	23.13	25.20	120.66	22.11	25.73	23.13	23.86	25.78	99.57	25.78	23.86	23.13	25.20	121.15	25.20	23.13	23.86	24.85	64.18						
F	M1_F20	#05	2	894.65	17.89 m	63.81	25.13	23.98	23.04	25.17	119.91	22.10	25.70	23.04	23.98	25.78	99.55	25.78	23.98	23.04	25.17	120.41	25.17	23.04	23.98	24.87	64.15						
F	M1_F21	#05	2	893.09	17.86 m	63.78	25.14	24.10	22.95	25.14	119.16	22.10	25.67	22.95	24.10	25.77	99.52	25.77	24.10	22.95	25.14	119.66	25.14	22.95	24.10	24.88	64.11						
F	M1_F22	#05	2	891.52	17.83 m	63.76	25.15	24.22	22.86	25.11	118.42	22.09	25.63	22.86	24.22	25.77	99.50	25.77	24.22	22.86	25.11	118.91	25.11	22.86	24.22	24.90	64.08						
F	M1_F23	#05	2	889.96	17.80 m	63.74	25.16	24.34	22.78	25.07	117.67	22.08	25.60	22.78	24.34	25.76	99.47	25.76	24.34	22.78	25.07	118.16	25.07	22.78	24.34	24.92	64.05						
F	M1_F24	#05	2	888.40	17.77 m	63.72	25.17	24.46	22.69	25.04	116.92	22.07	25.57	22.69	24.46	25.76	99.44	25.76	24.46	22.69	25.04	117.41	25.04	22.69	24.46	24.93	64.02						
F	M1_F25	#05	2	886.83	17.74 m	63.69	25.18	24.58	22.60	25.01	116.18	22.07	25.53	22.60	24.58	25.75	99.42	25.75	24.58	22.60	25.01	116.66	25.01	22.60	24.58	24.95	63.98						
F	M1_F26	#05	2	885.27	17.71 m	63.67	25.19	24.70	22.51	24.98	115.43	22.06	25.50	22.51	24.70	25.74	99.39	25.74	24.70	22.51	24.98	115.91	24.98	22.51	24.70	24.97	63.95						
F	M1_F27	#05	2	883.71	17.67 m	63.65	25.19	24.82	22.43	24.95	114.68	22.05	25.47	22.43	24.82	25.74	99.37	25.74	24.82	22.43	24.95	115.16	24.95	22.43	24.82	24.98	63.92						
F	M1_F28	#05	2	882.14	17.64 m	63.63	25.20	24.94	22.34	24.92	113.94	22.04	25.43	22.34	24.94	25.73	99.34	25.73	24.94	22.34	24.92	114.41	24.92	22.34	24.94	25.00	63.88						
F	M1_F29	#05	2	880.58	17.61 m	63.61	25.21	25.05	22.25	24.89	113.19	22.04	25.40	22.25	25.05	25.73	99.31	25.73	25.05	22.25	24.89	113.67	24.89	22.25	25.05	25.02	63.85						
F	M1_F30	#05	2	879.02	17.58 m	63.58	25.22	25.17	22.16	24.86	112.44	22.03	25.36	22.16	25.17	25.72	99.29	25.72	25.17	22.16	24.86	112.92	24.86	22.16	25.17	25.03	63.82						
F	M1_F31	#05	2	877.46	17.55 m	63.56	25.23	25.29	22.08	24.82	111.70	22.02	25.33	22.08	25.29	25.72	99.26	25.72	25.29	22.08	24.82	112.17	24.82	22.08	25.29	25.05	63.78						
F	M1_F32	#05	2	875.90	17.51 m	63.53	25.24	25.44	21.97	24.79	110.96	22.01	25.29	21.97	25.44	25.71	99.23	25.71	25.44	21.97	24.79	111.23	24.79	21.97	25.44	25.07	63.74						
F	M1_F33	#05	2	873.55	17.47 m	63.51	25.25	25.59	21.86	24.75	109.83	22.00	25.25	21.86	25.59	25.70	99.20	25.70	25.59	21.86	24.75	110.30	24.75	21.86	25.59	25.09	63.70						
F	M1_F34	#05	2	871.60	17.43 m	63.48	25.26	25.74	21.75	24.71	108.90	21.99	25.21	21.75	25.74	25.70	99.17	25.70	25.74	21.75	24.71	109.36	24.71	21.75	25.74	25.11	63.66						
F	M1_F35	#05	2	869.64	17.39 m	63.45	25.27	25.89	21.64	24.67	107.96	21.98	25.16	21.64	25.89	25.69	99.13	25.69	25.89	21.64	24.67	108.42	24.67	21.64	25.89	25.13	63.62						
F	M1_F36	#05	2	867.69	17.35 m	63.42	25.28	26.04	21.53	24.63	107.03	21.97	25.12	21.53	26.04	25.68	99.10	25.68	26.04	21.53	24.63	107.49	24.63	21.53	26.04	25.16	63.58						
F	M1_F37	#05	2	865.74	17.31 m	63.39	25.29	26.19	21.42	24.59	106.10	21.97	25.08	21.42	26.19	25.68	99.07	25.68	26.19	21.42	24.59	106.55	24.59	21.42	26.19	25.18	63.53						
F	M1_F38	#05	2	863.78	17.28 m	63.37	25.30	26.34	21.31	24.55	105.16	21.96	25.04	21.31	26.34	25.67	99.04	25.67	26.34	21.31	24.55	105.62	24.55	21.31	26.34	25.20	63.49						
F	M1_F39	#05	2	861.83	17.24 m	63.34	25.31	26.49	21.20	24.51	104.23	21.95	25.00	21.20	26.49	25.66	99.00	25.66	26.49	21.20	24.51	104.68	24.51	21.20	26.49	25.22	63.45						
F	M1_F40	#05	2	859.23	17.18 m	63.30	25.33	26.69	21.06	24.46	102.99	21.93	24.94	21.06	26.69	25.65	98.96	25.65	26.69	21.06	24.46	103.43	24.46	21.06	26.69	25.25	63.40						
F	M1_F41	#05	2	856.62	17.13 m	63.26	25.34	26.89	20.91	24.41	101.74	21.92	24.88	20.91	26.89	25.64	98.92	25.64	26.89	20.91	24.41	102.18	24.41	20.91	26.89	25.27	63.34						
F	M2_F42	#05	2	854.02	17.08 m	63.23	25.35	27.09	20.76	24.36	100.50	21.91	24.83	20.76	27.09	25.63	98.88	25.63	27.09	20.76	24.36	100.94	24.36	20.76	27.09	25.30	63.28						
F	M2_F43	#05	2	851.42	17.03 m	63.19	25.37	27.29	20.62	24.30	99.25	21.90	24.77	20.62	27.29	25.62	98.83	25.62	27.29	20.62	24.31	99.69	24.31	20.62	27.29	25.33	63.23						
F	M2_F44	#05	2	848.82	16.98 m	63.15	25.38	27.49	20.47	24.25	98.01	21.88	24.72	20.47	27.49	25.62	98.79	25.62	27.49	20.47	24.25	98.44	24.25	20.47	27.49	25.36	63.17						
F	M2_F45	#05	2	846.21	16.92 m	63.12	25.40	27.69	20.33	24.20	96.76	21.87	24.66	20.33	27.69	25.61	98.75	25.61	27.69	20.33	24.20	97.19	24.20	20.33	27.69	25.38	63.12						
F	M2_F46	#05	2	843.61	16.87 m	63.08	25.41	27.88	20.18	24.15	95.52	21.86	24.61	20.18	27.88	25.60	98.70	25.60	27.88	20.18	24.15	95.94	24.15	20.18	27.88	25.41	63.06						
F	M3_F47	#05	2	841.01	16.82 m	63.04	25.42	28.08	20.03	24.10	94.28	21.84	24.55	20.03	28.08	25.59	98.66	25.59	28.08	20.03	24.10	94.70	24.10	20.03	28.08	25.44	63.01						
F	M3_F48	#05	2	838.41	16.77 m	63.00	25.44	28.28	19.89	24.04	93.03	21.83	24.49	19.89	28.28	25.58	98.62	25.58	28.28	19.89	24.05	93.45	24.05	19.89	28.28	25.47	62.95						
F	M3_F49	#05	2	835.81	16.72 m	62.97	25.45	28.48	19.74	23.99	91.79	21.82	24.44	19.74	28.48	25.57	98.57	25.57	28.48	19.74	23.99	92.20	23.99	19.74	28.48	25.50	62.90						
F	M3_F50	#05	2	833.20	16.66 m	62.93	25.47	28.68	19.60	23.94	90.54	21.81	24.38	19.60	28.68	25.56	98.53	25.56	28.68	19.60	23.94	90.95	23.94	19.60	28.68	25.52	62.84						
F	M3_F51	#05	2	822.79	16.46 m	62.78	25.52	29.48	19.01	23.73	85.63	21.76																					



INGENIEROS CIVILES ASOCIADOS
PREFABRICADOS Y TRANSPORTES S.A. DE C.V.
BUILDING INFORMATION MODELING



ESPECIFICACIONES GENERALES:

PLANOS DE REFERENCIA:

VPE-PGE-F5-CUPRI-III-048-P-00
VPE-EST-F5-CUPRI-III-013-P-00 (05CZ6B11_G1 DE 2)
VPE-EST-F5-CUPRI-III-014-P-00 (05CZ6B11_G2 DE 2)
VPE-EST-F5-CUPRI-III-015-P-01 (05CZ6B11_R1 DE 3)
VPE-EST-F5-CUPRI-III-016-P-01 (05CZ6B11_R2 DE 3)
VPE-EST-F5-CUPRI-III-017-P-01 (05CZ6B11_R3 DE 3)

MATERIALES:

CONCRETO $F'c = 600 \text{ kg/cm}^2$
EDAD=
AGREGADO MAX. = $1/2"$ (1.27cm)
ACERO DE REFUERZO $F_y = 4200 \text{ Kg/cm}^2$
ACERO EN PLACAS, ACCESORIOS METALICOS Y
TENSORES $F_y = 2530 \text{ kg/cm}^2$
ACERO DE PRESFUERZO $F_y = 19000 \text{ kg/cm}^2$ (torones de $3/8"$)
ACERO ESTRUCTURAL = A-50
POLIESTIRENO:

ELEMENTO:
CAPITEL_B6

HOJA:
ECP01

CROQUIS:
GEOMETRIA

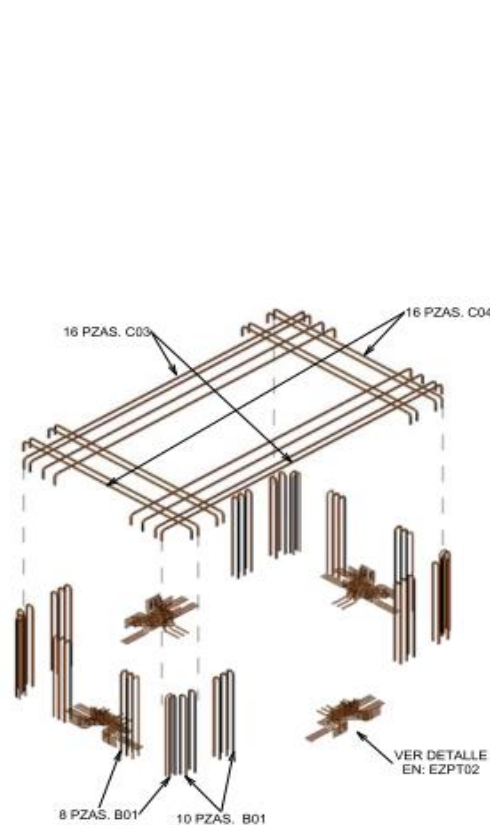
PIEZA A FABRICAR:
B6_8.50m

PLANO DE REFERENCIA:

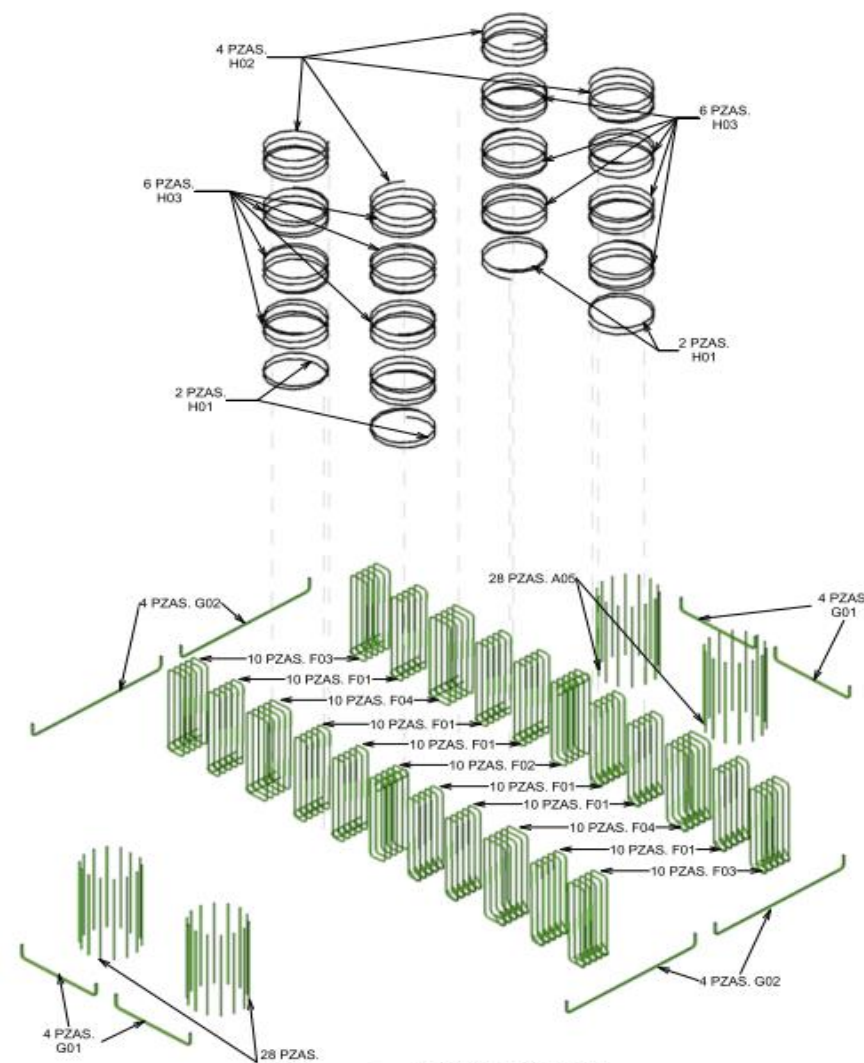
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COTAS: CM

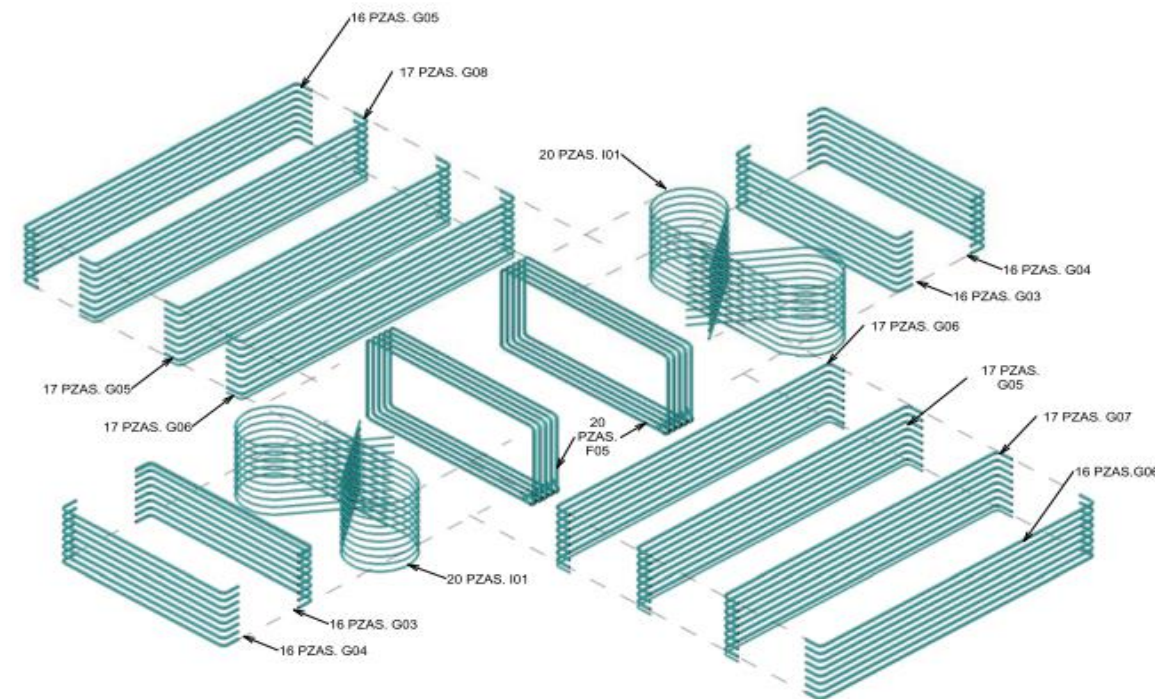
FECHA:
02/09/14



① ENSAMBLE #4



② ENSAMBLE #5



③ ENSAMBLE #6



ELABORÓ:

Arq. Aarón De Marcos Estévez
MODELADOR

REVISÓ Y APROBÓ:

Ing. David Muñoz Vizuet
INGENIERIA

REVISÓ Y APROBÓ:

Ing. Fernando Mendez Galicia
SUPERINTENDENTE





PLANOS DE REFERENCIA:

VPE-PGE-F5-CUPRI-III-048-P-00
VPE-EST-F5-CUPRI-III-013-P-00 (05C26B11_G1 DE 2)
VPE-EST-F5-CUPRI-III-014-P-00 (05C26B11_G2 DE 2)
VPE-EST-F5-CUPRI-III-015-P-01 (05C26B11_R1 DE 3)
VPE-EST-F5-CUPRI-III-016-P-01 (05C26B11_R2 DE 3)
VPE-EST-F5-CUPRI-III-017-P-01 (05C26B11_R3 DE 3)

MATERIALES:

CONCRETO F'c= 600 kg/cm2
EDAD= 28 dias
AGREGADO MAX. = 1/2" (1.27cm)
ACERO DE REFUERZO Fy= 4200 Kg/cm2
ACERO EN PLACAS, ACCESORIOS METALICOS Y
TENSORES Fy= 2530kg/cm2
ACERO DE PRESFUERZO Fy= 19000kg/cm2 (torones de
3/8")
ACERO ESTRUCTURAL = A-50
POLIESTIRENO:

ELEMENTO:
CAPITEL B6

HOJA:
ECP01

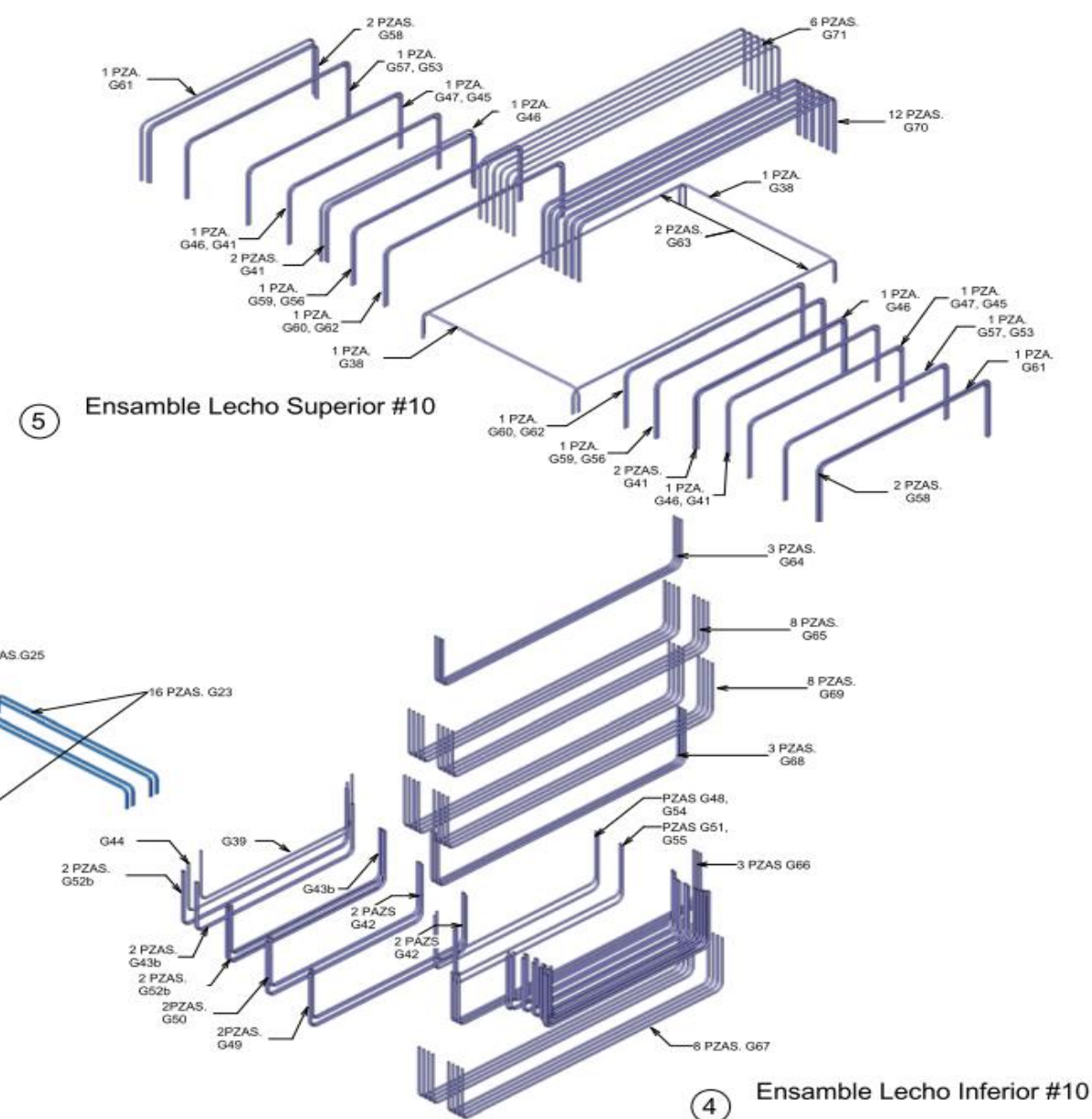
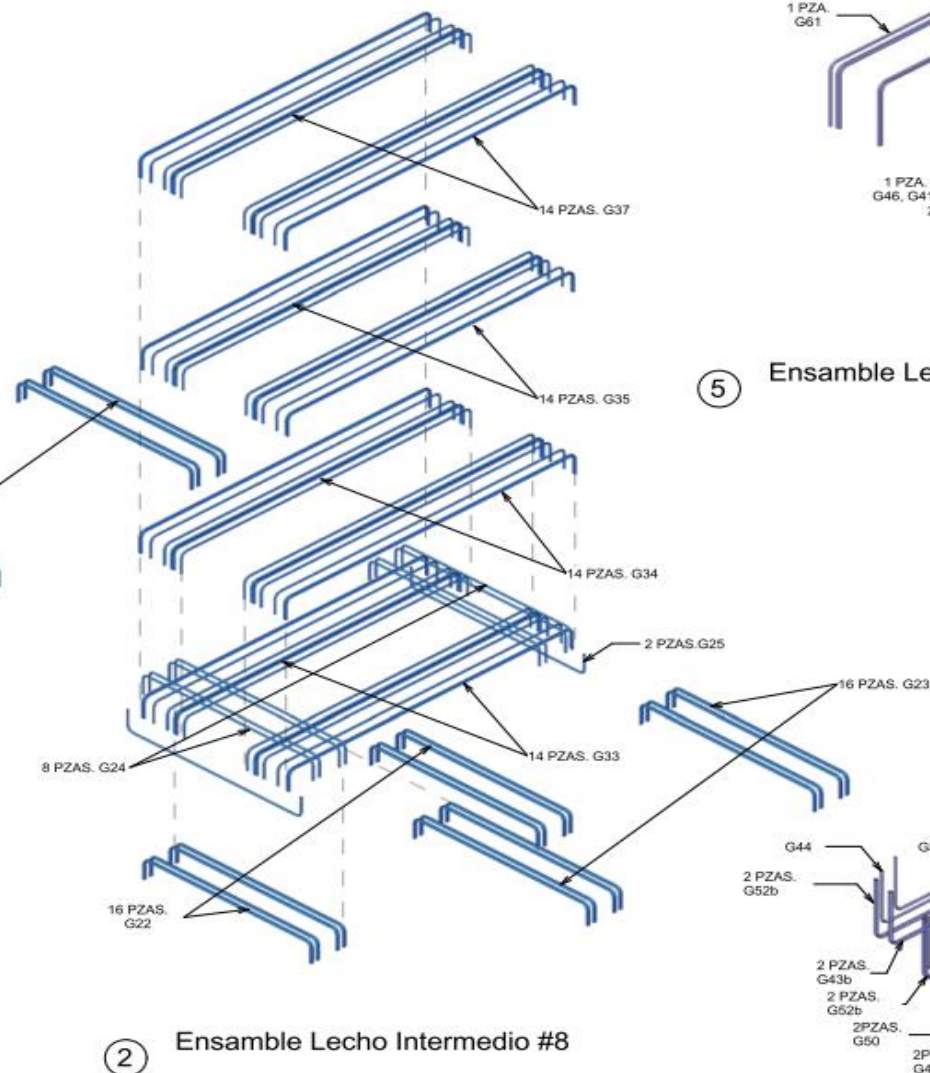
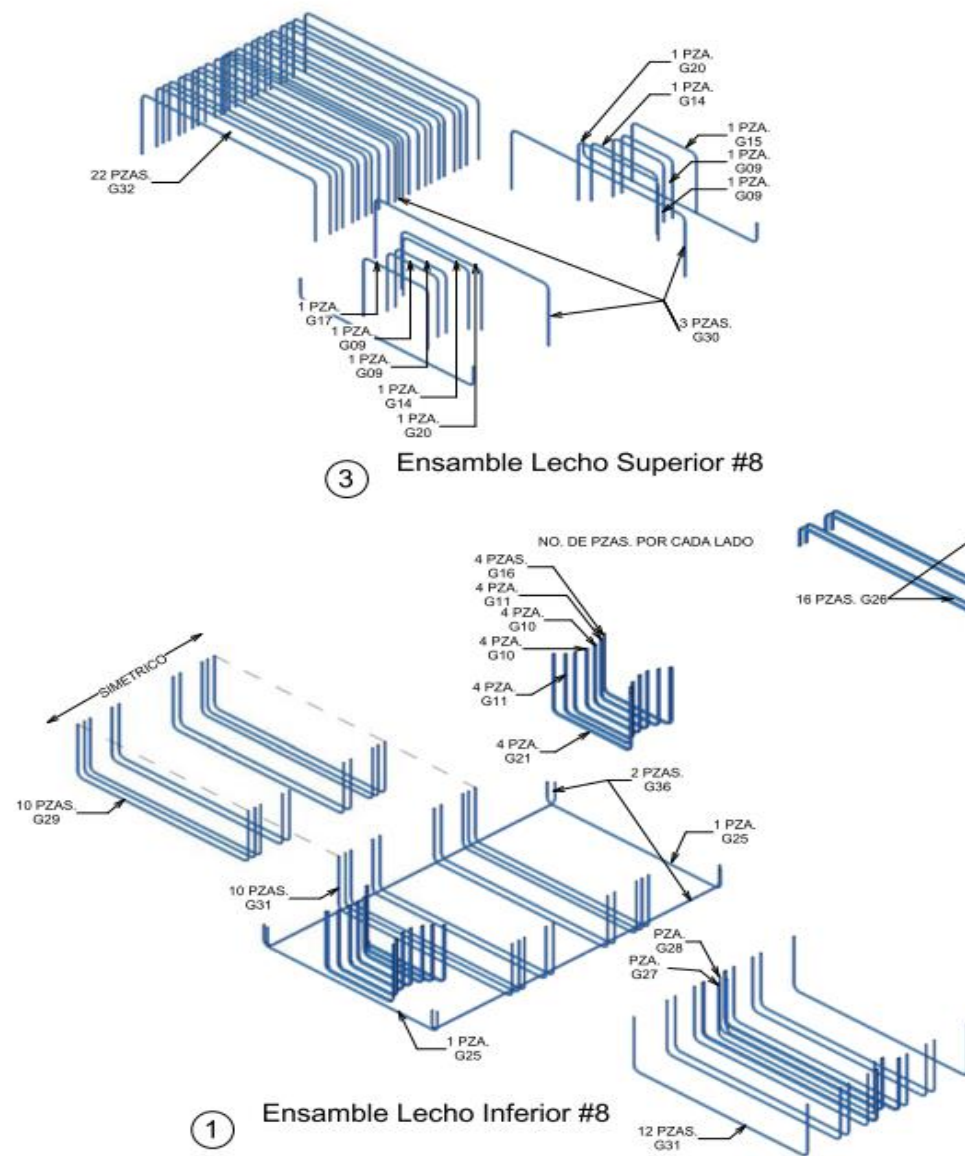
CROQUIS:
GEOMETRIA

PIEZA A FABRICAR:
B6 8.50m

PLANO DE REFERENCIA

ESCALA: 1 : 50

FECHA:
02/09/14





INGENIEROS CIVILES ASOCIADOS PREFABRICADOS Y TRANSPORTES S.A. DE C.V. BUILDING INFORMATION MODELING



ESPECIFICACIONES GENERALES:

PLANOS DE REFERENCIA:

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VPE-EST-F5-CUPRI-III-014-P-00 (05CZ6B11_G2 DE 2)
VPE-EST-F5-CUPRI-III-015-P-01 (05CZ6B11_R1 DE 3)
VPE-EST-F5-CUPRI-III-016-P-01 (05CZ6B11_R2 DE 3)
VPE-EST-F5-CUPRI-III-017-P-01 (05CZ6B11_R3 DE 3)

MATERIALES:

CONCRETO F'c= 600 kg/cm2
EDAD=
AGREGADO MAX. = 1/2" (1.27cm)
ACERO DE REFUERZO Fy= 4200 Kg/cm2
ACERO EN PLACAS, ACCESORIOS METALICOS Y
TENSORES Fy= 2530kg/cm2
ACERO DE PRESFUERZO Fy= 19000kg/cm2 (torones de 3/8")
ACERO ESTRUCTURAL = A-50
POLIESTIRENO:

ELEMENTO:
CAPITEL_B6

HOJA:
ECP01

CROQUIS:
GEOMETRIA

PIEZA A FABRICAR:
B6_8.50m

PLANO DE REFERENCIA:

ESCALA: 1 : 50

COTAS: CM

FECHA:
02/09/14

00_Resumen de Varillas por Diametro

Modelo	Diametro	Cantidad	Suma Longitudinal	PesoLineal	Peso Total
#04	16 mm	156	279.77 m	0.996 kg/m	279 kg
Fuste					
#04	16 mm	422	702.76 m	0.996 kg/m	700 kg
#05	20 mm	674	2.955.65 m	1.560 kg/m	4.611 kg
#06	20 mm	120	833.42 m	2.250 kg/m	1.875 kg
#08	30 mm	136	823.91 m	3.975 kg/m	3.275 kg
		1508	5.595.51 m		10.740 kg

01_Torones

Coldigo Proyecto	Descripción	Diametro (mm)	Cantidad	Suma Longitudinal	PesoLineal	Peso Total
Toron 3/8"						
	Toron 3/8"	0.95	340	3,405.56 m	0.432 kg/m	1,471 kg
			340	3,405.56 m		1,471 kg

02_Volumetria de Geometria Fuste

Pertenece a	Tipo	Volumen
Fuste	B6	23.75 m³ 23.75 m³

03_Volumetria Aligeramiento Poliestireno

Pertenece a	Tipo	Volumen
Aligeramiento		
Aligeramiento	B6	11.55 m³
1		11.55 m³

04_Tubo B.A.P.

Descripción	Diametro	Longitud (ml)
TUBERIA DE ACERO ESPECIFICACIÓN ASTM - A53 DE DIAM. 203 mm. (8") CÉDULA 40.	200 mm	150.5
TUBERIA DE ACERO ESPECIFICACIÓN ASTM - A53 DE DIAM. 203 mm. (8") CÉDULA 40.	200 mm	27.8
TUBERIA DE ACERO ESPECIFICACIÓN ASTM - A53 DE DIAM. 203 mm. (8") CÉDULA 40.	200 mm	99.6
TUBERIA DE ACERO ESPECIFICACIÓN ASTM - A53 DE DIAM. 203 mm. (8") CÉDULA 40.	200 mm	96.6
TUBERIA DE ACERO ESPECIFICACIÓN ASTM - A53 DE DIAM. 203 mm. (8") CÉDULA 40.	200 mm	98.8
TUBERIA DE ACERO ESPECIFICACIÓN ASTM - A53 DE DIAM. 203 mm. (8") CÉDULA 40.	200 mm	98.1
TUBERIA DE ACERO ESPECIFICACIÓN ASTM - A53 DE DIAM. 203 mm. (8") CÉDULA 40.	200 mm	844.2
		1215.5

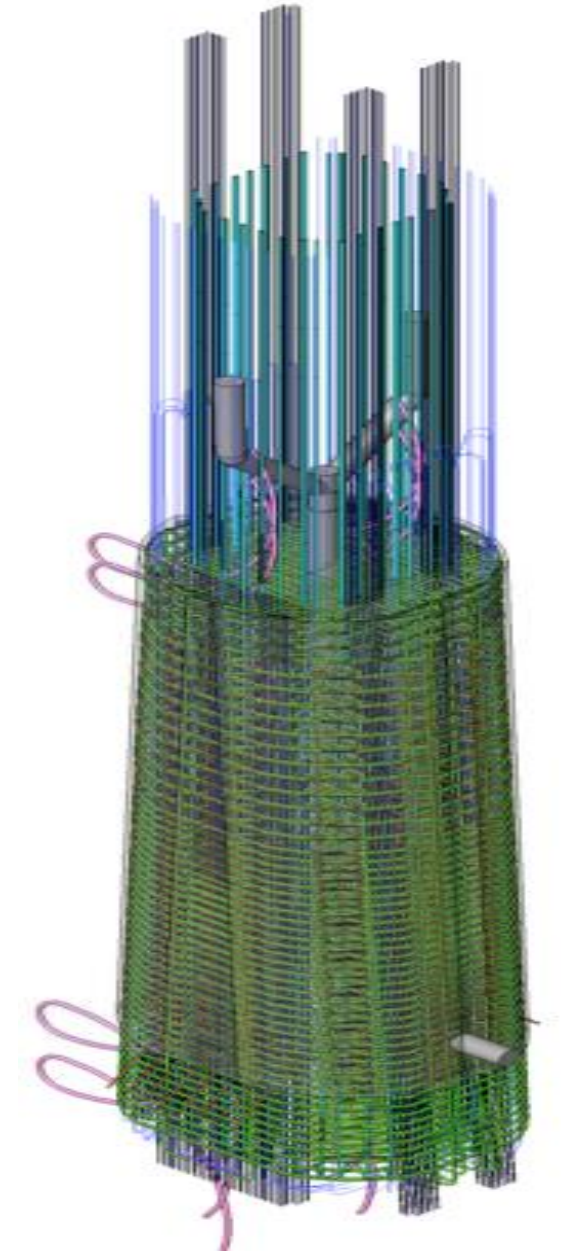
05_Accesorios Tuberia de Acero B.A.P.

Accesorio	Tipo	Pertenece a	Diametro	Pieza
Codo 45	Fierro Fundido	Fuste	200 mm-200 mm	1
"Y"	Fierro Fundido	Fuste	200 mm-200 mm-200 mm-200 mm	1
Codo 45	Fierro Fundido	Fuste	200 mm-200 mm	1
Codo 45	Fierro Fundido	Fuste	200 mm-200 mm	1
Codo 45	Fierro Fundido	Fuste	200 mm-200 mm	1
				5

06_Ganchos de Izaje

Coldigo Proyecto	Descripción	Diametro	Longitud Unitaria	Cantidad	Suma Longitudinal
GI	Cable de 1 1/2" tipo Boa con alma de acero fpu=7877kg/cm2	40 mm	7.19 m	8	57.52 m
				8	57.52 m

68 CONECTORES PARA VARILLA DEL #8



ELABORÓ:

Arq. Aarón De Marcos Estévez
MODELADOR

REVISÓ Y APROBÓ:

Ing. David Muñoz Vizuet
INGENIERIA

REVISÓ Y APROBÓ:

Ing. Fernando Mendez Galicia
SUPERINTENDENTE



AUTODESK UNIVERSITY 2014





INGENIEROS CIVILES ASOCIADOS
PREFABRICADOS Y TRANSPORTES S.A. DE C.V.
BUILDING INFORMATION MODELING



ESPECIFICACIONES GENERALES:

PLANOS DE REFERENCIA:

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VPE-EST-F5-CUPRI-III-015-P-01 (05CZ6B11_R1 DE 3)
VPE-EST-F5-CUPRI-III-016-P-01 (05CZ6B11_R2 DE 3)
VPE-EST-F5-CUPRI-III-017-P-01 (05CZ6B11_R3 DE 3)

MATERIALES:

CONCRETO $F'c = 600 \text{ kg/cm}^2$
EDAD =
AGREGADO MAX. = $1/2"$ (1.27cm)
ACERO DE REFUERZO $F_y = 4200 \text{ Kg/cm}^2$
ACERO EN PLACAS, ACCESORIOS METALICOS Y
TENSORES $F_y = 2530 \text{ kg/cm}^2$
ACERO DE PRESFUERZO $F_y = 19000 \text{ kg/cm}^2$ (torones de $3/8"$)
ACERO ESTRUCTURAL = A-50
POLIESTIRENO:

ELEMENTO:
CAPITEL_B6

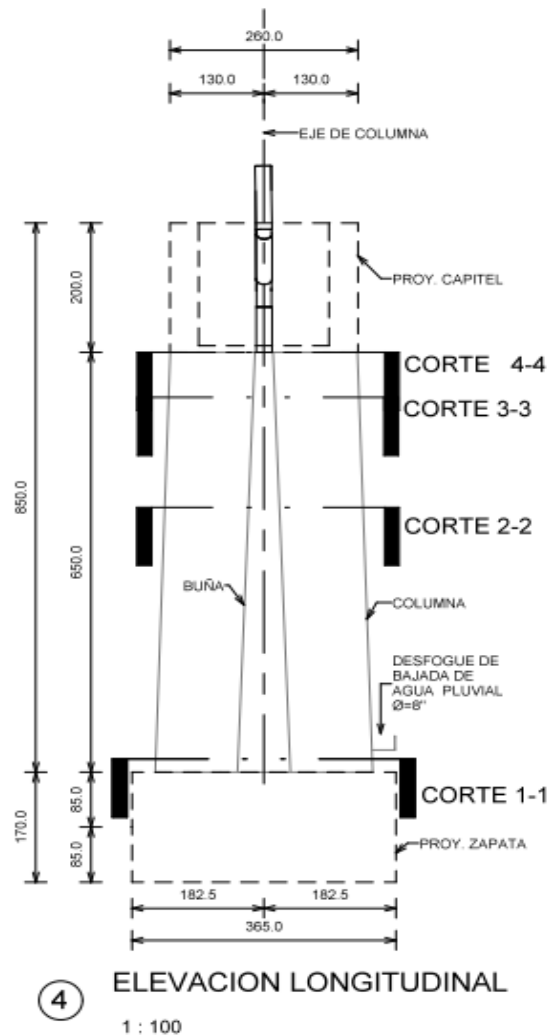
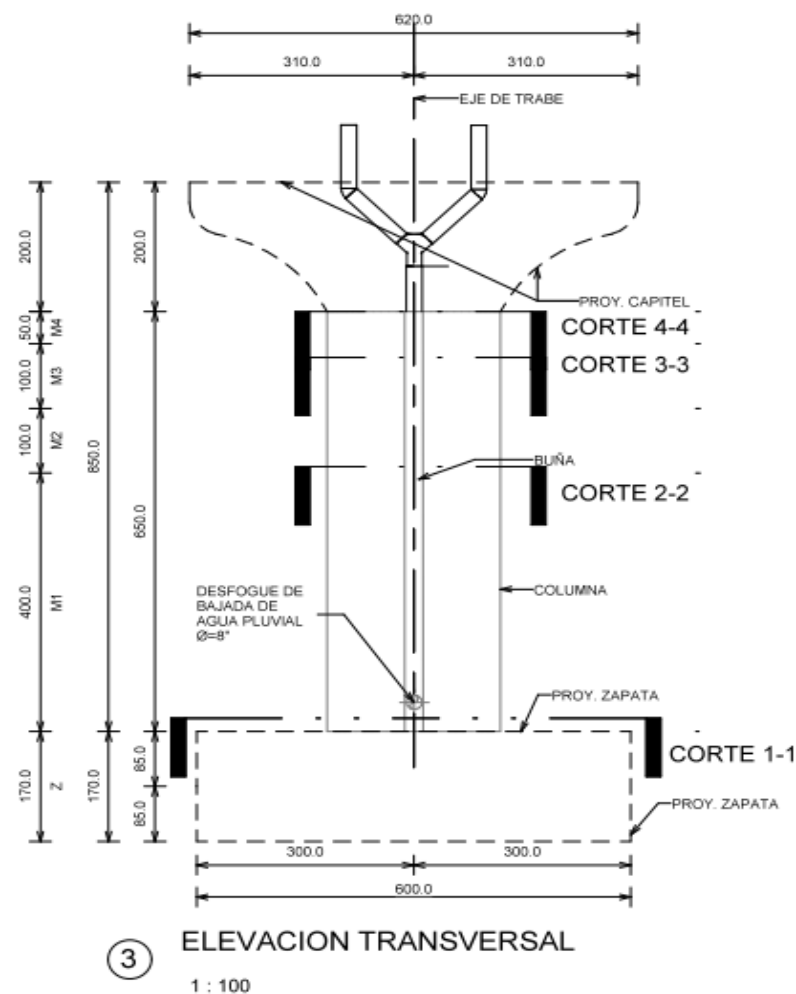
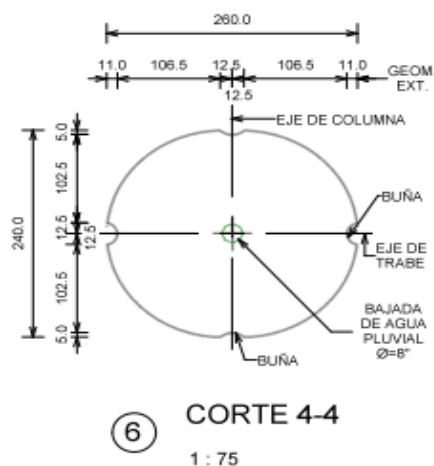
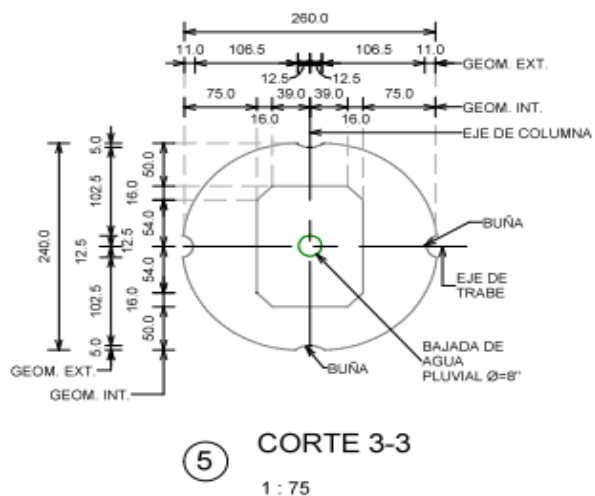
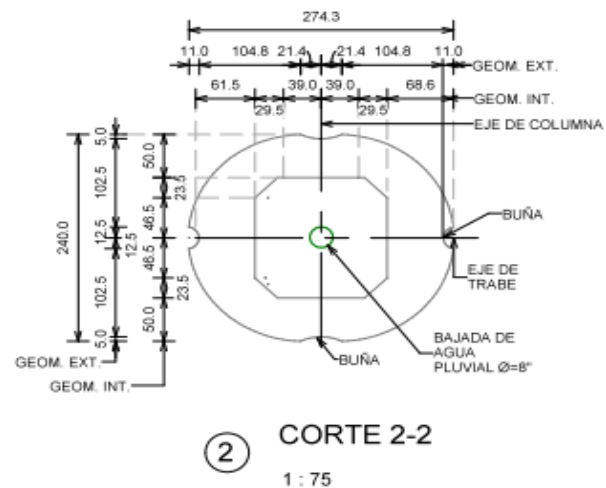
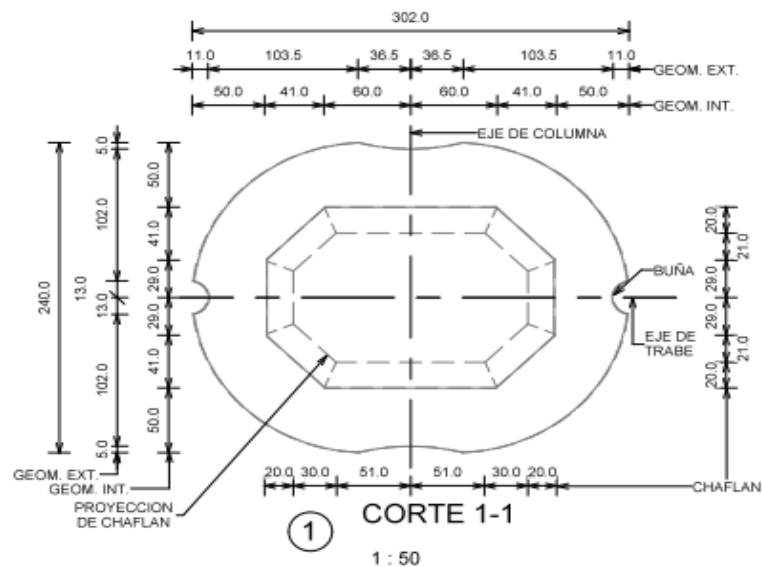
HOJA:
ECP01

CROQUIS:
GEOMETRIA

PIEZA A FABRICAR:
B6_8.50m

PLANO DE REFERENCIA:
ESCALA: 1 : 50

FECHA:
02/09/14



ELABORÓ:

Arq. Aarón De Marcos Estévez
MODELADOR

REVISÓ Y APROBÓ:

Ing. David Muñoz Vizuet
INGENIERIA

REVISÓ Y APROBÓ:

Ing. Fernando Mendez Galicia
SUPERINTENDENTE



INGENIEROS CIVILES ASOCIADOS
PREFABRICADOS Y TRANSPORTES S.A. DE C.V.
BUILDING INFORMATION MODELING



ESPECIFICACIONES GENERALES:

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VPE-EST-F5-CUPRI-III-016-P-01 (05C26B11_R2 DE 3)
VPE-EST-F5-CUPRI-III-017-P-01 (05C26B11_R3 DE 3)

MATERIALES:

CONCRETO $F'c = 600 \text{ kg/cm}^2$
EDAD =
AGREGADO MAX. = $1/2"$ (1.27cm)
ACERO DE REFUERZO $F_y = 4200 \text{ Kg/cm}^2$
ACERO EN PLACAS, ACCESORIOS METALICOS Y
TENSORES $F_y = 2530 \text{ kg/cm}^2$
ACERO DE PRESFUERZO $F_y = 19000 \text{ kg/cm}^2$ (torones de $3/8"$)
ACERO ESTRUCTURAL = A-50
POLIESTIRENO:

ELEMENTO:
CAPITEL_B6

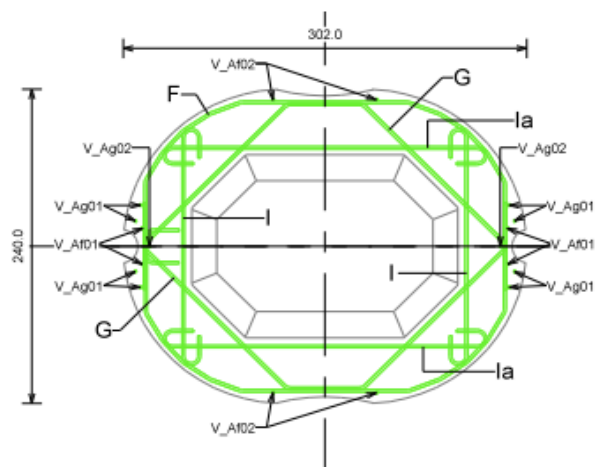
HOJA:
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CROQUIS:
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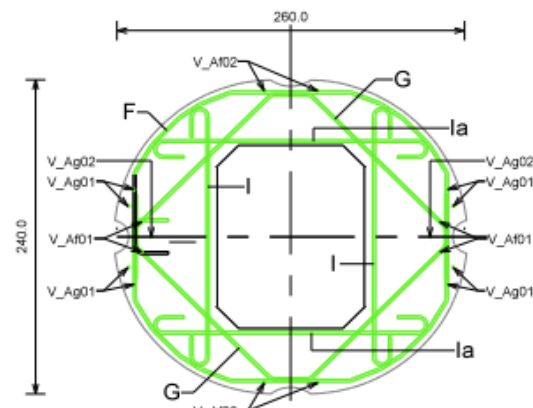
PIEZA A FABRICAR:
B6_8.50m

PLANO DE REFERENCIA:
ESCALA: 1 : 50 COTAS: CM

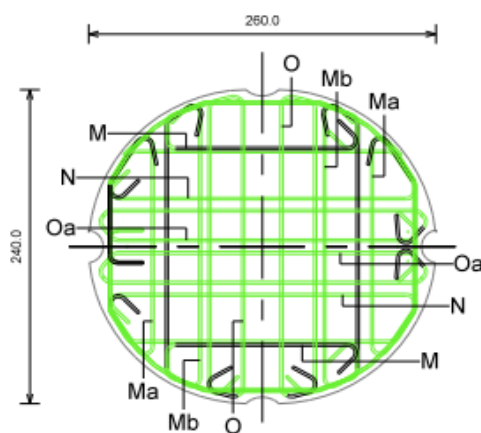
FECHA:
02/09/14



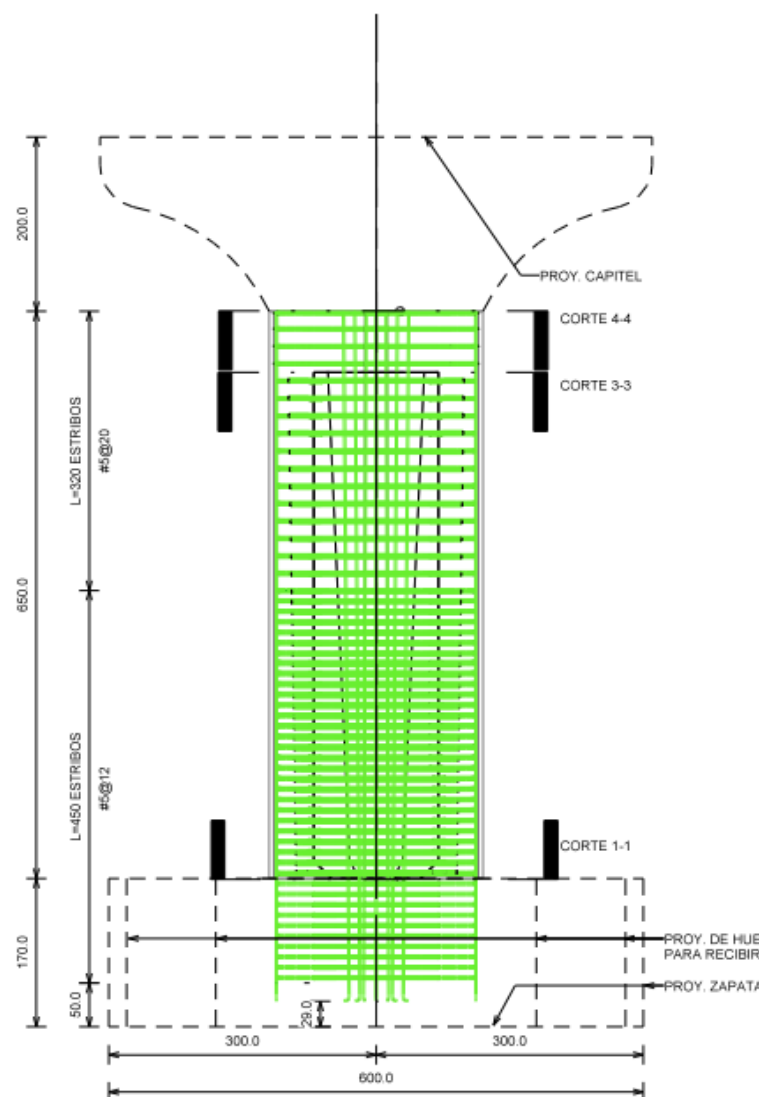
① REFUERZO CORTE 1-1 #5
1 : 50



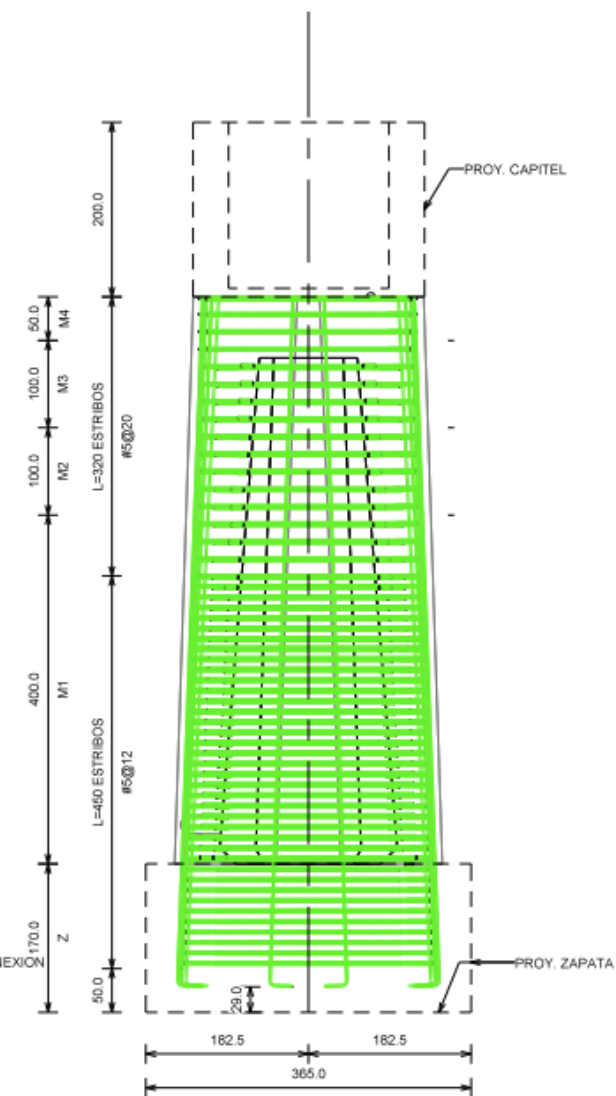
② REFUERZO CORTE 3-3 #5
1 : 50



⑤ REFUERZO CORTE 4-4 #5
1 : 50



③ REFUERZO TRANSVERSAL #5
1 : 75



④ REFUERZO LONGITUDINAL #5
1 : 75



ELABORÓ:

Arq. Aarón De Marcos Estévez
MODELADOR

REVISÓ Y APROBÓ:

Ing. David Muñoz Vizuet
INGENIERIA

REVISÓ Y APROBÓ:

Ing. Fernando Mendez Galicia
SUPERINTENDENTE





INGENIEROS CIVILES ASOCIADOS
PREFABRICADOS Y TRANSPORTES S.A. DE C.V.
BUILDING INFORMATION MODELING



ESPECIFICACIONES GENERALES:

PLANOS DE REFERENCIA:

VPE-PGE-F5-CUPRI-III-048-P-00
VPE-EST-F5-CUPRI-III-013-P-00 (05C26B11_G1 DE 2)
VPE-EST-F5-CUPRI-III-014-P-00 (05C26B11_G2 DE 2)
VPE-EST-F5-CUPRI-III-015-P-01 (05C26B11_R1 DE 3)
VPE-EST-F5-CUPRI-III-016-P-01 (05C26B11_R2 DE 3)
VPE-EST-F5-CUPRI-III-017-P-01 (05C26B11_R3 DE 3)

MATERIALES:

CONCRETO $F'c = 600 \text{ kg/cm}^2$
EDAD =
AGREGADO MAX. = 1/2" (1.27cm)
ACERO DE REFUERZO $F_y = 4200 \text{ Kg/cm}^2$
ACERO EN PLACAS, ACCESORIOS METALICOS Y
TENSORES $F_y = 2530 \text{ kg/cm}^2$
ACERO DE PRESFUERZO $F_y = 19000 \text{ kg/cm}^2$ (torones de 3/8")
ACERO ESTRUCTURAL = A-50
POLIESTIRENO:

ELEMENTO:
CAPITEL_B6

HOJA:
ECP01

CROQUIS:
GEOMETRIA

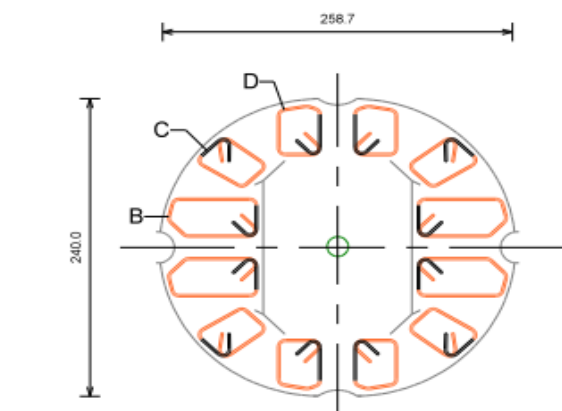
PIEZA A FABRICAR:
B6_8.50m

PLANO DE REFERENCIA:
ESCALA: 1 : 50

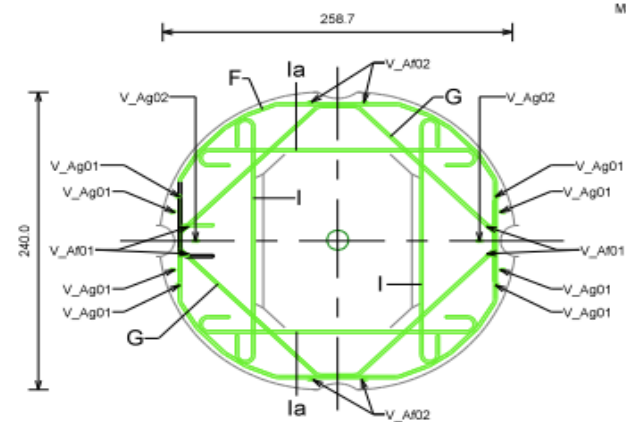
FECHA:
02/09/14

COTAS: CM

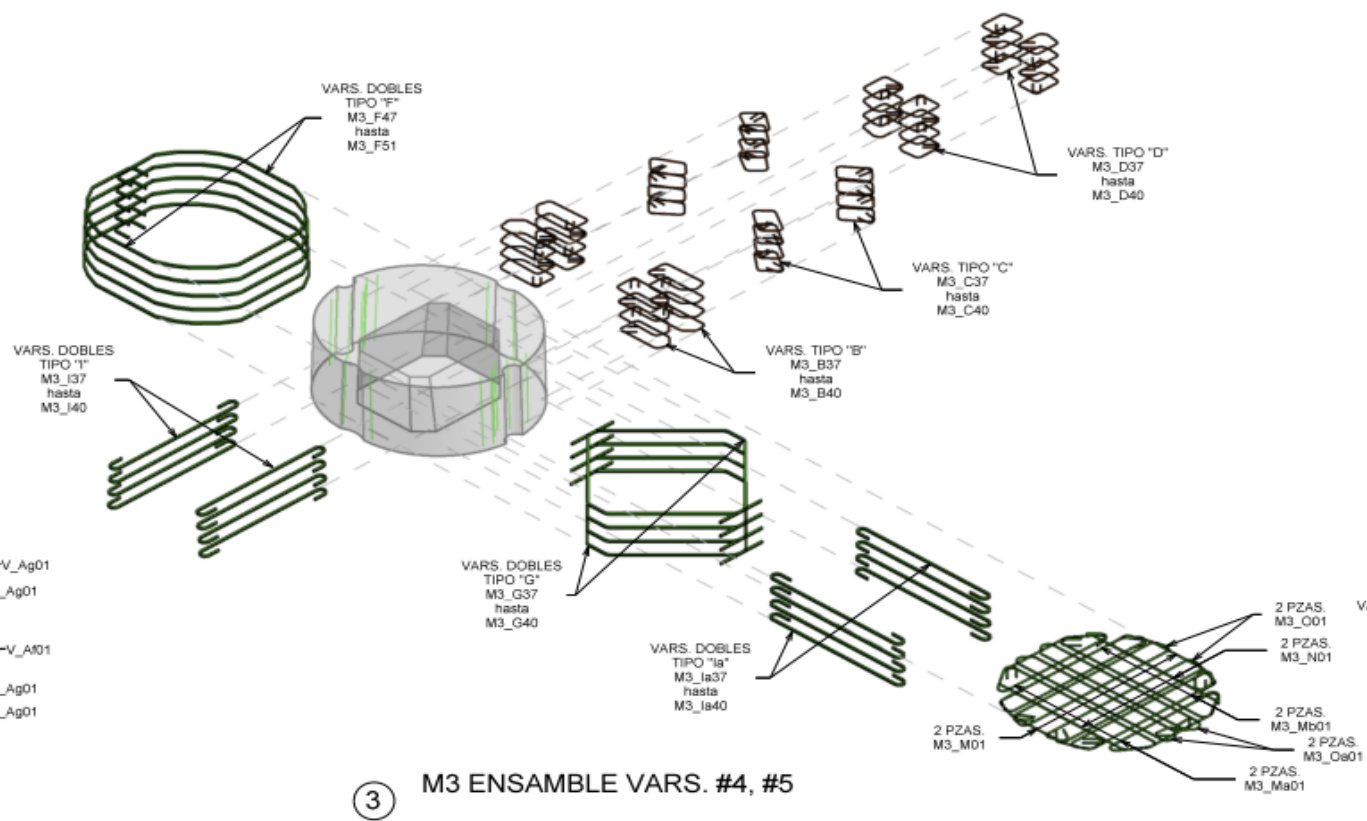
ARCO SUR INTERTRAMO 5



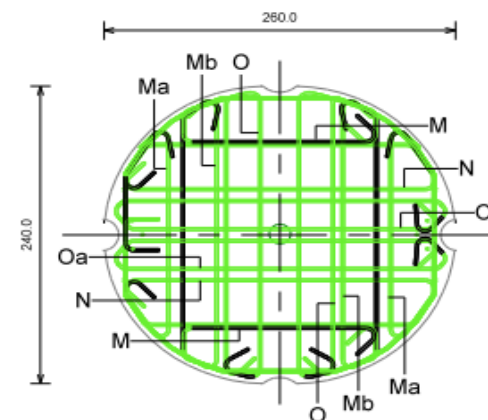
1 CORTE M3 VARS. #4
1 : 50



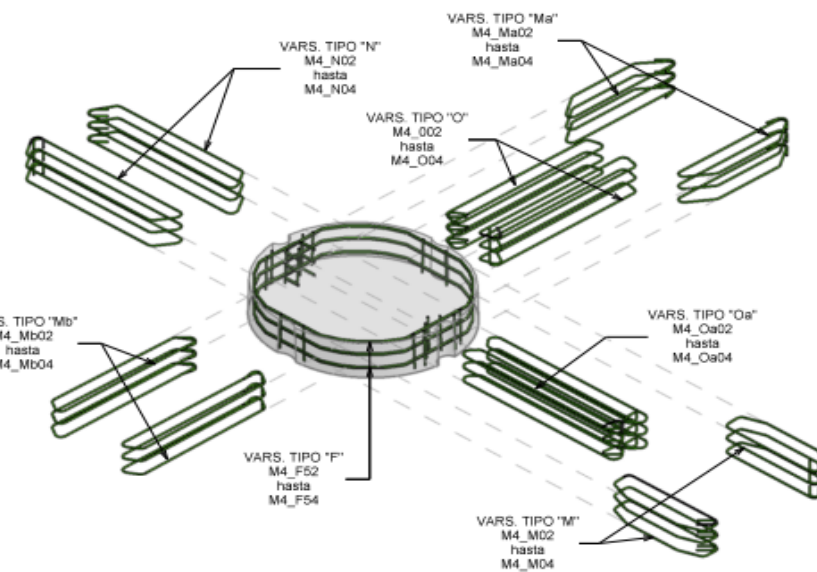
2 CORTE M3 VARS. #5
1 : 50



3 M3 ENSAMBLE VARS. #4, #5



4 CORTE M4 VARS. #5
1 : 50



5 M4 ENSAMBLE VARS. #5



ELABORÓ:

Arq. Aarón De Marcos Estévez
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REVISÓ Y APROBÓ:

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SUPERINTENDENTE





INGENIEROS CIVILES ASOCIADOS
PREFABRICADOS Y TRANSPORTES S.A. DE C.V.
BUILDING INFORMATION MODELING



ESPECIFICACIONES GENERALES:

PLANOS DE REFERENCIA:

VPE-PGE-F5-CUPRI-III-048-P-00
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VPE-EST-F5-CUPRI-III-014-P-00 (05CZ6B11_G2 DE 2)
VPE-EST-F5-CUPRI-III-015-P-01 (05CZ6B11_R1 DE 3)
VPE-EST-F5-CUPRI-III-016-P-01 (05CZ6B11_R2 DE 3)
VPE-EST-F5-CUPRI-III-017-P-01 (05CZ6B11_R3 DE 3)

MATERIALES:

CONCRETO F'c= 600 kg/cm2
EDAD=
AGREGADO MAX. = 1/2" (1.27cm)
ACERO DE REFUERZO Fy= 4200 Kg/cm2
ACERO EN PLACAS, ACCESORIOS METALICOS Y
TENSORES Fy= 2530kg/cm2
ACERO DE PRESFUERZO Fy= 19000kg/cm2 (torones de 3/8")
ACERO ESTRUCTURAL = A-50
POLIESTIRENO:

ELEMENTO:
CAPITEL_B6

HOJA:
ECP01

CROQUIS:
GEOMETRIA

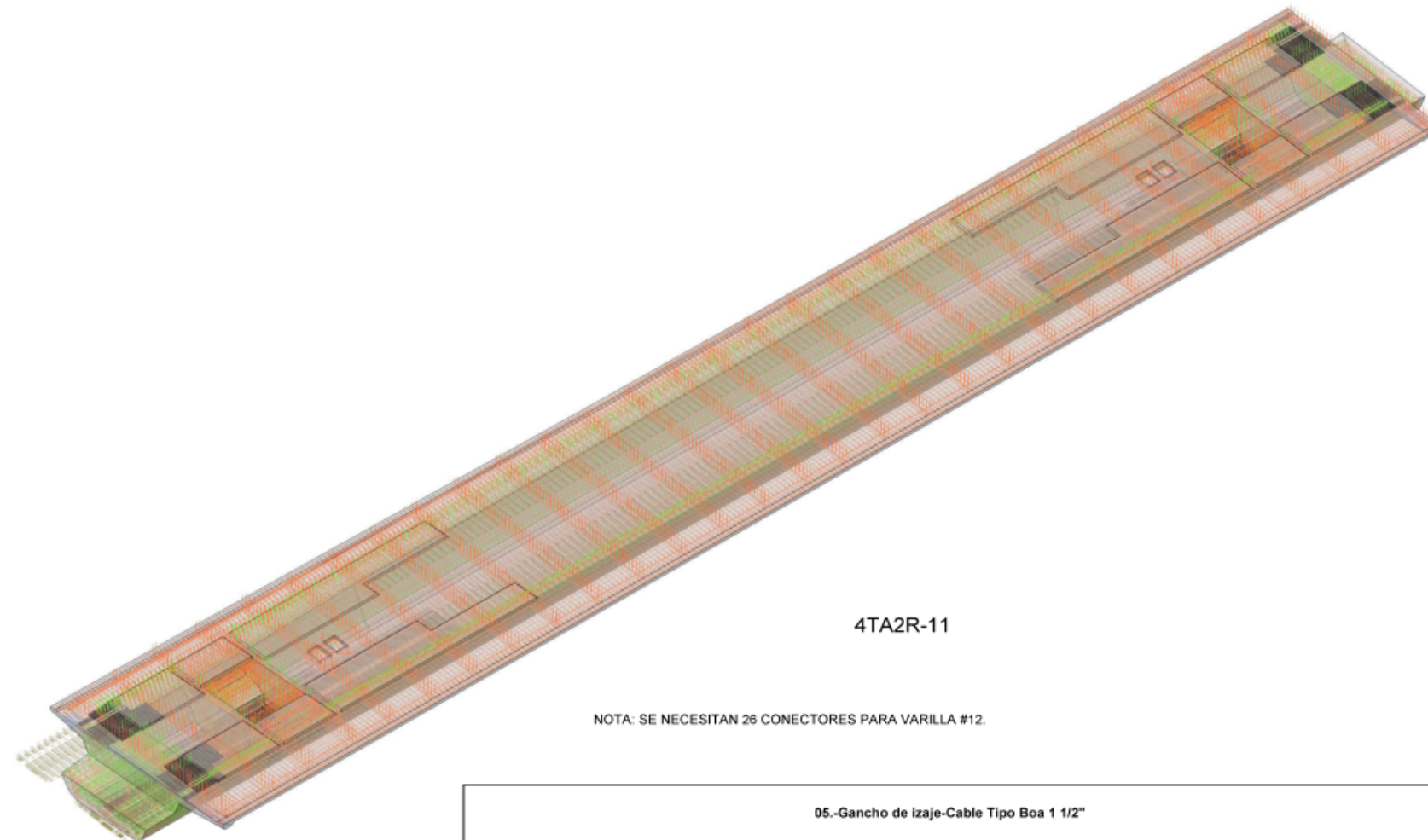
PIEZA A FABRICAR:
B6_8.50m

PLANO DE REFERENCIA:

ESCALA: 1 : 50

COTAS: CM

FECHA:
02/09/14



4TA2R-11

NOTA: SE NECESITAN 26 CONECTORES PARA VARILLA #12.

01_Volumen Geometrico

Pertenece a	No Pieza	Volumen
Trabe	4TA2R-11_V1	106.101 m³

02_Acero de refuerzo fy=4200 kg/cm2

Model	Cantidad	Suma Longitudinal	Peso Lineal	Peso Total
#04	7301	20,506.21 m		20424.181 kg
#05	1790	7,000.39 m		10920.611 kg
#06	60	136.31 m		306.706 kg
#08	16	100.96 m		401.316 kg
#12	58	506.84 m		4530.096 kg
	9225	28,250.71 m		36582.911 kg

03_Toron fpu=19000 kg/cm2

Clave	Diametro (mm)	Cantidad	Longitud Unitaria	Suma Longitudinal	PesoLineal	Peso Total
T001	1.27	196	4625.00	9,065.00 m	0.820 kg/m	7433.300 kg
		196		9,065.00 m		7433.300 kg

05.-Gancho de izaje-Cable Tipo Boa 1 1/2"

Description	Piezas	Longitud Unitaria	Suma Longitudinal	PesoLineal	Peso Total
Cable de 1 1/2" Tipo Boa	4.00	3.42 m	54.74 m	6.190 kg/m	338.836 kg
			54.74 m		338.836 kg

04_Poliducto para engrase

Clave	Cantidad	Description	Longitud Unitaria	Longitud Total
Toron N.1	128	Poliducto de PVC de 3/4" (19mm)	50.00 cm	6400.00 cm
Toron N.2	44	Poliducto de PVC de 3/4" (19mm)	100.00 cm	4400.00 cm
Toron N.3	64	Poliducto de PVC de 3/4" (19mm)	150.00 cm	9600.00 cm
Toron N.4	44	Poliducto de PVC de 3/4" (19mm)	700.00 cm	30800.00 cm
Toron N.5	16	Poliducto de PVC de 3/4" (19mm)	800.00 cm	12800.00 cm
Toron N.6	48	Poliducto de PVC de 3/4" (19mm)	1200.00 cm	57600.00 cm
	344			121600.00 cm



ELABORÓ:

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PREFABRICADOS Y TRANSPORTES S.A. DE C.V.
BUILDING INFORMATION MODELING



ESPECIFICACIONES GENERALES:

PLANOS DE REFERENCIA:

VPE-PGE-F5-CUPRI-III-048-P-00
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VPE-EST-F5-CUPRI-III-014-P-00 (05CZ6B11_G2 DE 2)
VPE-EST-F5-CUPRI-III-015-P-01 (05CZ6B11_R1 DE 3)
VPE-EST-F5-CUPRI-III-016-P-01 (05CZ6B11_R2 DE 3)
VPE-EST-F5-CUPRI-III-017-P-01 (05CZ6B11_R3 DE 3)

MATERIALES:

CONCRETO $F'c = 600 \text{ kg/cm}^2$
EDAD =
AGREGADO MAX. = $1/2"$ (1.27cm)
ACERO DE REFUERZO $F_y = 4200 \text{ Kg/cm}^2$
ACERO EN PLACAS, ACCESORIOS METALICOS Y
TENSORES $F_y = 2530 \text{ kg/cm}^2$
ACERO DE PRESFUERZO $F_y = 19000 \text{ kg/cm}^2$ (torones de $3/8"$)
ACERO ESTRUCTURAL = A-50
POLIESTIRENO:

ELEMENTO:
CAPITEL_B6

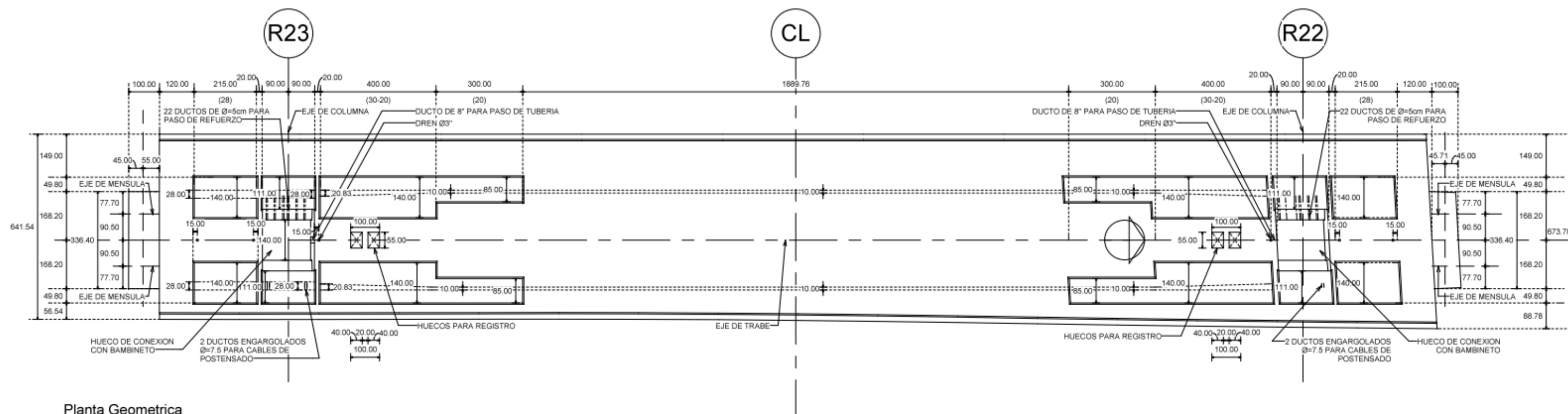
HOJA:
ECP01

CROQUIS:
GEOMETRIA

PIEZA A FABRICAR:
B6_8.50m

PLANO DE REFERENCIA:
ESCALA: 1 : 50
COTAS: CM

FECHA:
02/09/14



Planta Geometrica



ELABORÓ:

Arq. Aarón De Marcos Estévez
MODELADOR

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REVISÓ Y APROBÓ:

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INGENIEROS CIVILES ASOCIADOS
PREFABRICADOS Y TRANSPORTES S.A. DE C.V.
BUILDING INFORMATION MODELING



ESPECIFICACIONES GENERALES:

PLANOS DE REFERENCIA:

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VPE-EST-F5-CUPRI-III-013-P-00 (05C26B11_G1 DE 2)
VPE-EST-F5-CUPRI-III-014-P-00 (05C26B11_G2 DE 2)
VPE-EST-F5-CUPRI-III-015-P-01 (05C26B11_R1 DE 3)
VPE-EST-F5-CUPRI-III-016-P-01 (05C26B11_R2 DE 3)
VPE-EST-F5-CUPRI-III-017-P-01 (05C26B11_R3 DE 3)

MATERIALES:

CONCRETO $F'c = 600 \text{ kg/cm}^2$
EDAD =
AGREGADO MAX. = $1/2"$ (1.27cm)
ACERO DE REFUERZO $F_y = 4200 \text{ Kg/cm}^2$
ACERO EN PLACAS, ACCESORIOS METALICOS Y
TENSORES $F_y = 2530 \text{ kg/cm}^2$
ACERO DE PRESFUERZO $F_y = 19000 \text{ kg/cm}^2$ (torones de $3/8"$)
ACERO ESTRUCTURAL = A-50
POLIESTIRENO:

ELEMENTO:

CAPITEL_B6

HOJA:

ECP01

CROQUIS:

GEOMETRIA

PIEZA A FABRICAR:

B6_8.50m

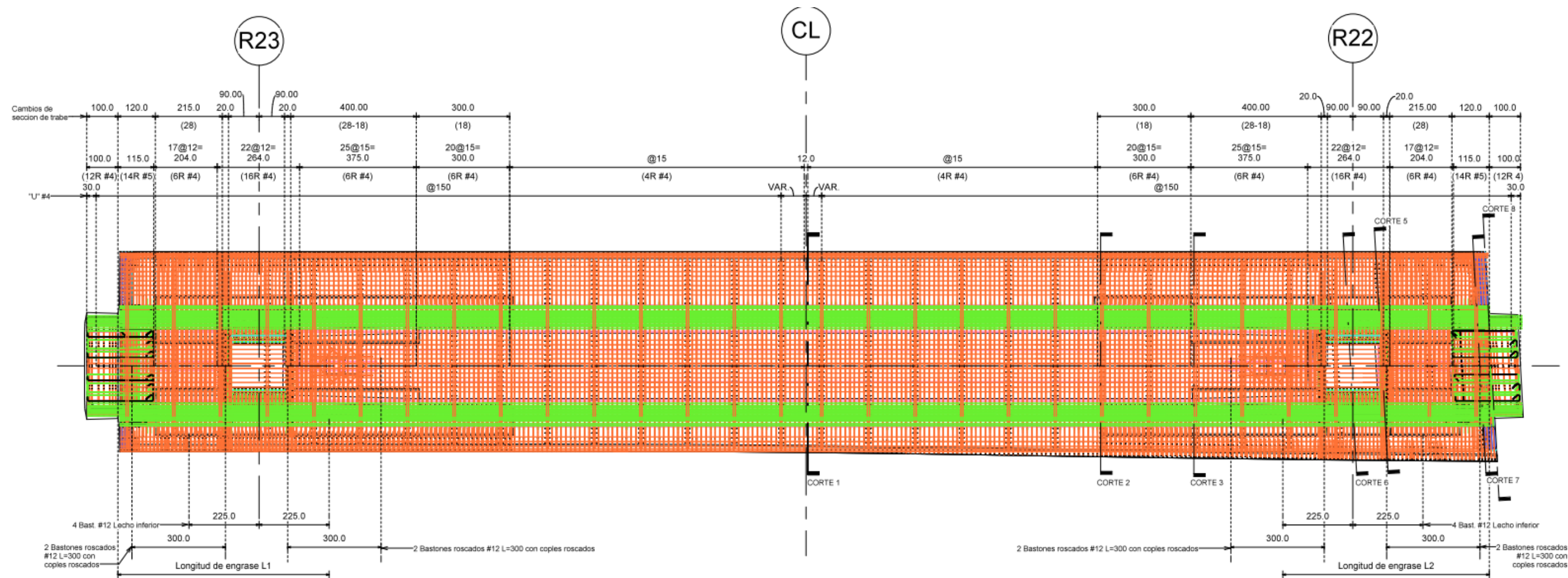
PLANO DE REFERENCIA:

ESCALA: 1 : 50

COTAS: CM

FECHA:

02/09/14



Planta de Refuerzo



ELABORÓ:

Arq. Aarón De Marcos Estévez
MODELADOR

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INGENIEROS CIVILES ASOCIADOS
PREFABRICADOS Y TRANSPORTES S.A. DE C.V.
BUILDING INFORMATION MODELING



ESPECIFICACIONES GENERALES:

PLANOS DE REFERENCIA:

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VPE-EST-F5-CUPRI-III-014-P-00 (05C26B11_G2 DE 2)
VPE-EST-F5-CUPRI-III-015-P-01 (05C26B11_R1 DE 3)
VPE-EST-F5-CUPRI-III-016-P-01 (05C26B11_R2 DE 3)
VPE-EST-F5-CUPRI-III-017-P-01 (05C26B11_R3 DE 3)

MATERIALES:

CONCRETO F'c= 600 kg/cm2
EDAD= 28 días
AGREGADO MAX. = 1/2" (1.27cm)
ACERO DE REFUERZO Fy= 4200 Kg/cm2
ACERO EN PLACAS, ACCESORIOS METALICOS Y
TENSORES Fy= 2530kg/cm2
ACERO DE PRESFUERZO Fy= 19000kg/cm2 (torones de 3/8")
ACERO ESTRUCTURAL = A-50
POLIESTIRENO:

ELEMENTO:
CAPITEL_B6

HOJA:
ECP01

CROQUIS:
GEOMETRIA

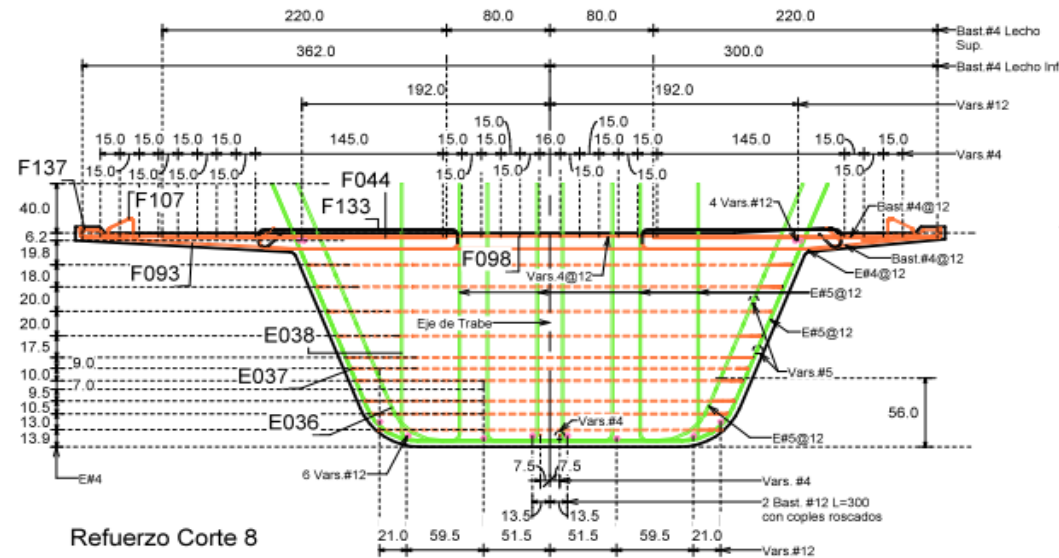
PIEZA A FABRICAR:
B6_8.50m

PLANO DE REFERENCIA:

ESCALA: 1 : 50

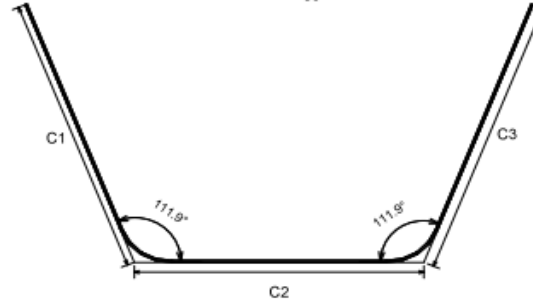
COTAS: CM

FECHA:
02/09/14



Refuerzo Corte 8

1.Acero Corte 8_E036,E037						
Clave	No. de Varilla	Sub-Zona	Longitud Unitaria	Cantidad	C1	C2
E036	#05	Corte 8	660.97	12	197.16	166.08
E037	#05	Corte 8	699.34	24	197.16	204.45
				36		

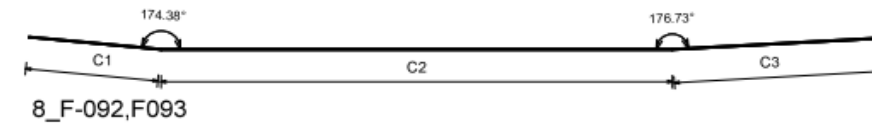


8_E036,E037

1.Acero Corte 8_F-037,F044					
Clave	No. de Varilla	Sub-Zona	Longitud Unitaria	Cantidad	B
F037	#04	Corte 8	190.00	6	190.00
F044	#04	Corte 8	220.00	18	220.00
				24	

8_F-037,F044

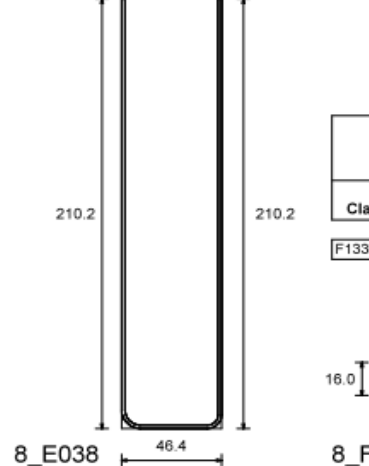
1.Acero Corte 8_F-092,F093						
Clave	No. de Varilla	Sub-Zona	Longitud Unitaria	Cantidad	A	B
F092	#04	Corte 8	634.45	6	164.36	395.83
F093	#04	Corte 8	662.76	6	164.36	395.83
				12		



8_F-092,F093

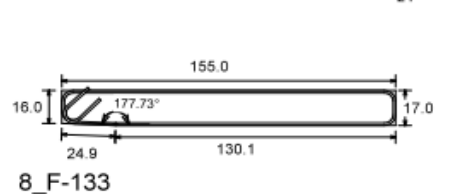
1.Acero Corte 8				
Clave	No. de Varilla	Sub-Zona	Longitud Unitaria	Cantidad
F037	#04	Corte 8	190.00	6
F044	#04	Corte 8	220.00	18
F092	#04	Corte 8	634.45	6
F093	#04	Corte 8	662.76	6
F098	#04	Corte 8	673.32	6
F100	#04	Corte 8	644.50	6
F133	#04	Corte 8	369.64	24
E025	#05	Corte 8	452.32	24
E036	#05	Corte 8	660.97	12
E037	#05	Corte 8	699.34	24
E038	#05	Corte 8	457.58	24
				156

1.Acero Corte 8_E038				
Clave	No. de Varilla	Sub-Zona	Longitud Unitaria	Cantidad
E038	#05	Corte 8	457.58	24



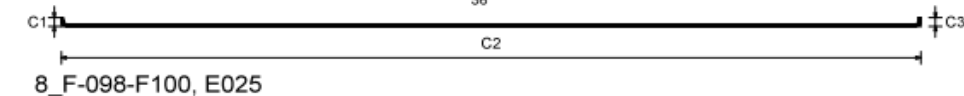
8_E038

1.Acero Corte 8_F-133				
Clave	No. de Varilla	Sub-Zona	Longitud Unitaria	Cantidad
F133	#04	Corte 8	369.64	24



8_F-133

1.Acero Corte 8_F-098-F100, E025						
Clave	No. de Varilla	Sub-Zona	Longitud Unitaria	Cantidad	C1	C2
F098	#04	Corte 8	673.32	6	7.20	665.27
F100	#04	Corte 8	644.50	6	7.20	636.45
E025	#05	Corte 8	452.32	24	210.20	40.10
				36		



8_F-098-F100, E025



ELABORÓ:
Arq. Aarón De Marcos Estévez
MODELADOR

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INGENIERIA

REVISÓ Y APROBÓ:
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SUPERINTENDENTE



INGENIEROS CIVILES ASOCIADOS PREFABRICADOS Y TRANSPORTES S.A. DE C.V. BUILDING INFORMATION MODELING



ESPECIFICACIONES GENERALES:

PLANOS DE REFERENCIA:

VPE-PGE-F5-CUPRI-III-048-P-00
VPE-EST-F5-CUPRI-III-013-P-00 (05C26B11_G1 DE 2)
VPE-EST-F5-CUPRI-III-014-P-00 (05C26B11_G2 DE 2)
VPE-EST-F5-CUPRI-III-015-P-01 (05C26B11_R1 DE 3)
VPE-EST-F5-CUPRI-III-016-P-01 (05C26B11_R2 DE 3)
VPE-EST-F5-CUPRI-III-017-P-01 (05C26B11_R3 DE 3)

MATERIALES:

CONCRETO F'c= 600 kg/cm2
EDAD= 28 días
AGREGADO MAX. = 1/2" (1.27cm)
ACERO DE REFUERZO Fy= 4200 Kg/cm2
ACERO EN PLACAS, ACCESORIOS METALICOS Y
TENSORES Fy= 2530kg/cm2
ACERO DE PRESFUERZO Fy= 19000kg/cm2 (torones de 3/8")
ACERO ESTRUCTURAL = A-50
POLIESTIRENO:

ELEMENTO:
CAPITEL_B6

HOJA:
ECP01

CROQUIS:
GEOMETRIA

PIEZA A FABRICAR:
B6_8.50m

PLANO DE REFERENCIA:

ESCALA: 1 : 50

COTAS: CM

FECHA:
02/09/14

1.Acero Estribos Planta

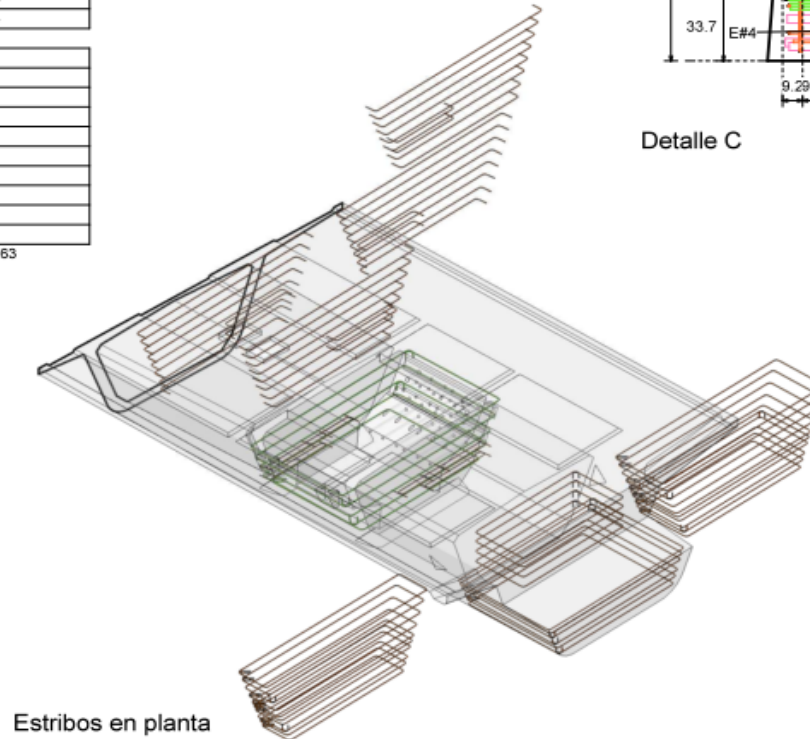
Clave	No. de Varilla	Sub-Zona	Longitud Unitaria	Cantidad
F010	#04	Corte EP	687.19	1
F011	#04	Corte EP	700.79	3
F012	#04	Corte EP	708.09	1
F013	#04	Corte EP	724.98	1
F014	#04	Corte EP	751.51	1
F015	#04	Corte EP	767.19	1
F016	#04	Corte EP	767.59	1
F017	#04	Corte EP	782.06	1
F018	#04	Corte EP	788.09	1
F019	#04	Corte EP	804.98	1
F020	#04	Corte EP	810.20	1
F021	#04	Corte EP	831.51	1
F022	#04	Corte EP	847.59	1
F023	#04	Corte EP	862.06	1
F024	#04	Corte EP	874.53	1
F025	#04	Corte EP	890.20	1
F026	#04	Corte EP	903.47	1
F027	#04	Corte EP	954.53	1
F028	#04	Corte EP	981.52	1
F029	#04	Corte EP	983.47	1
F030	#04	Corte EP	1002.42	1
F031	#04	Corte EP	1019.30	1
F032	#04	Corte EP	1045.84	1
F033	#04	Corte EP	1061.92	1
F034	#04	Corte EP	1076.39	1
F155	#04	Corte EP	709.38	4
F156	#04	Corte EP	725.58	1
F157	#04	Corte EP	796.81	1
F158	#04	Corte EP	843.16	1
F159	#04	Corte EP	856.40	1
F160	#04	Corte EP	931.30	1
F161	#04	Corte EP	963.87	1
F162	#04	Corte EP	1001.72	1
F163	#04	Corte EP	1028.30	1
F164	#04	Corte EP	1044.40	1
F165	#04	Corte EP	1058.90	1
F166	#04	Corte EP	687.21	1
F167	#04	Corte EP	708.15	1
F168	#04	Corte EP	751.64	1
F169	#04	Corte EP	767.74	1
F170	#04	Corte EP	775.35	1
F171	#04	Corte EP	782.24	1
F172	#04	Corte EP	810.42	1
F173	#04	Corte EP	813.20	1
F174	#04	Corte EP	839.78	1
F175	#04	Corte EP	870.38	1
F176	#04	Corte EP	874.85	1
F177	#04	Corte EP	898.56	1
F178	#04	Corte EP	903.84	1
F179	#04	Corte EP	962.99	1
F180	#04	Corte EP	984.29	1
F181	#04	Corte EP	991.98	1
F207	#04	Corte EP	135.65	4
F208	#04	Corte EP	135.81	4

1.Acero Estribos Planta

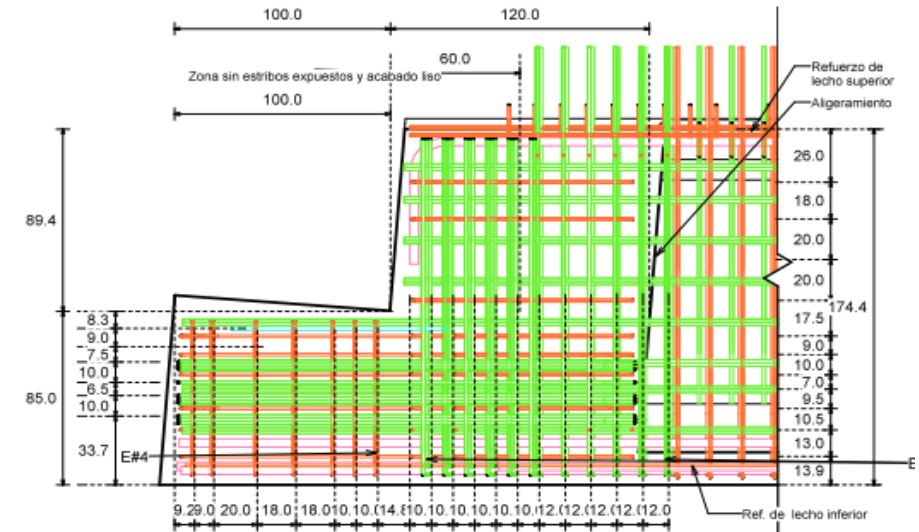
Clave	No. de Varilla	Sub-Zona	Longitud Unitaria	Cantidad
F209	#04	Corte EP	169.07	8
F210	#04	Corte EP	169.28	16
F211	#04	Corte EP	308.40	4
F212	#04	Corte EP	308.83	4
F213	#04	Corte EP	324.48	4
F214	#04	Corte EP	324.94	3
F215	#04	Corte EP	325.17	1
F216	#04	Corte EP	342.17	4
F217	#04	Corte EP	342.66	4
F218	#04	Corte EP	358.25	4
F219	#04	Corte EP	358.76	4
F220	#04	Corte EP	374.33	4
F221	#04	Corte EP	374.87	4
F222	#04	Corte EP	390.41	4
F223	#04	Corte EP	390.98	4
F224	#04	Corte EP	406.49	4
F225	#04	Corte EP	407.08	4
F226	#04	Corte EP	422.57	4
F227	#04	Corte EP	423.19	4

E003	#05	Corte EP	984.16	1
E004	#05	Corte EP	1059.42	1
E005	#05	Corte EP	1093.35	1
E006	#05	Corte EP	1134.52	1
E007	#05	Corte EP	1175.68	1
E030	#05	Corte EP	1060.68	1
E031	#05	Corte EP	1094.67	1
E032	#05	Corte EP	1135.90	1
E033	#05	Corte EP	1177.13	1
E035	#05	Corte EP	984.65	1

163



Estribos en planta





INGENIEROS CIVILES ASOCIADOS
BUILDING INFORMATION MODELING



ESPECIFICACIONES GENERALES:

CONCRETO:
Clase D, según especificaciones del M.O.P.
Resistencia cilíndrica a la compresión al momento de transferencia = 385.612 kg/cm² (5,200 p.s.i.)
Resistencia cilíndrica a la compresión a los 28 días = 457.015 kg/cm² (6,500 p.s.i.)
Tamaño máximo del agregado grueso = 0.013 m

ACERO DE REFUERZO:
El acero de refuerzo se ajustará a los requisitos de las especificaciones A.S.T.M. A615, para barras de deformación grado 42. El acero de refuerzo tendrá una resistencia a la cedencia de 4,218 kg/cm² (60,000 p.s.i.) obtenida mediante una adecuada composición química del acero tal como lo establece el A.S.T.M. A615 (A.A.S.H.T.O. M31). No se permitirán los aceros endurecidos por deformación en frío.

ACERO DE PRESFUERZO:
Se ajustará a los requisitos de las especificaciones A.S.T.M. A416 (A.A.S.H.T.O. M203) y será del grado 270.

NOTAS GENERALES:

El tendido de los cables sólo se podrá iniciar después que se haya inspeccionado su colocación y la armadura pasiva. Antes de iniciar cualquier tendido, el Contratista debe presentar el cálculo de las pérdidas para su confrontación con las pérdidas asumidas en el diseño. A todos los Cables, Torones o Alambres se le aplicará una tensión máxima de 0.75 fpu. Las Viguetas pre tensionadas se instalarán sobre sus apoyos, ubicados en los Estribos, tal como se indica en el Plano. Todas las vistas vistas se echafrarán 0.02m.

Piezas a Fabricar: 8

CORREDOR NORTE PANAMÁ - VILLALOBOS RAMPA 42

ELEMENTO:
VLR_BI-48

HOJA:
2/3

CROQUIS:
ACERO

PIEZA A FABRICAR:
VLR_BI-48

PLANO DE REFERENCIA:
CN-TR-VLR-ES-04

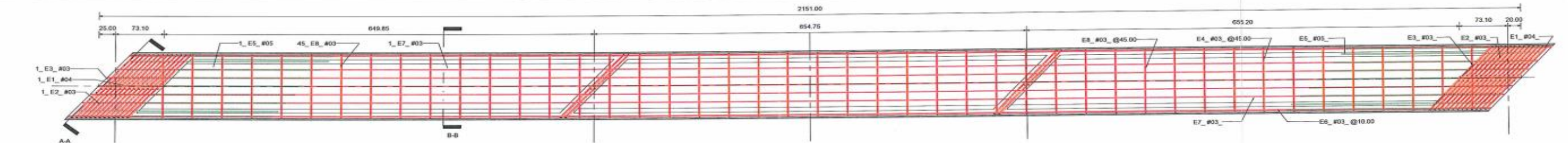
ESCALA:

S/E

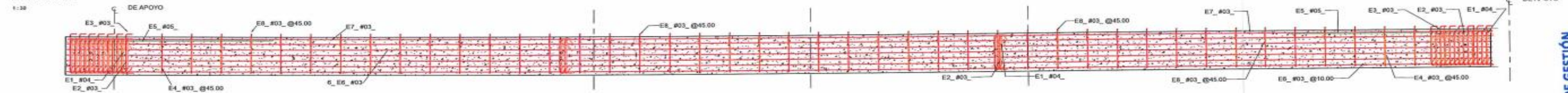
COTAS:

CM

FECHA:
19 DE MARZO DE 2013



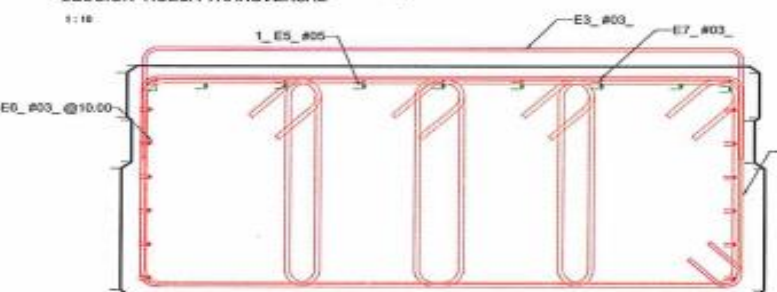
PLANTA ACERO
1:30



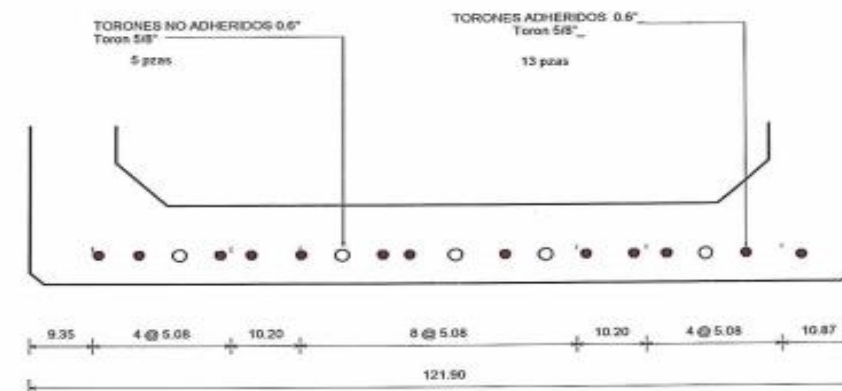
DISPOSICION DE LA ARMADURA (VISTA LONGITUDINAL)
1:30



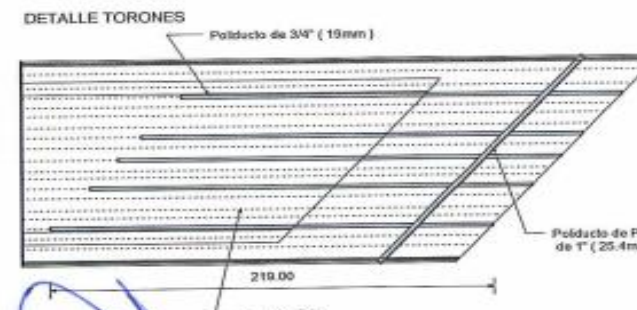
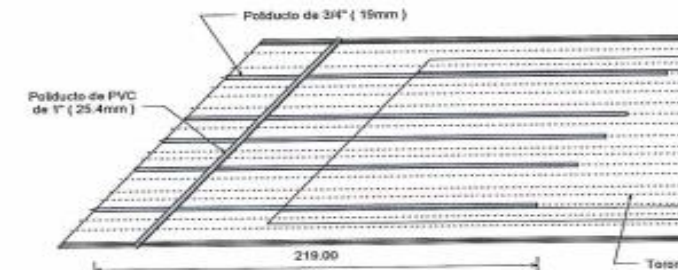
SECCION HUECA TRANSVERSAL
1:10



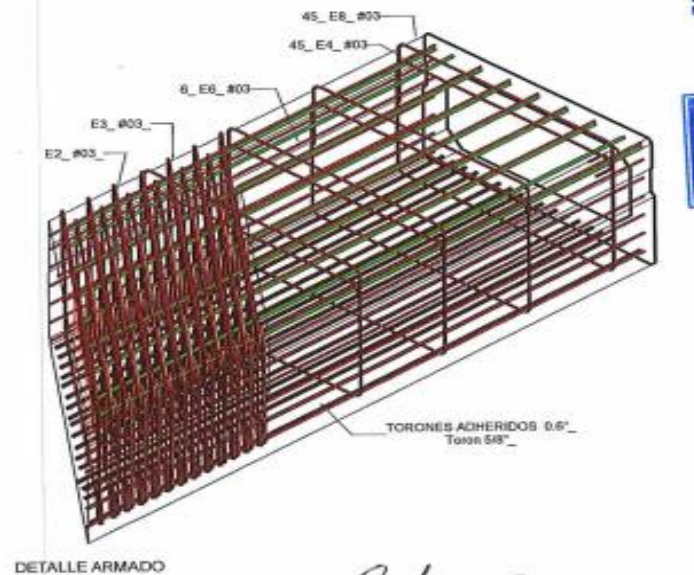
SECCION SOLIDA TRANSVERSAL A-A
1:10



SECCION TRANSVERSAL A-A (TORONES)
1:6



DETALLE TORONES



DETALLE ARMADO



ELABORÓ:
Arq. Alejandro Ortega Anaya
JEFE DE FRENTÉ

REVISÓ:
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VISTO BUENO:
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AUTORIZÓ PARA SU EJECUCIÓN:
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INGENIERÍA
ICAPSA

AUTORIZÓ PARA SU EJECUCIÓN:
Ing. Carlos Barredes Esquer
GERENTE DE PROYECTO

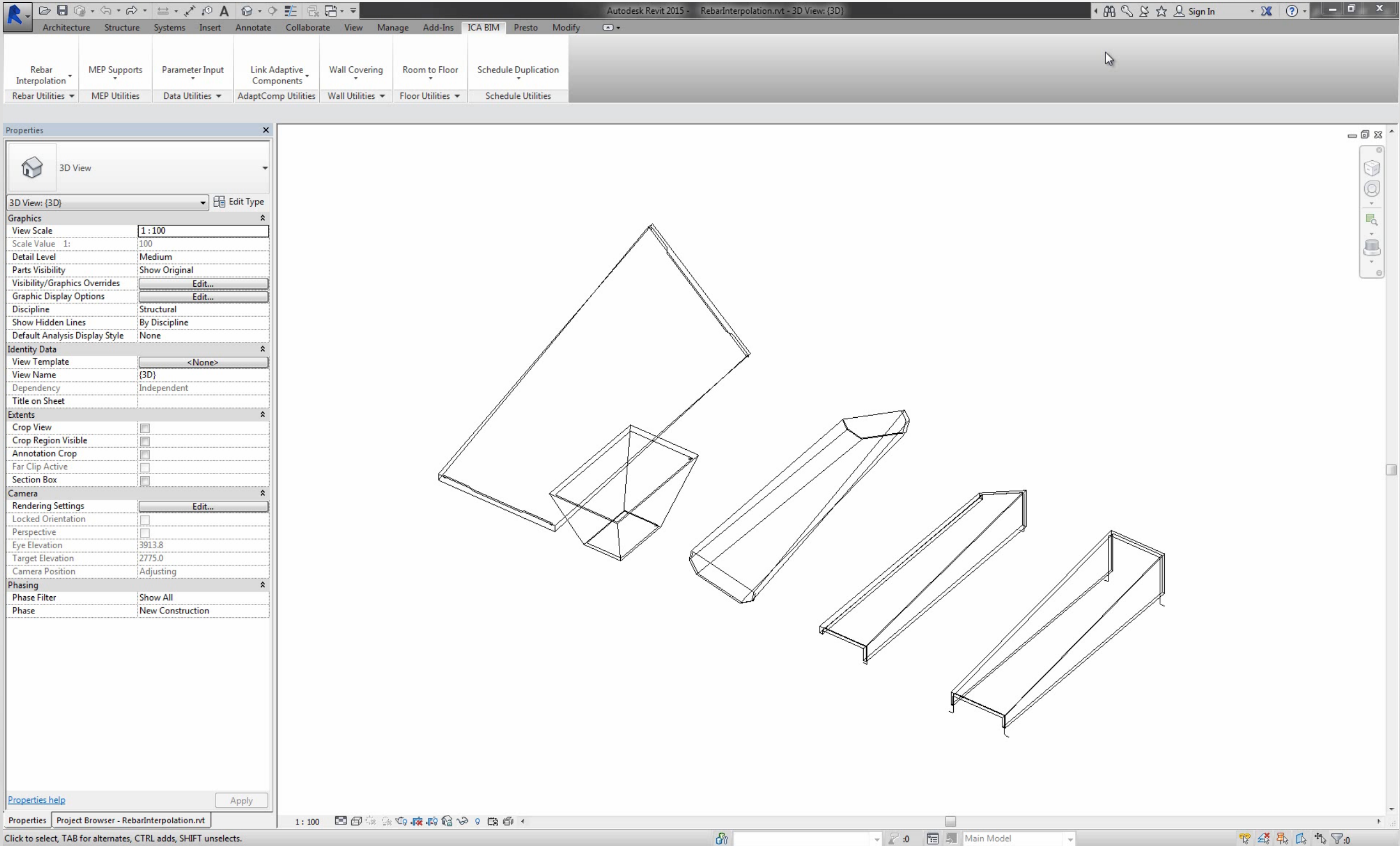
SISTEMA DE GESTIÓN DOCUMENTAL



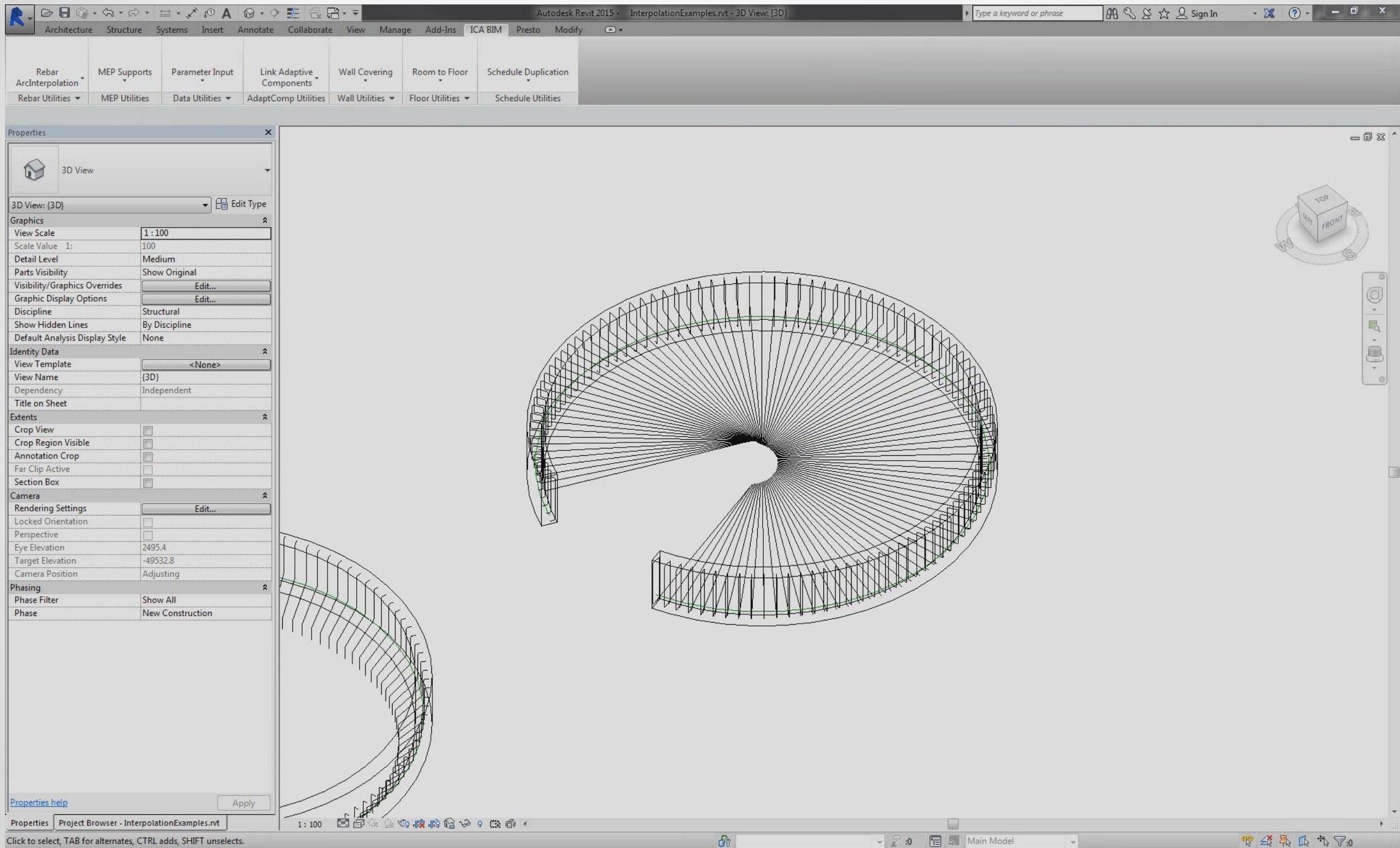
AUTODESK UNIVERSITY 2014

AUTODESK

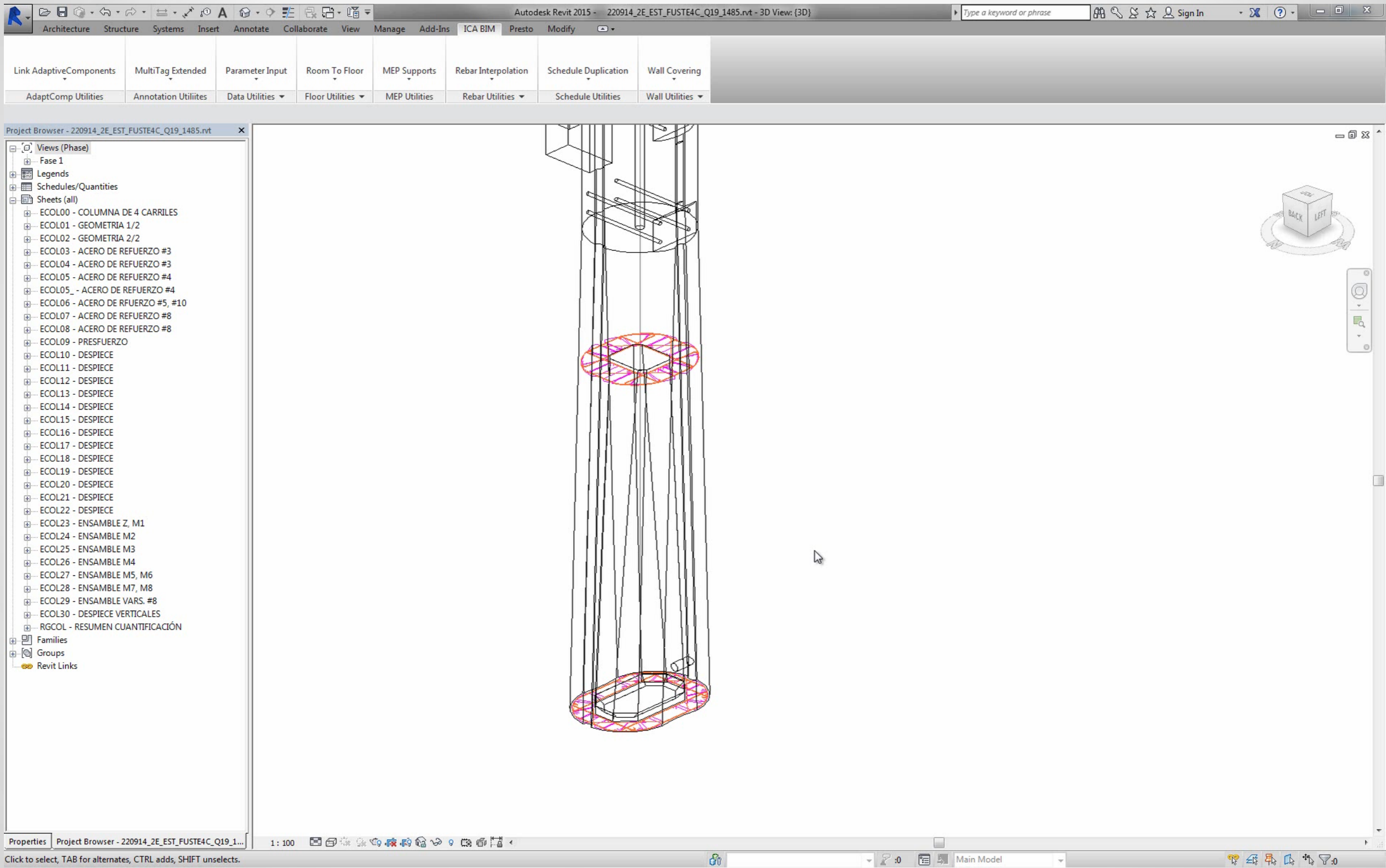
Rebar Interpolation App

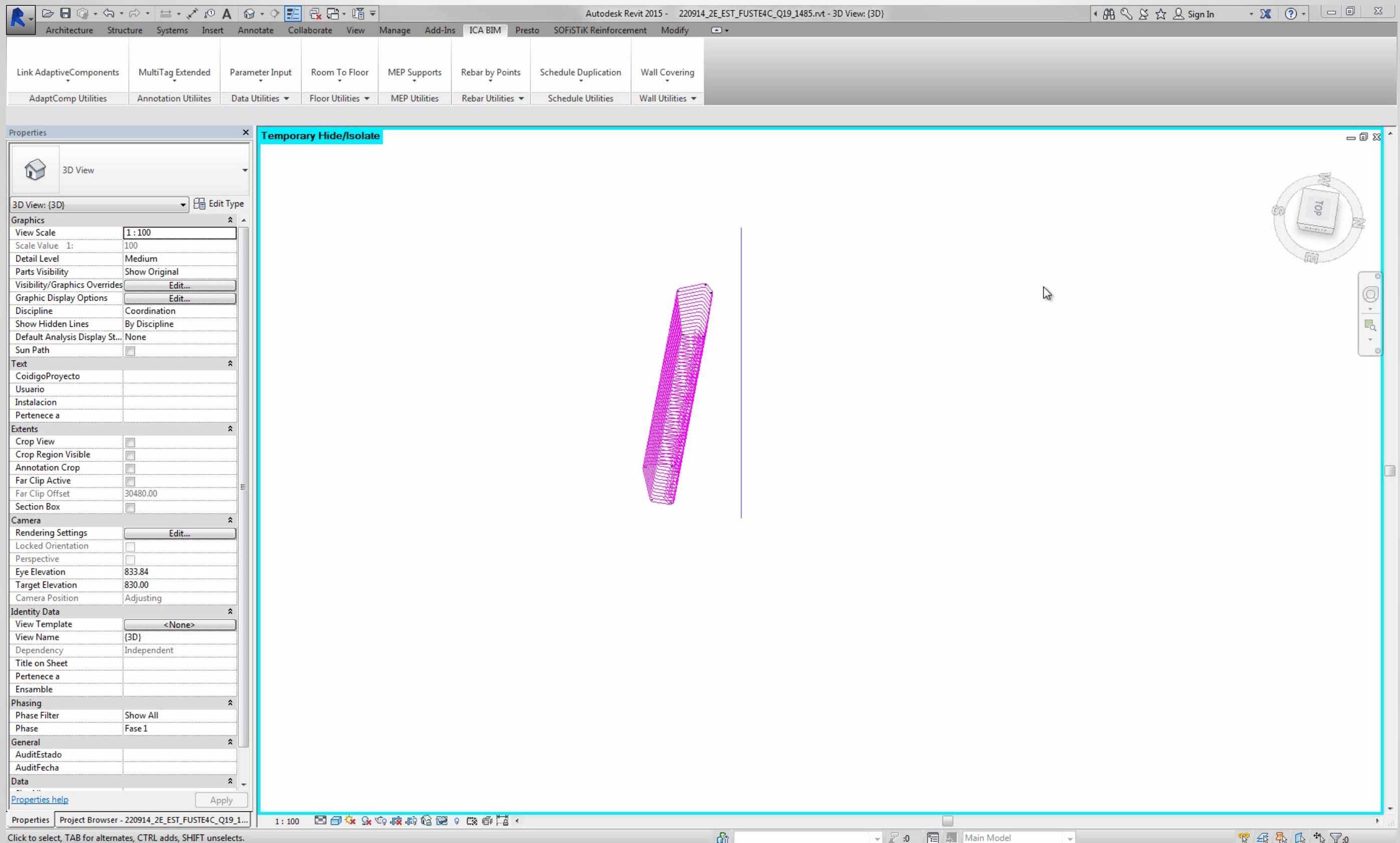


Rebar Arc Interpolation App

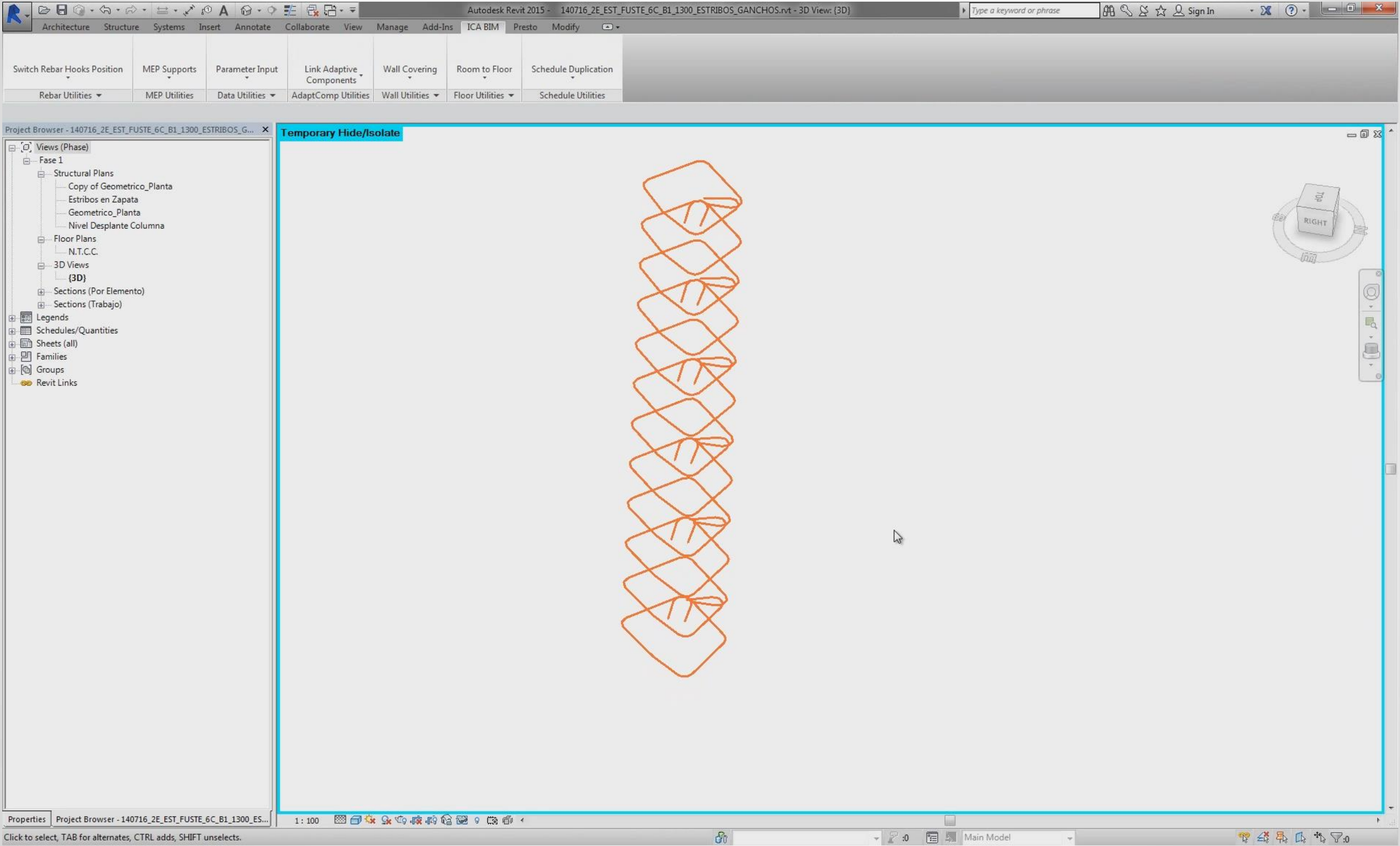


Rebar Interpolation App

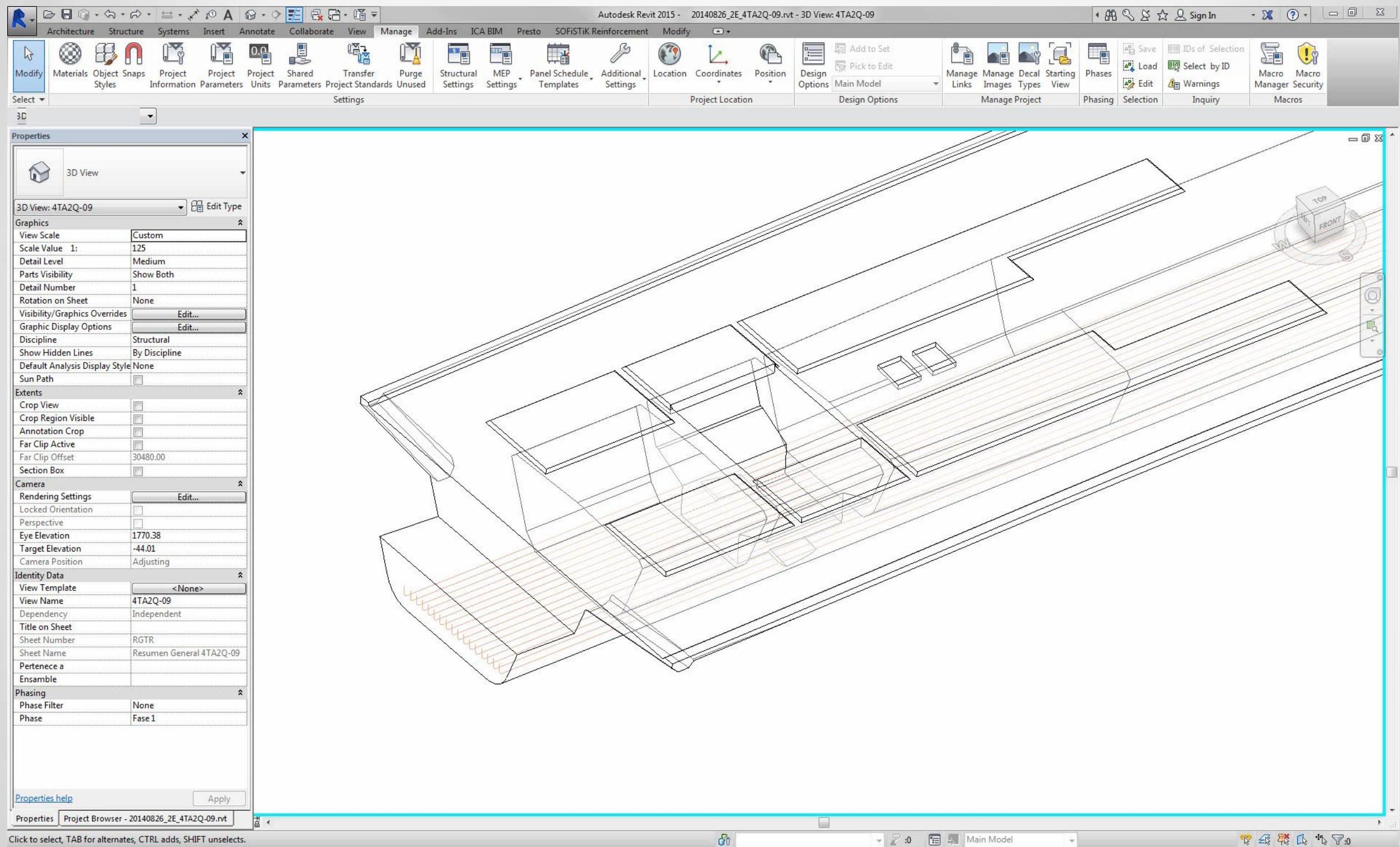




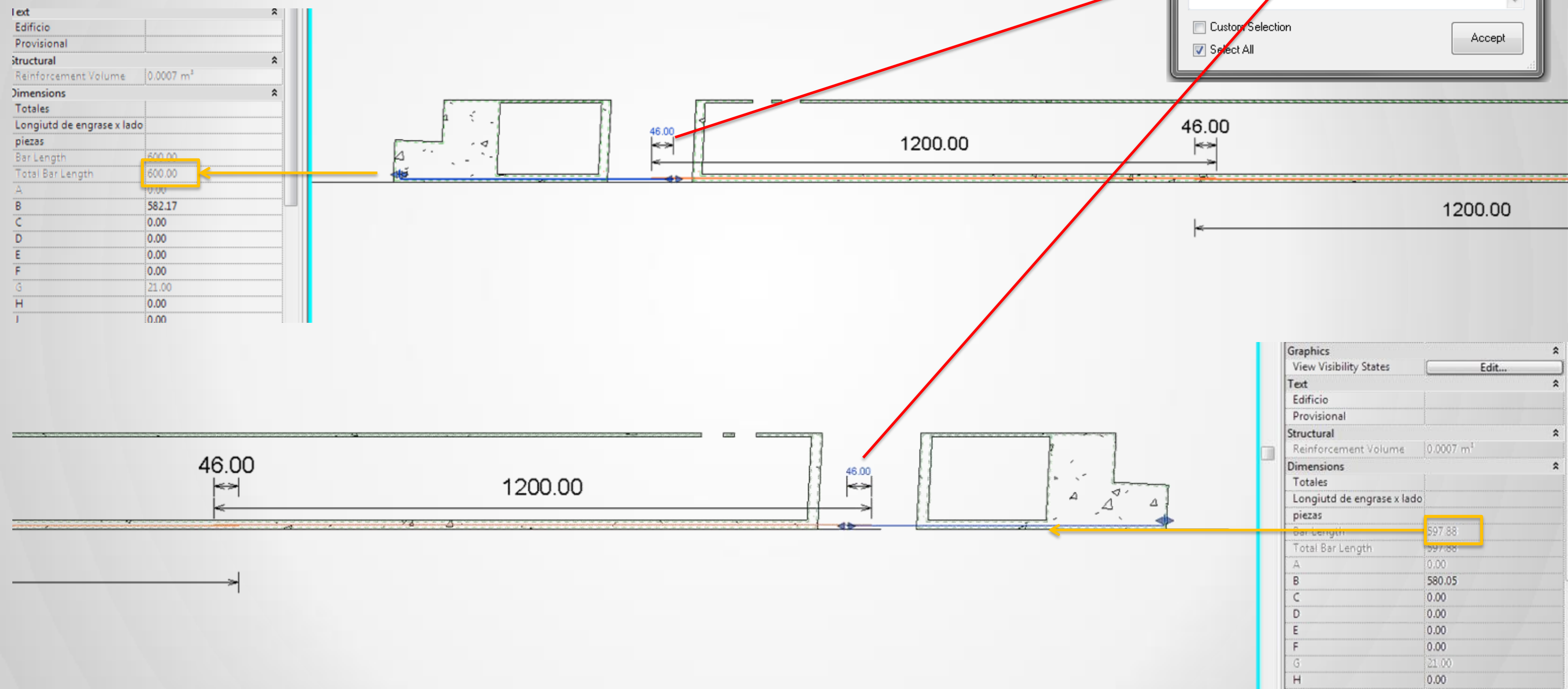
Change Rebar Hook Position App



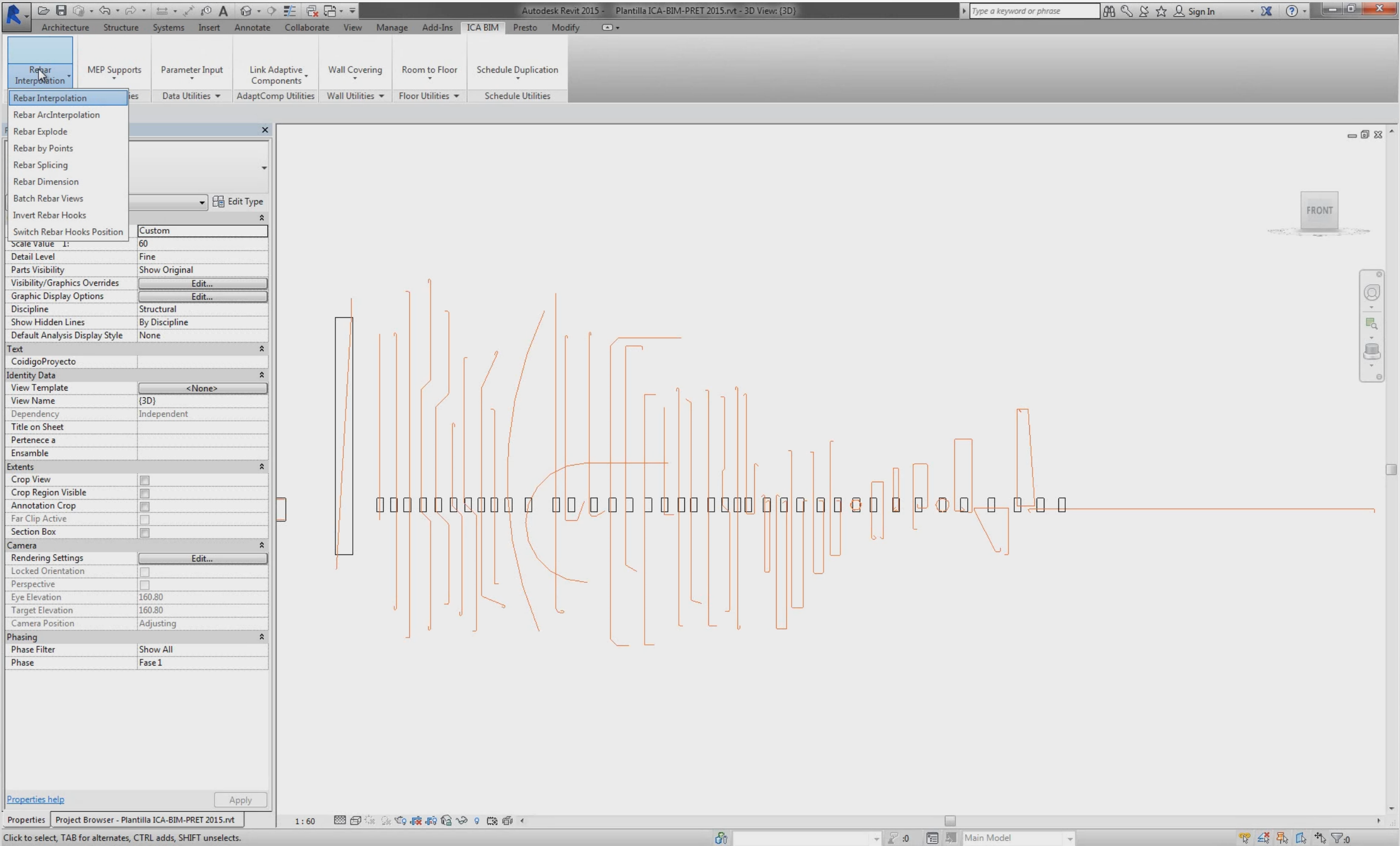
Rebar Splice App



Rebar Splice App



Rebar Splice App



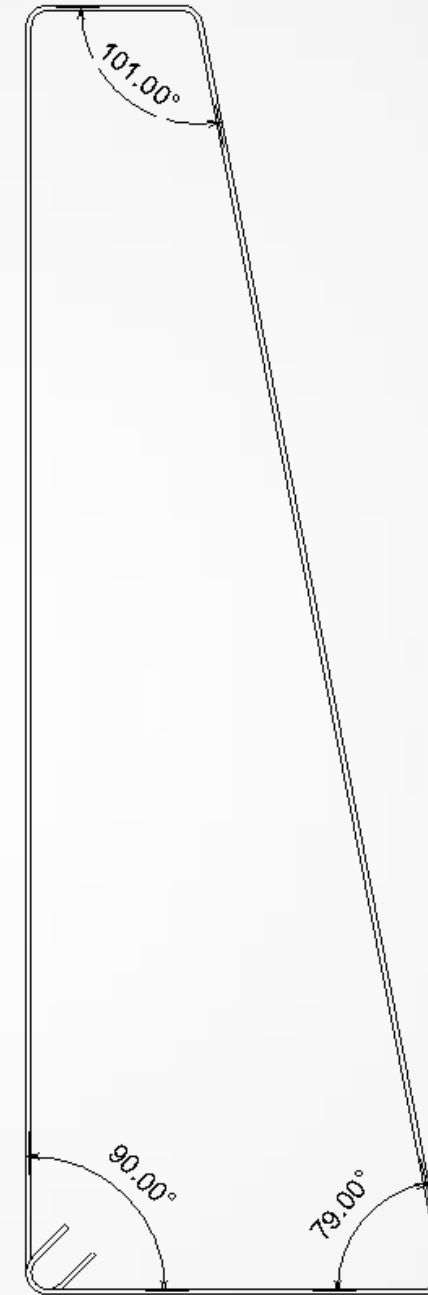
Tips and Tricks

Rebar Shapes

- When sketch a new rebar, Revit tries to match the segments and hooks with its internal library if didn't find a match it creates a new form.
- The rules for matching are:
 - Number of segments regardless of the dimension.
 - Angle relationship between contiguous segments
 - Hook Types and Hook Orientations
 - Style of rebar (Standard or Stirrup/Tie)

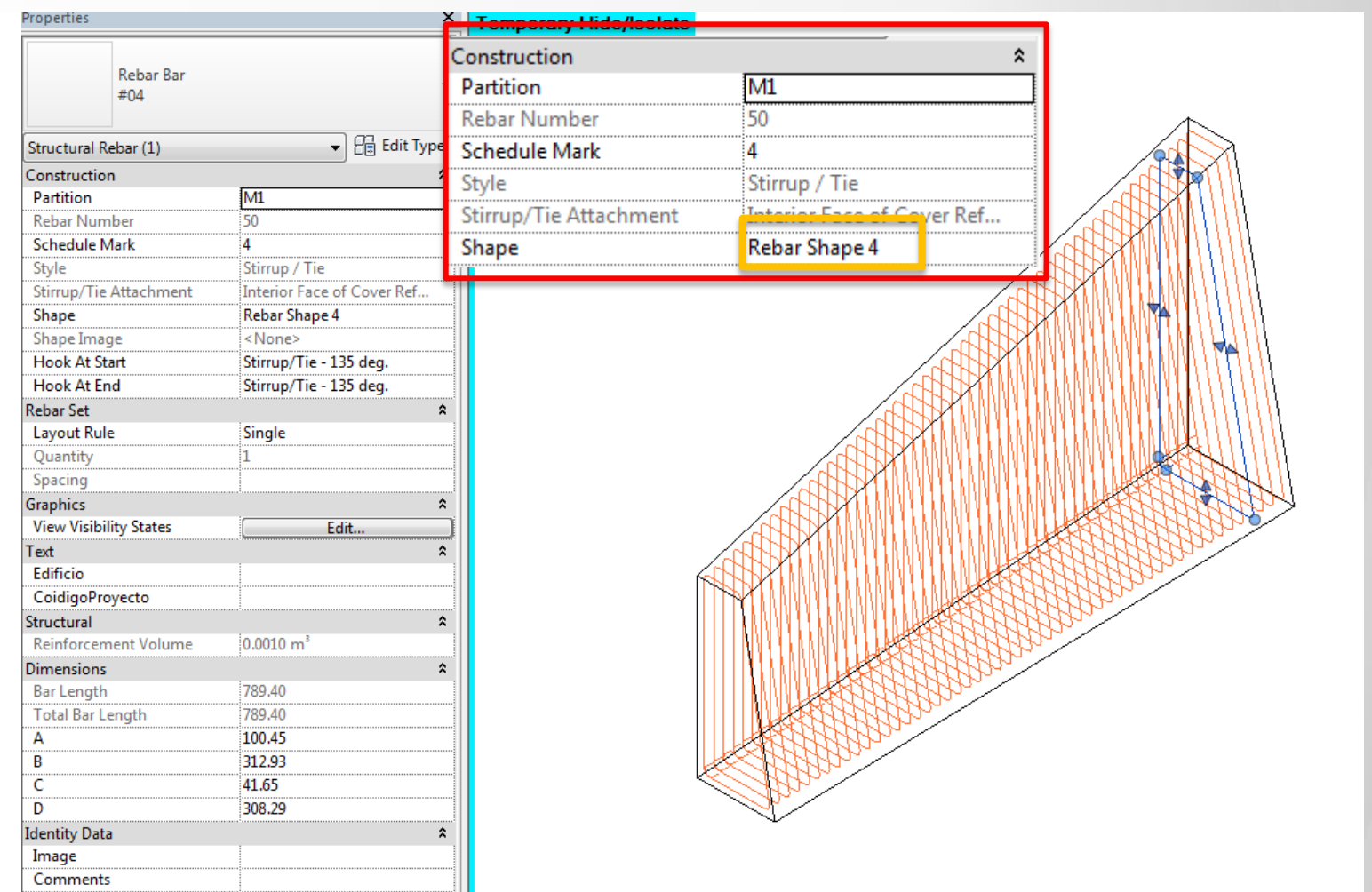
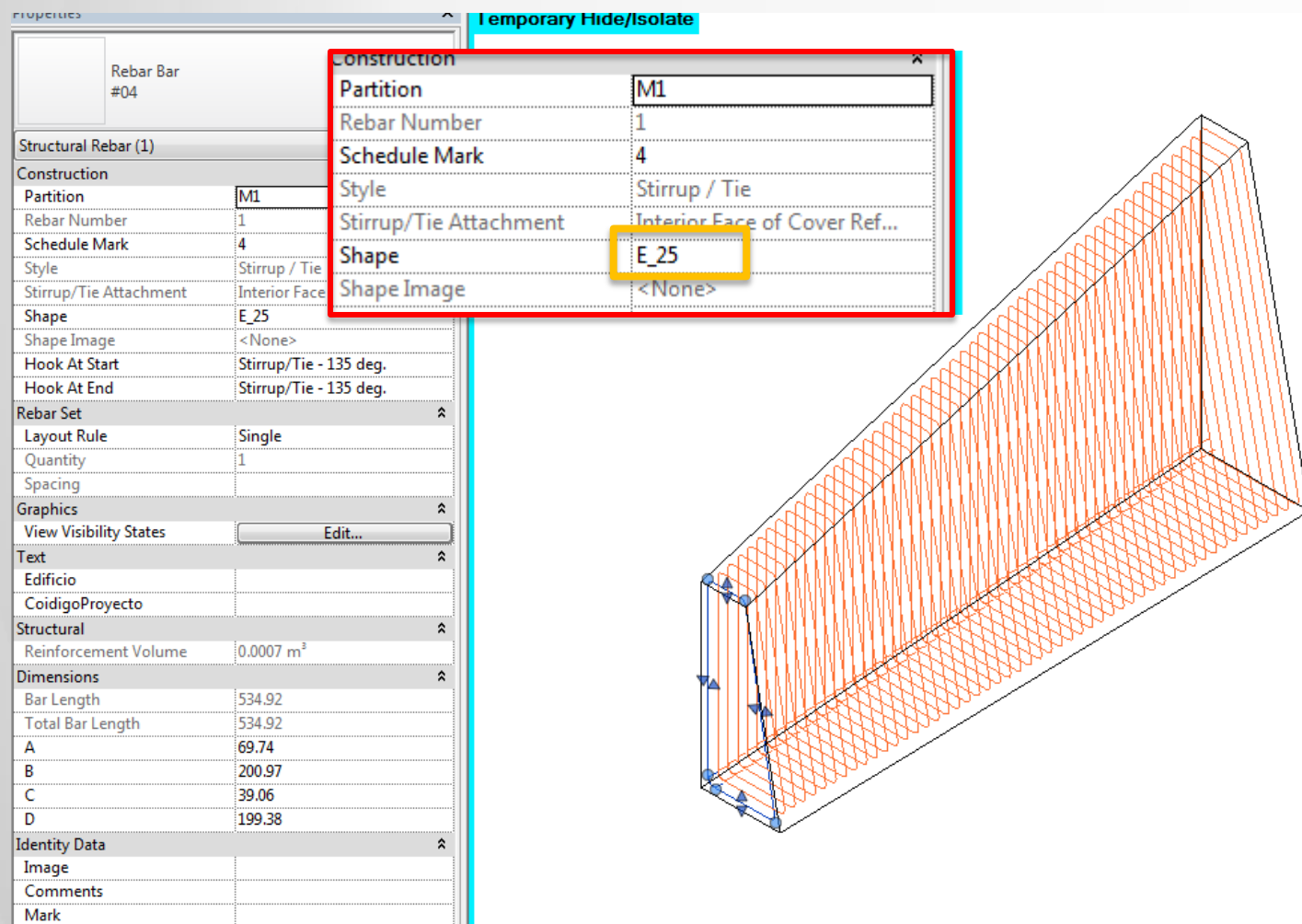
Rebar in Twisted Elements

- If the element is twisted the rebar stirrups need to be adjusted along the section, this implies different angles.



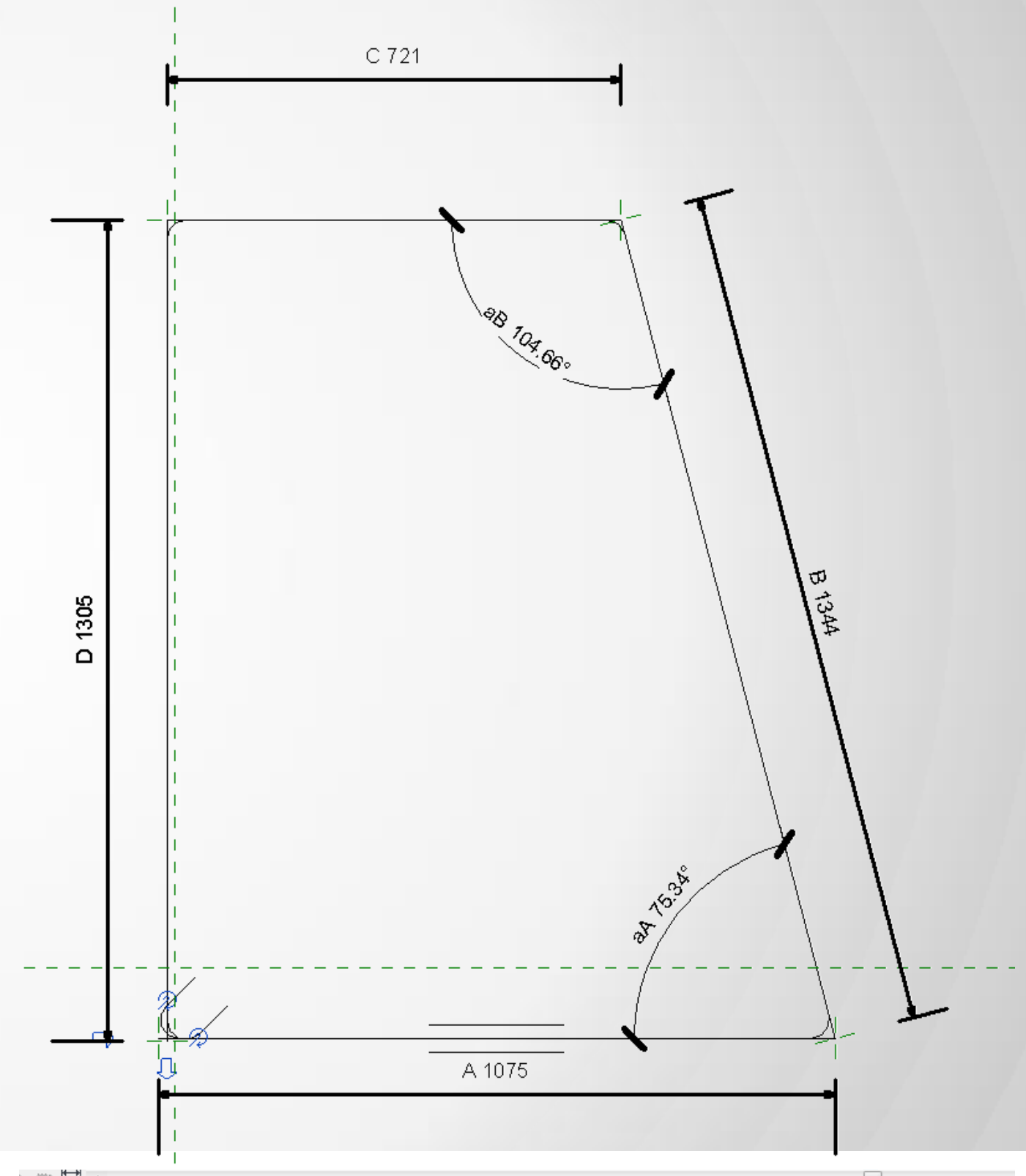
Rebar in Twisted Elements

- The problem is that the model ends with a new shape for every stirrup.



Rebar in Twisted Elements

- To solve this problem before modeling all rebars, select the rebar and click edit family inside the sketch dimension the angle between contiguous segments and assign a shared parameter



Rebar in Twisted Elements

- The problem is resolved and every stirrup has the same shape although it has different angles

Properties

Rebar Bar #04

Construction

Partition	
Rebar Number	95
Schedule Mark	4
Style	Stirrup / Tie
Stirrup/Tie Attachment	Interior Face of Cover R...
Shape	E_26
Shape Image	<None>
Hook At Start	Stirrup/Tie - 135 deg.
Hook At End	Stirrup/Tie - 135 deg.

Rebar Set

Layout Rule: Single

Quantity: 1

Spacing:

Graphics

View Visibility States: Edit...

Text

Edificio

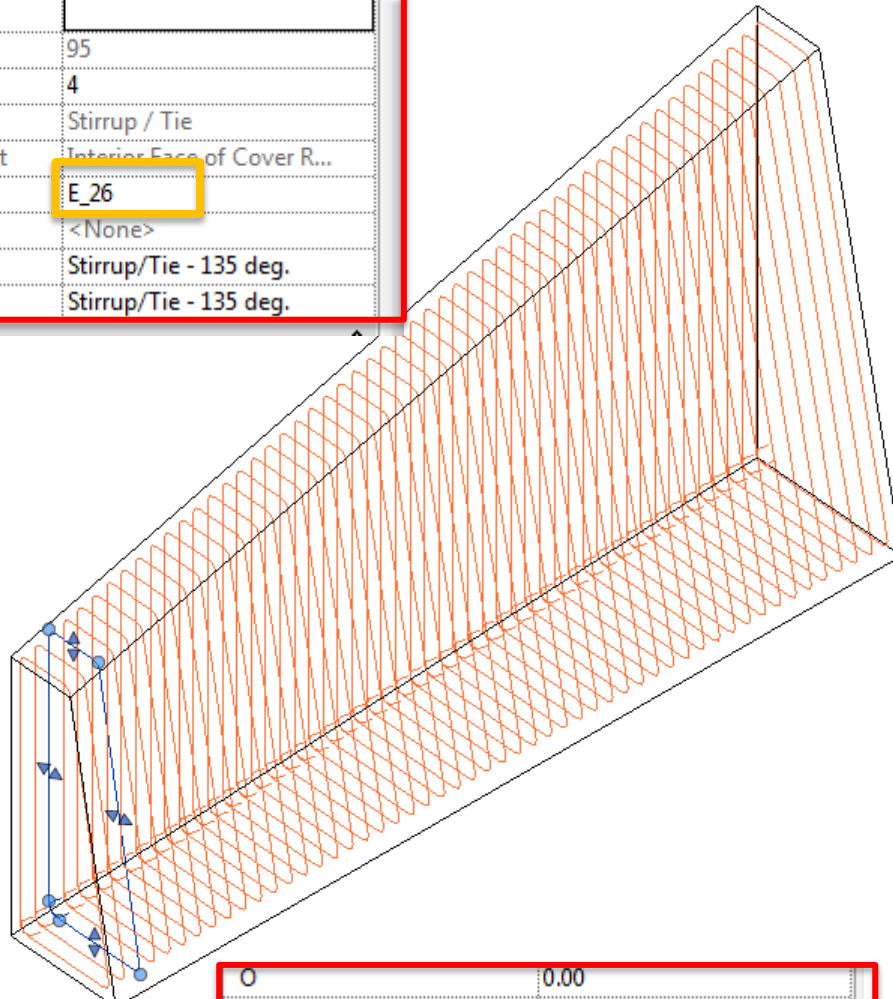
CodigoProyecto

Structural

Reinforcement Volume: 0.0007 m³

Dimensions

Bar Length	545.62
Total Bar Length	545.62
A	71.04
B	205.68
C	39.14
D	203.97
E	0.00
F	0.00
G	0.00
H	0.00
J	0.00
K	0.00
O	0.00
R	0.00
aA	80.860°
aB	99.140°



Properties

Rebar Bar #04

Construction

Partition	
Rebar Number	143
Schedule Mark	4
Style	Stirrup / Tie
Stirrup/Tie Attachment	Interior Face of Cover R...
Shape	E_26
Shape Image	<None>
Hook At Start	Stirrup/Tie - 135 deg.
Hook At End	Stirrup/Tie - 135 deg.

Rebar Set

Layout Rule: Single

Quantity: 1

Spacing:

Graphics

View Visibility States: Edit...

Text

Edificio

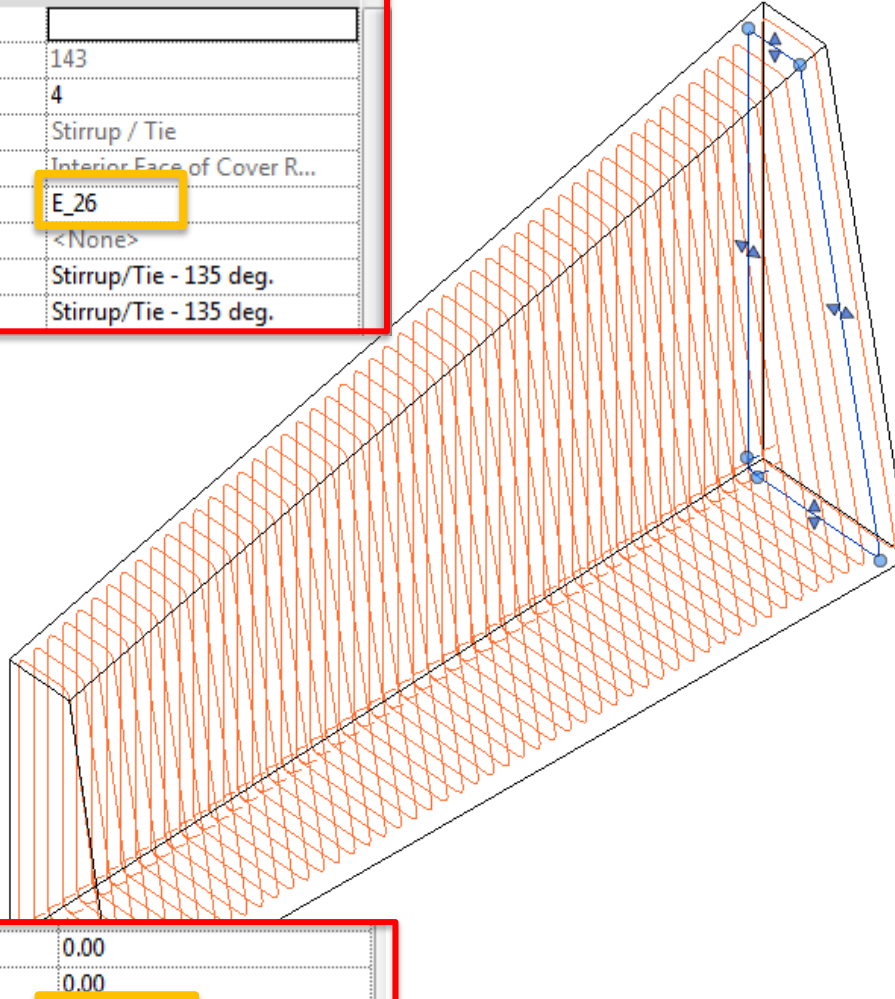
CodigoProyecto

Structural

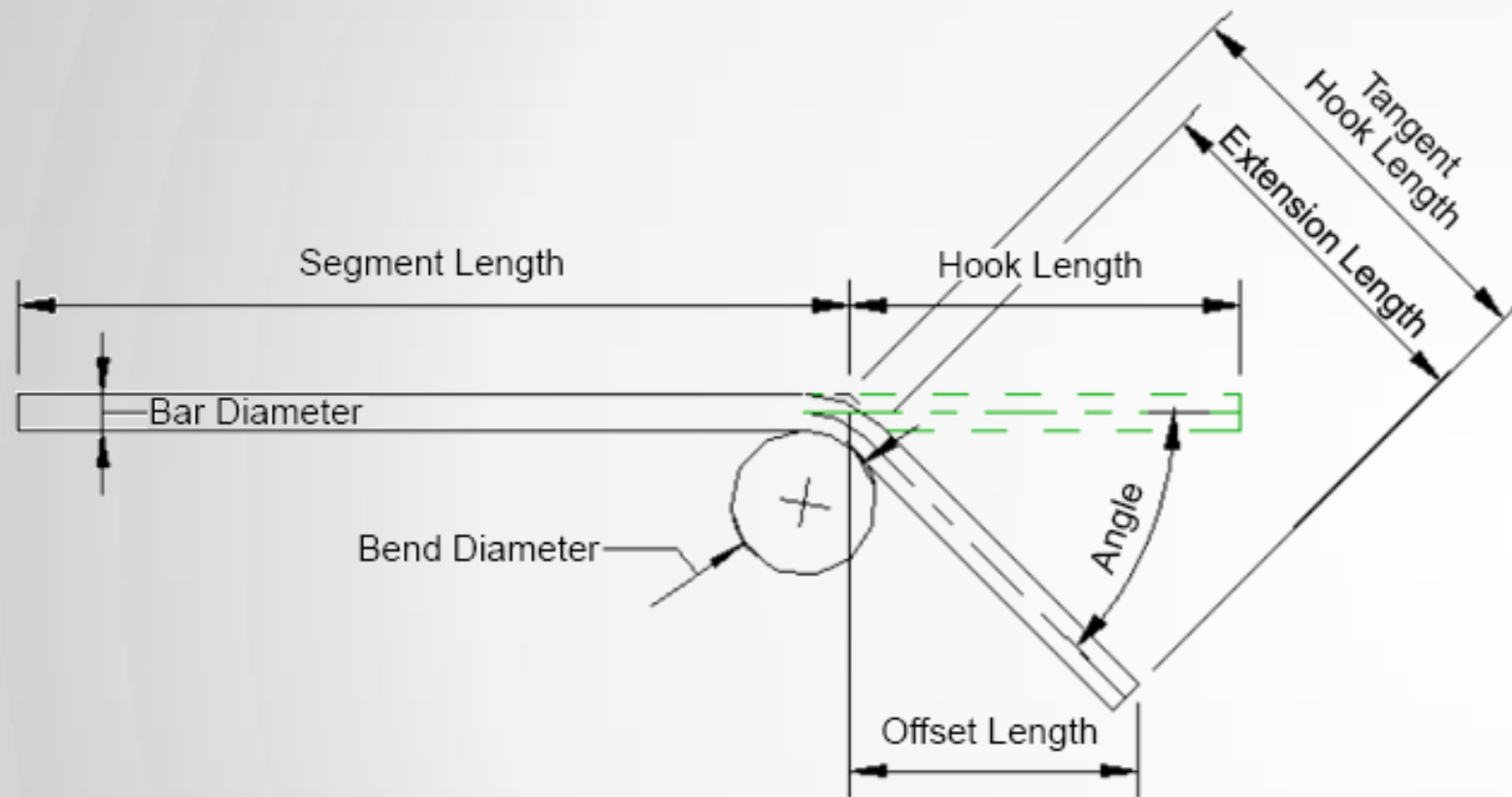
Reinforcement Volume: 0.0010 m³

Dimensions

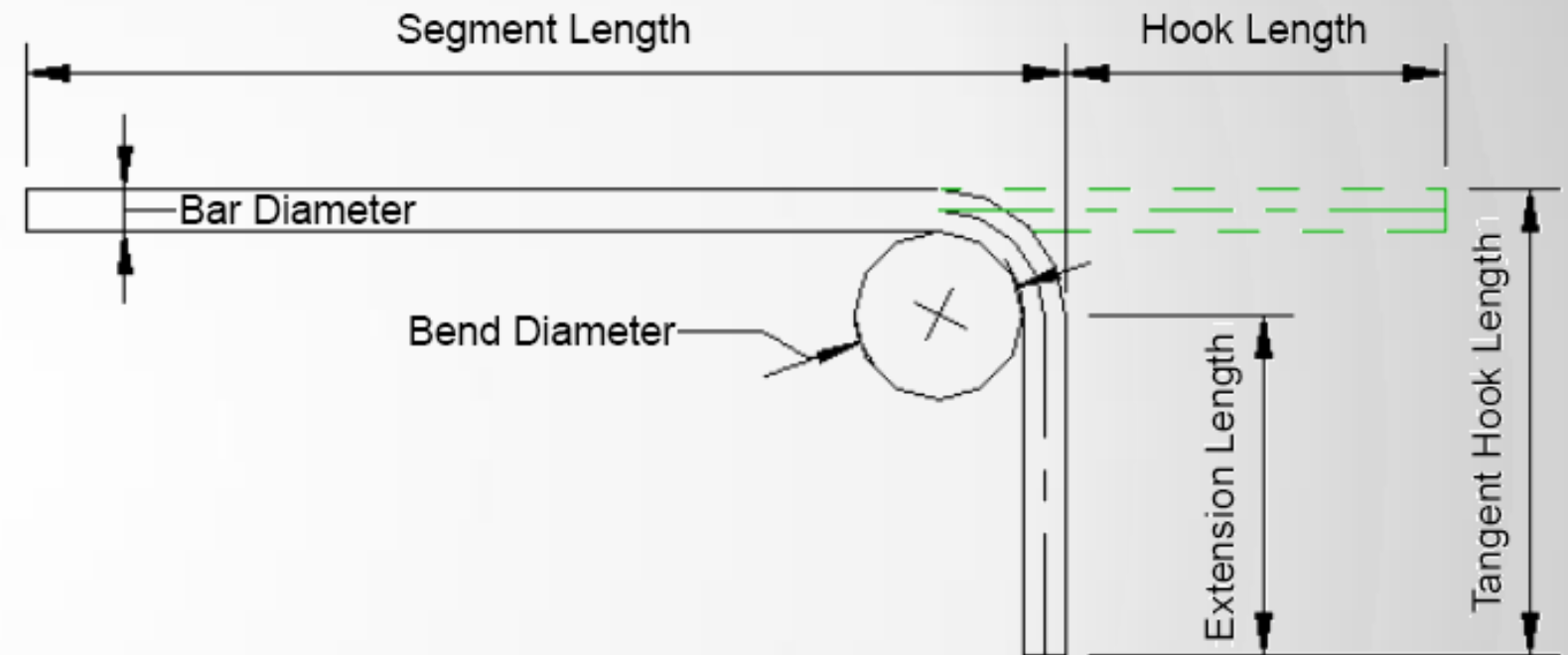
Bar Length	802.71
Total Bar Length	802.71
A	102.31
B	319.16
C	40.98
D	314.15
E	0.00
F	0.00
G	0.00
H	0.00
J	0.00
K	0.00
O	0.00
R	0.00
aA	78.751°
aB	101.249°



Hook Lengths



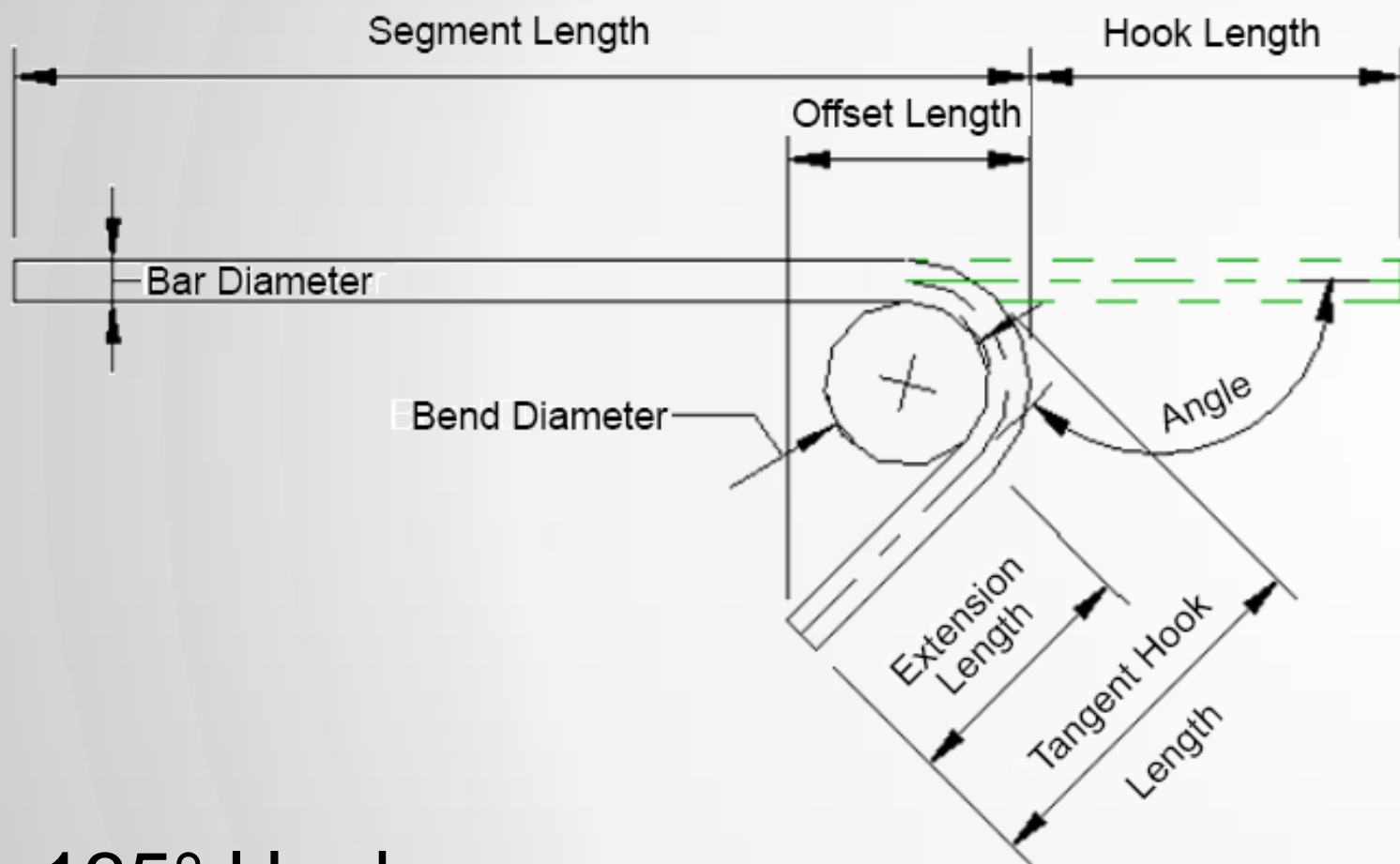
45° Hook



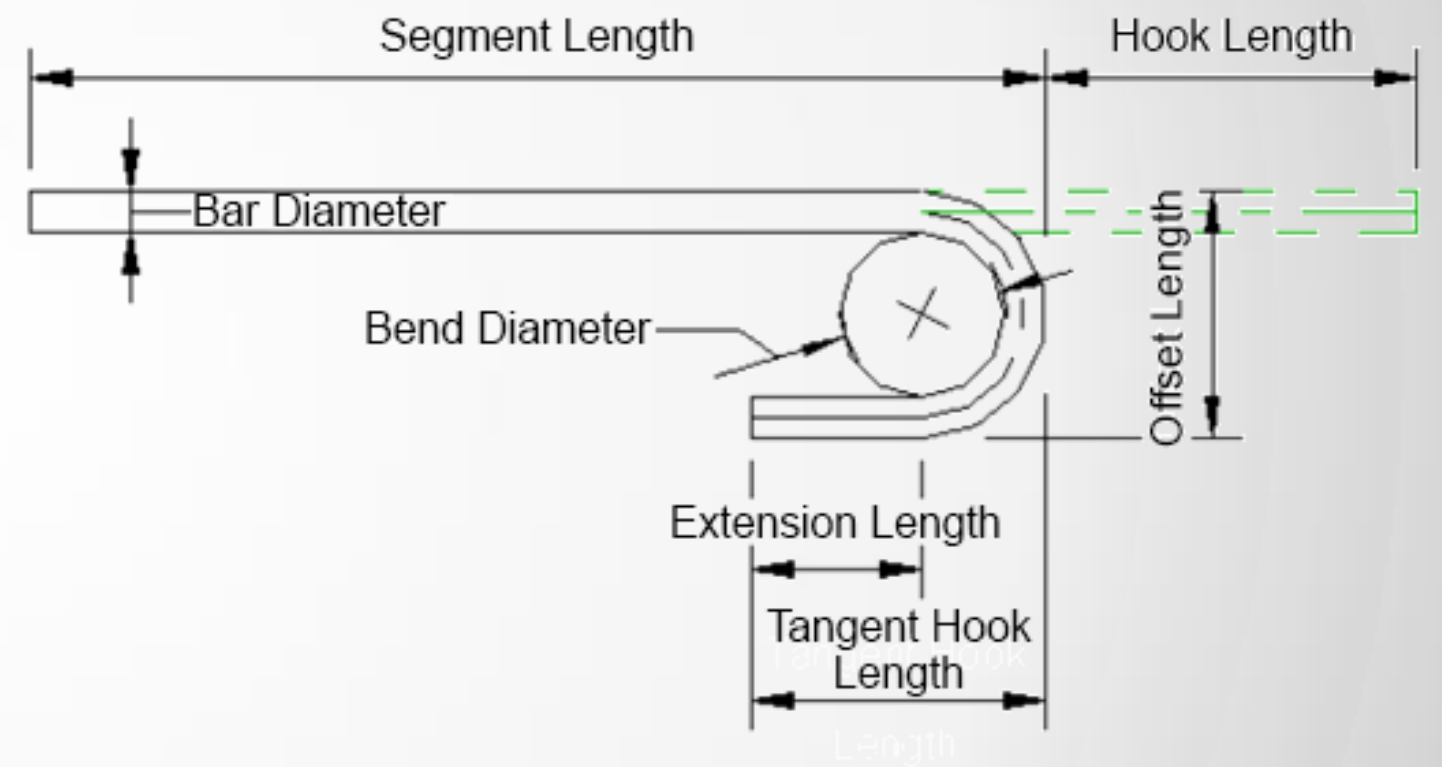
90° Hook

[SOURCE Revit 2015 Help](#)

Hook Lengths



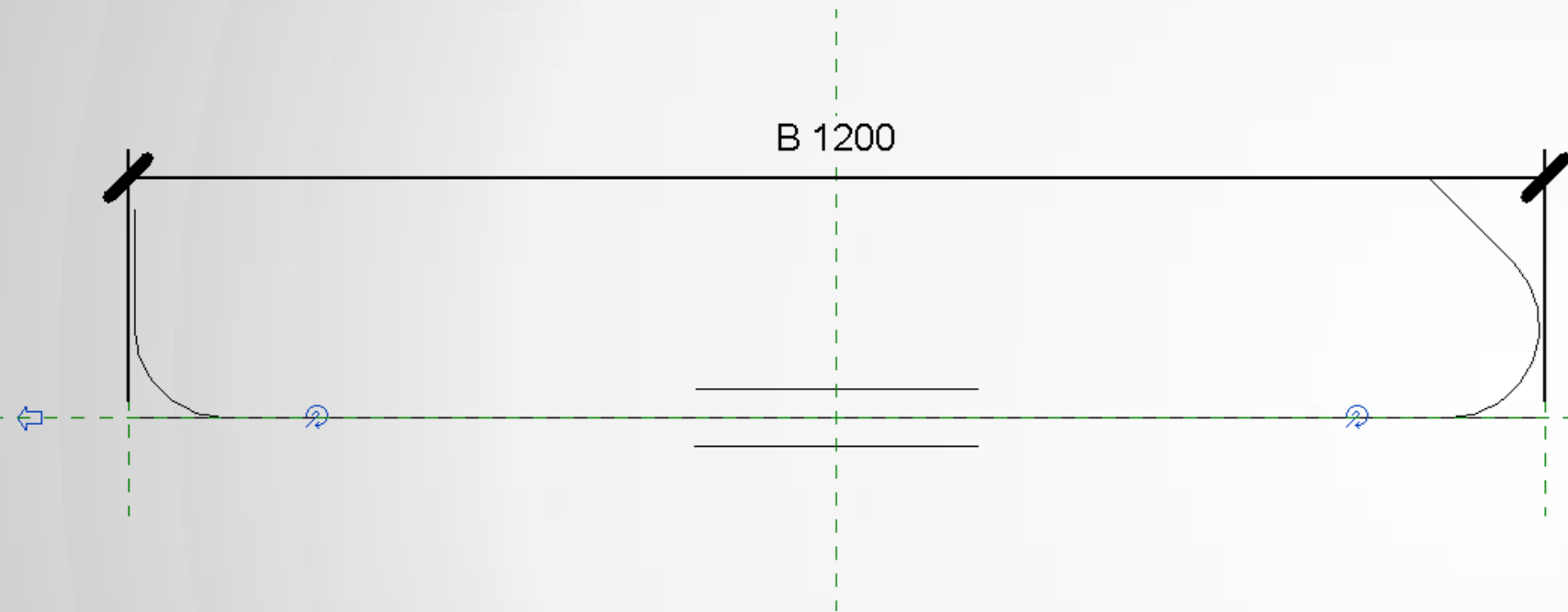
135° Hook



180° Hook

[SOURCE Revit 2015 Help](#)

Rebar Shape



Rebar Shape Parameters

Parameter	Value	Formula
Construction		
Style	Stirrup / Tie	=
Stirrup/Tie Attachment	Interior Face of Co	=
Start Tangent Hook Length (default)	115.0 mm	=
Start Hook Offset Length (default)	80.0 mm	=
Start Hook Length (default)	115.0 mm	=
Shape Image	<None>	=
Hook At Start	135°	=
Hook At End	90°	=
End Tangent Hook Length (default)	115.0 mm	=
End Hook Offset Length (default)	80.0 mm	=
End Hook Length (default)	115.0 mm	=
Bend Diameter (default)	155.0 mm	=
Bar Diameter (default)	10.0 mm	=
Dimensions		
R (default)	0.0 mm	=
O (default)	0.0 mm	=
K (default)	0.0 mm	=
J (default)	80.0 mm	= End Hook Offset Length
H (default)	80.0 mm	= Start Hook Offset Length
G (default)	115.0 mm	=
F (default)	0.0 mm	=
E (default)	0.0 mm	=
D (default)	0.0 mm	=
C (default)	115.0 mm	= End Hook Length
B (default)	1200.0 mm	=
A (default)	115.0 mm	= Start Hook Length

Parameters

Add...

Modify...

Remove

OK Cancel Apply Help

Rebar Hook Lengths

Rebar Bar Type: #04 Rebar Bar Diameter: 1.27

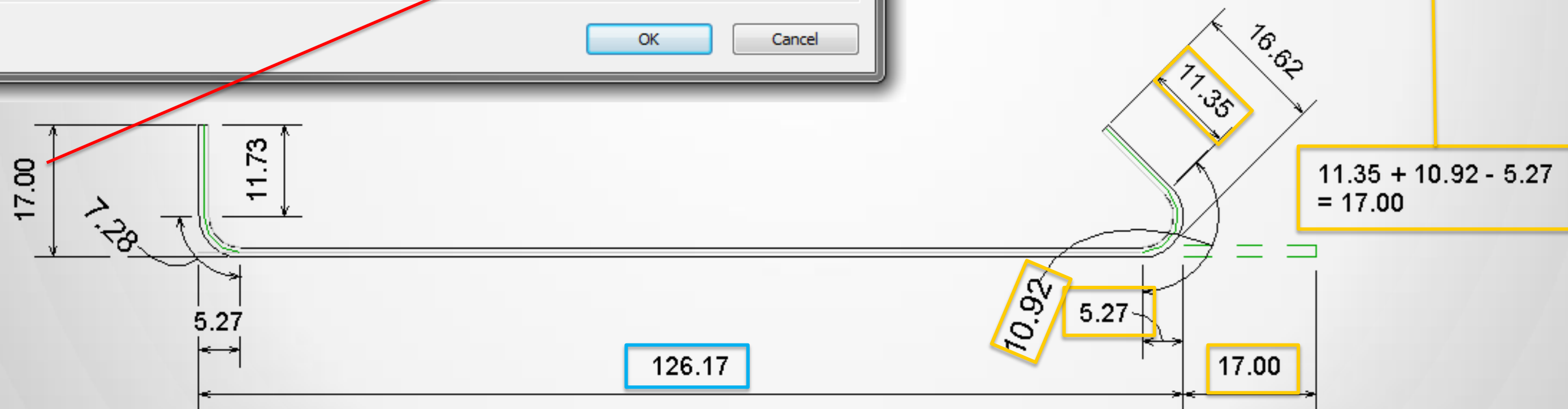
Rebar Hook Length can be automatically calculated based on the Rebar Hook Extension Multiplier property, or the Hook Length can be manually overridden here. The Offset Length is optional and is only used for scheduling

Rebar Hook Type	Auto Calculation	Hook Length	Tangent	Offset Length
<input checked="" type="checkbox"/> Standard - 90 deg.	<input type="checkbox"/>	17.97	17.97	
<input checked="" type="checkbox"/> Standard - 180 deg.	<input type="checkbox"/>	21.99	17.97	10.54
<input checked="" type="checkbox"/> Stirrup/Tie - 90 deg.	<input type="checkbox"/>	17.00	17.00	
<input checked="" type="checkbox"/> Stirrup/Tie - 135 deg.	<input type="checkbox"/>	17.00	16.62	7.39
<input checked="" type="checkbox"/> Stirrup/Tie Seismic - 135	<input type="checkbox"/>	17.00	16.62	7.39

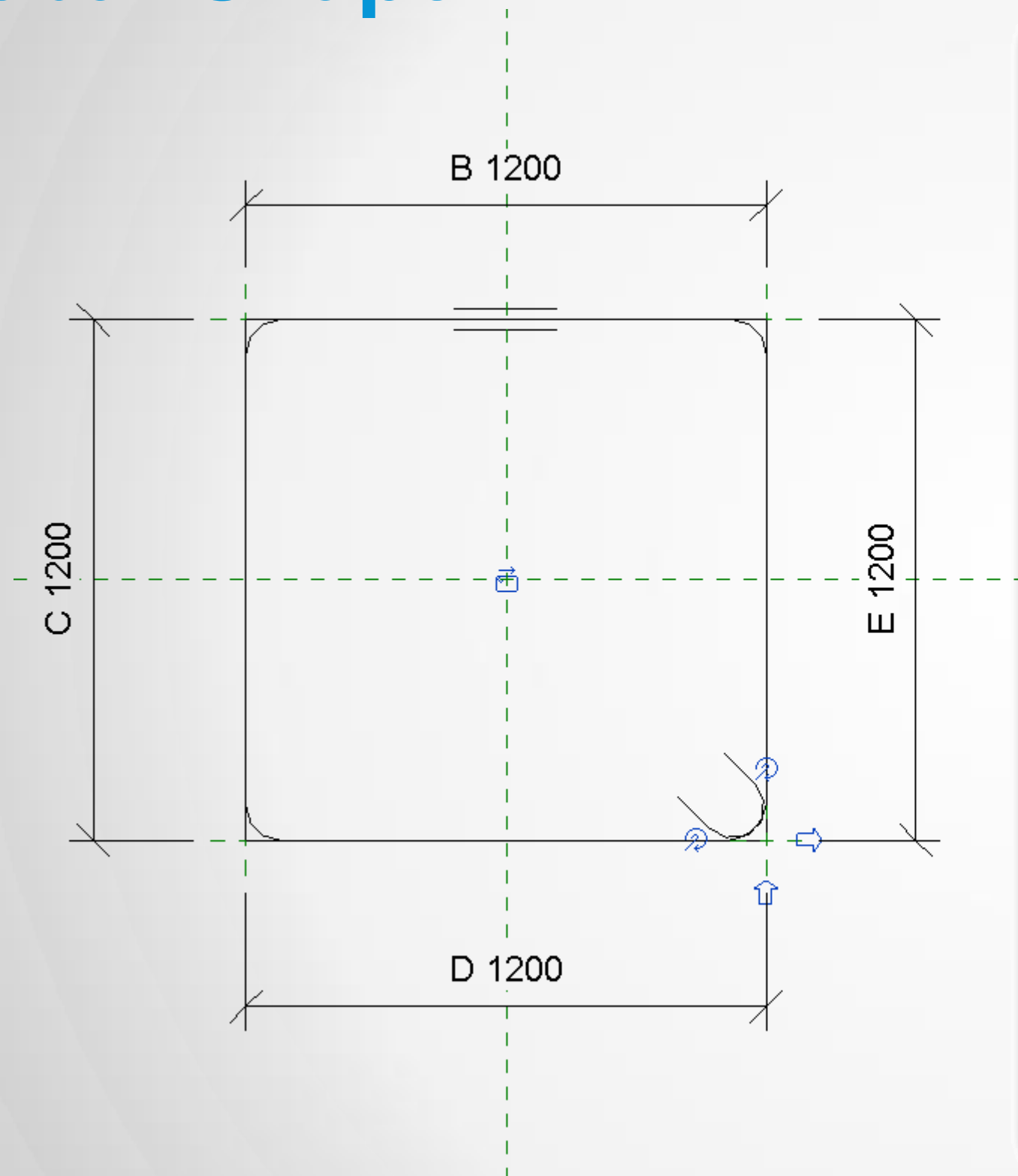
OK Cancel

Dimensions

Bar Length	156.91
Total Bar Length	156.91
A	17.00
B	126.17
C	17.00
D	0.00
E	0.00
F	0.00
G	11.50
H	8.00
J	7.39
K	0.00
O	0.00
R	0.00



Rebar Shape



Rebar Shape Parameters

Parameter	Value	Formula
Construction		
Style	Stirrup / Tie	=
Stirrup/Tie Attachment	Interior Face of Cover Referenc	=
Start Tangent Hook Length (defa	115.0 mm	=
Start Hook Offset Length (default	80.0 mm	=
Start Hook Length (default)	115.0 mm	=
Shape Image	<None>	=
Hook At Start	135°	=
Hook At End	135°	=
End Tangent Hook Length (defau	115.0 mm	=
End Hook Offset Length (default)	80.0 mm	=
End Hook Length (default)	115.0 mm	=
Bend Diameter (default)	155.0 mm	=
Bar Diameter (default)	10.0 mm	=
Dimensions		
R (default)	0.0 mm	=
O (default)	0.0 mm	=
K (default)	0.0 mm	=
J (default)	0.0 mm	=
H (default)	80.0 mm	=
G (default)	115.0 mm	=
F (default)	115.0 mm	= End Hook Length
E (default)	1200.0 mm	=
D (default)	1200.0 mm	=
C (default)	1200.0 mm	=
B (default)	1200.0 mm	=
A (default)	115.0 mm	= Start Hook Length

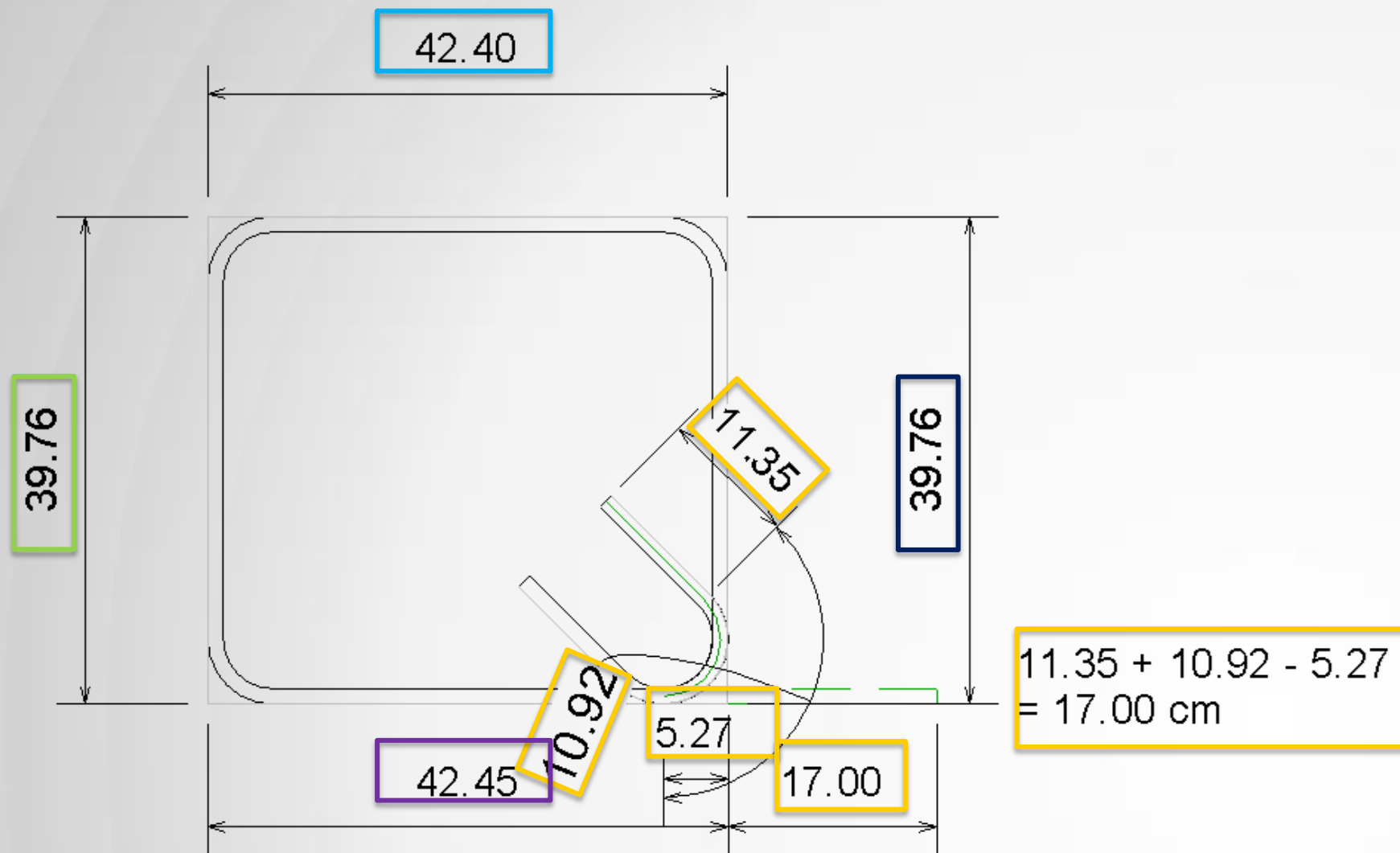
Parameters

Add...

Modify...

Remove

OK Cancel Apply Help



Dimensions	
Bar Length	188.59
Total Bar Length	188.59
A	17.00
B	42.40
C	39.76
D	42.45
E	39.76
F	17.00
G	0.00
H	8.00
J	0.00
K	0.00
O	0.00
R	0.00

Rebar Hook Lengths

Rebar Bar Type: #04 Rebar Bar Diameter: 1.27

Rebar Hook Length can be automatically calculated based on the Rebar Hook Extension Multiplier property, or the Hook Length can be manually overridden here. The Offset Length is optional and is only used for scheduling

Rebar Hook Type	Auto Calculation	Hook Length	Tangent	Offset Length
<input checked="" type="checkbox"/> Standard - 90 deg.	<input type="checkbox"/>	17.97	17.97	
<input checked="" type="checkbox"/> Standard - 180 deg.	<input type="checkbox"/>	21.99	17.97	10.54
<input checked="" type="checkbox"/> Stirrup/Tie - 90 deg.	<input type="checkbox"/>	17.00	17.00	
<input checked="" type="checkbox"/> Stirrup/Tie - 135 deg.	<input type="checkbox"/>	17.00	16.62	7.39
<input checked="" type="checkbox"/> Stirrup/Tie Seismic - 135	<input type="checkbox"/>	17.00	16.62	7.39

OK Cancel

Generic Model Bounding Box Family

- When the geometry is very complex, there are many rebar hosted in a single element or Revit slows down whenever try to put a rebar. One way to fix it is to put a bounding box around the element and host the rebars to that box

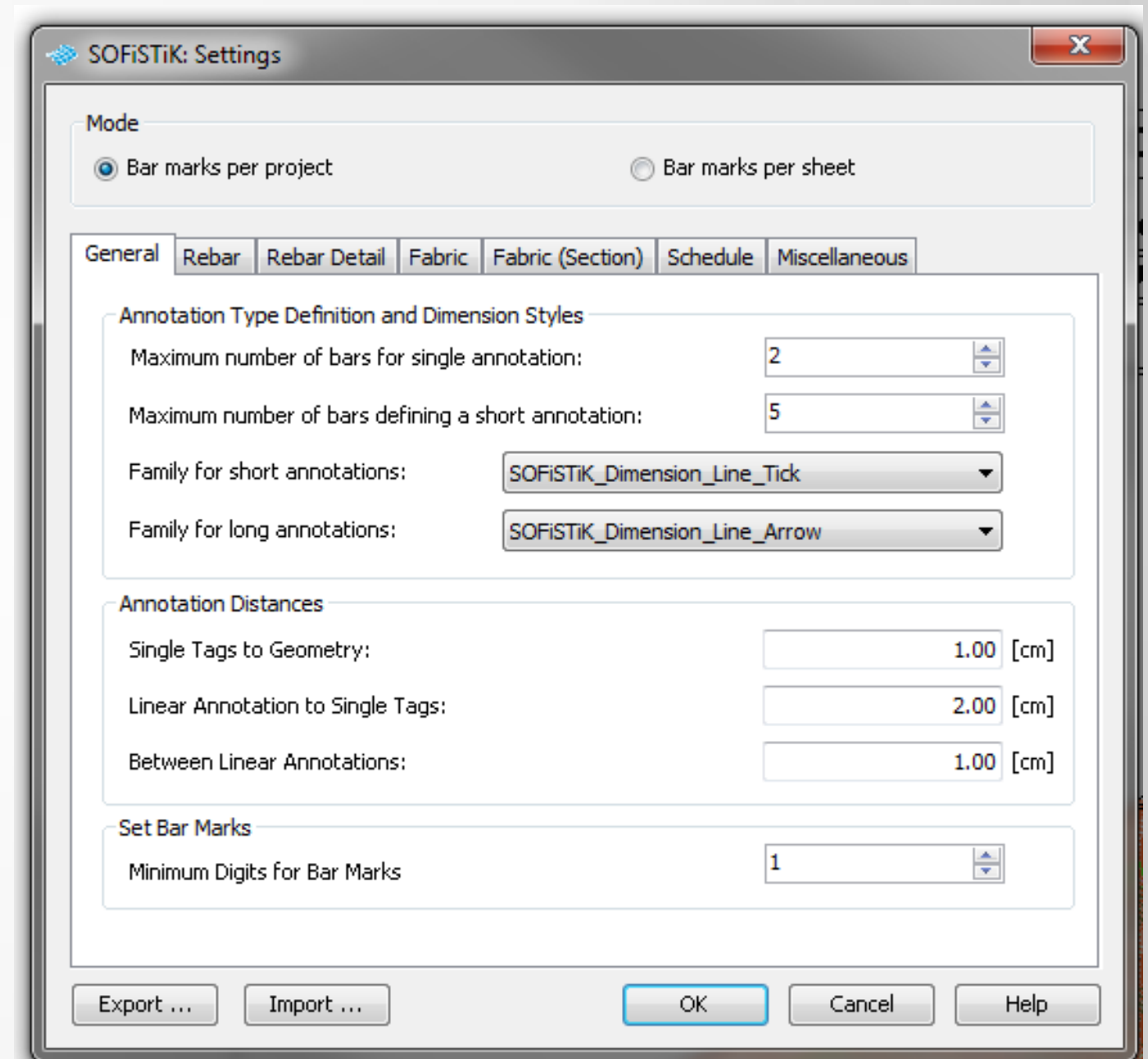


SOFiSTiK

Exporting rebar data to bending machine

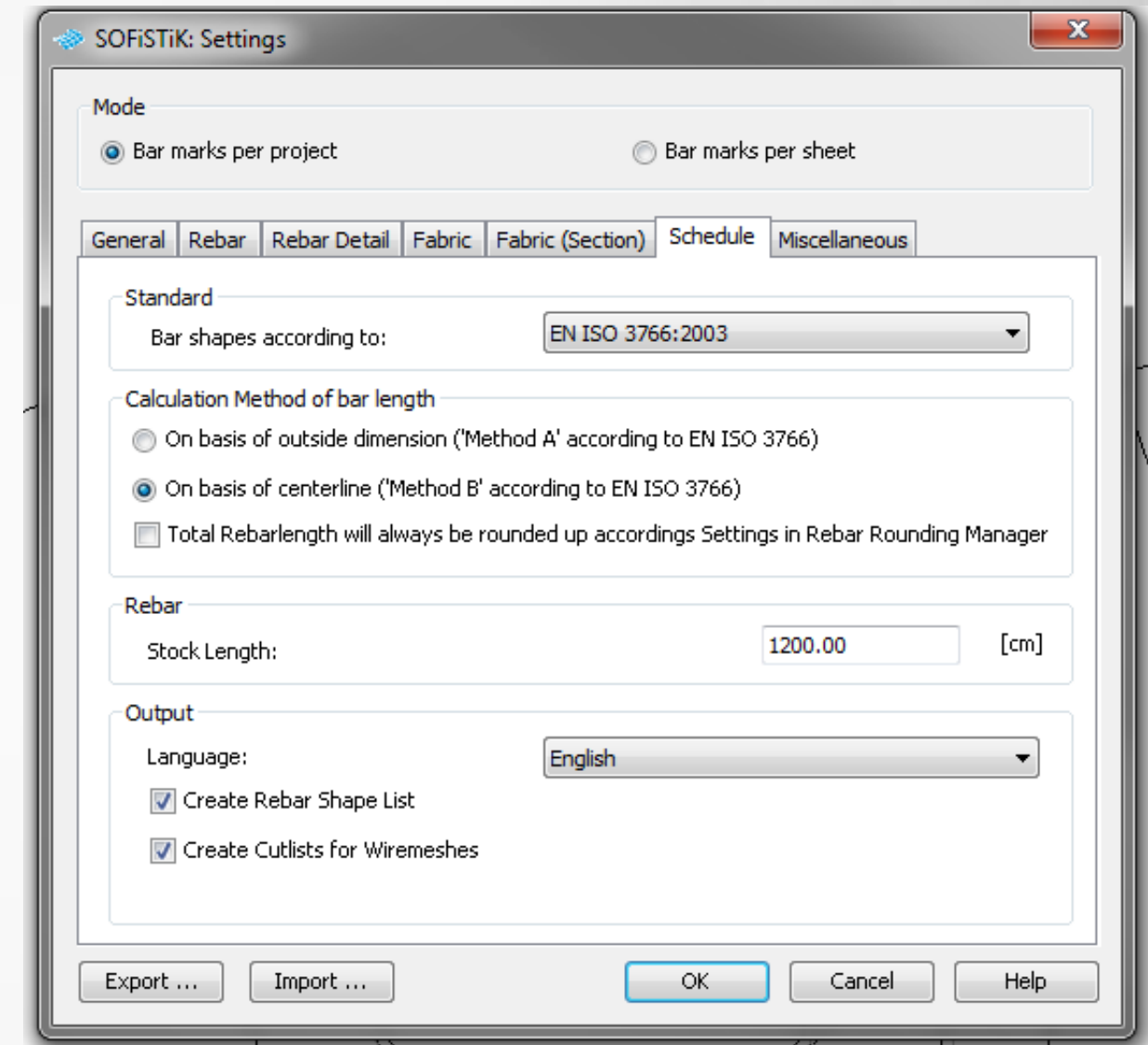
Settings

- In SOFiSTiK there are two ways to get the rebar schedule.
 - **Bar marks per project.** All the rebars of the file are exported
 - **Bar marks per sheet.** Only the rebar in the sheet



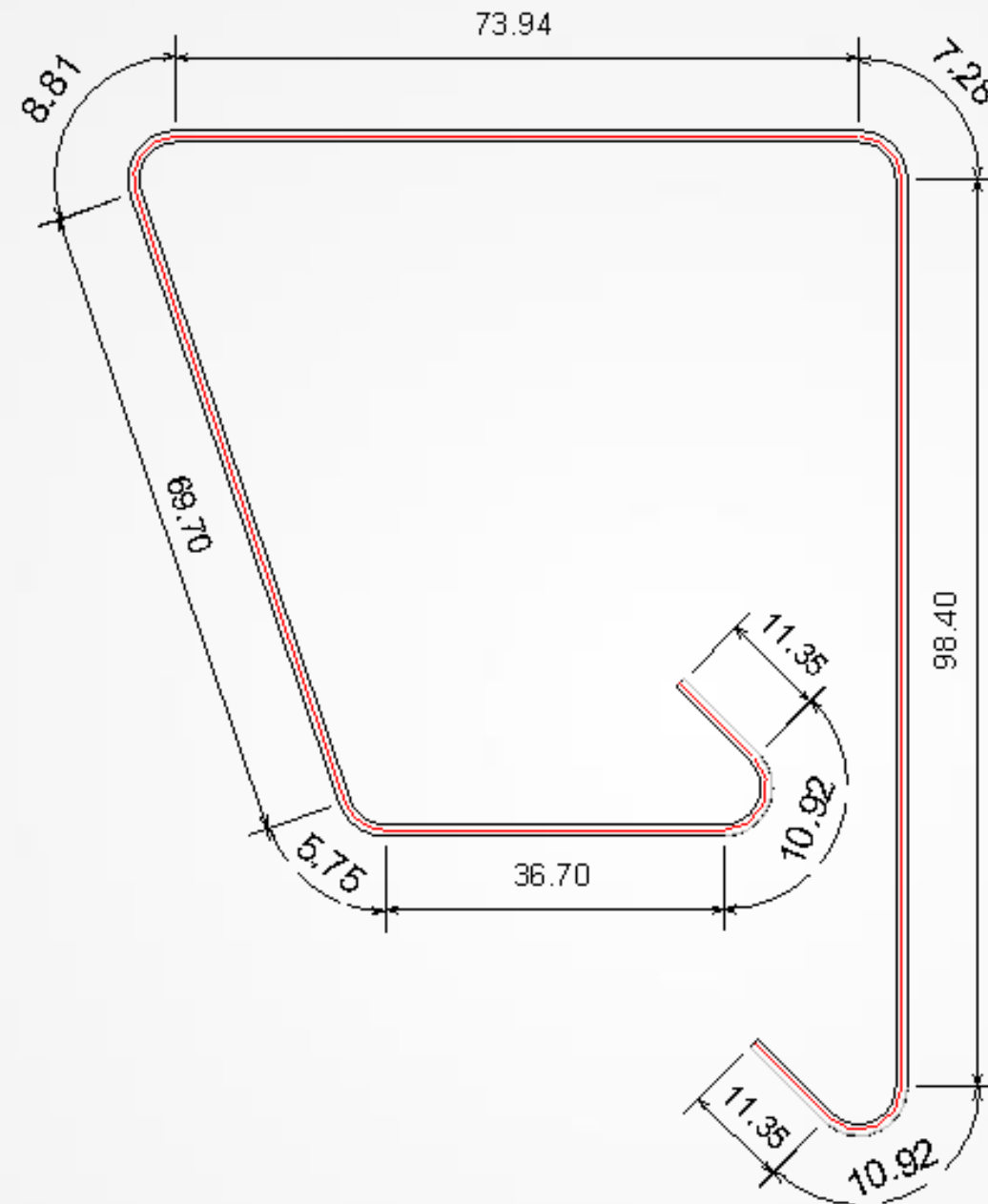
Settings – Rebar Length

- Revit. Use the “real” rebar length using the center line but all the dimension parameter are related to the exterior.
- SOFiSTiK. There are two options:
 - Method A. Use the projection of the exterior faces to calculate the bar length and the hook length
 - Method B. Use the same as Revit using the center lines



Rebar Length Differences

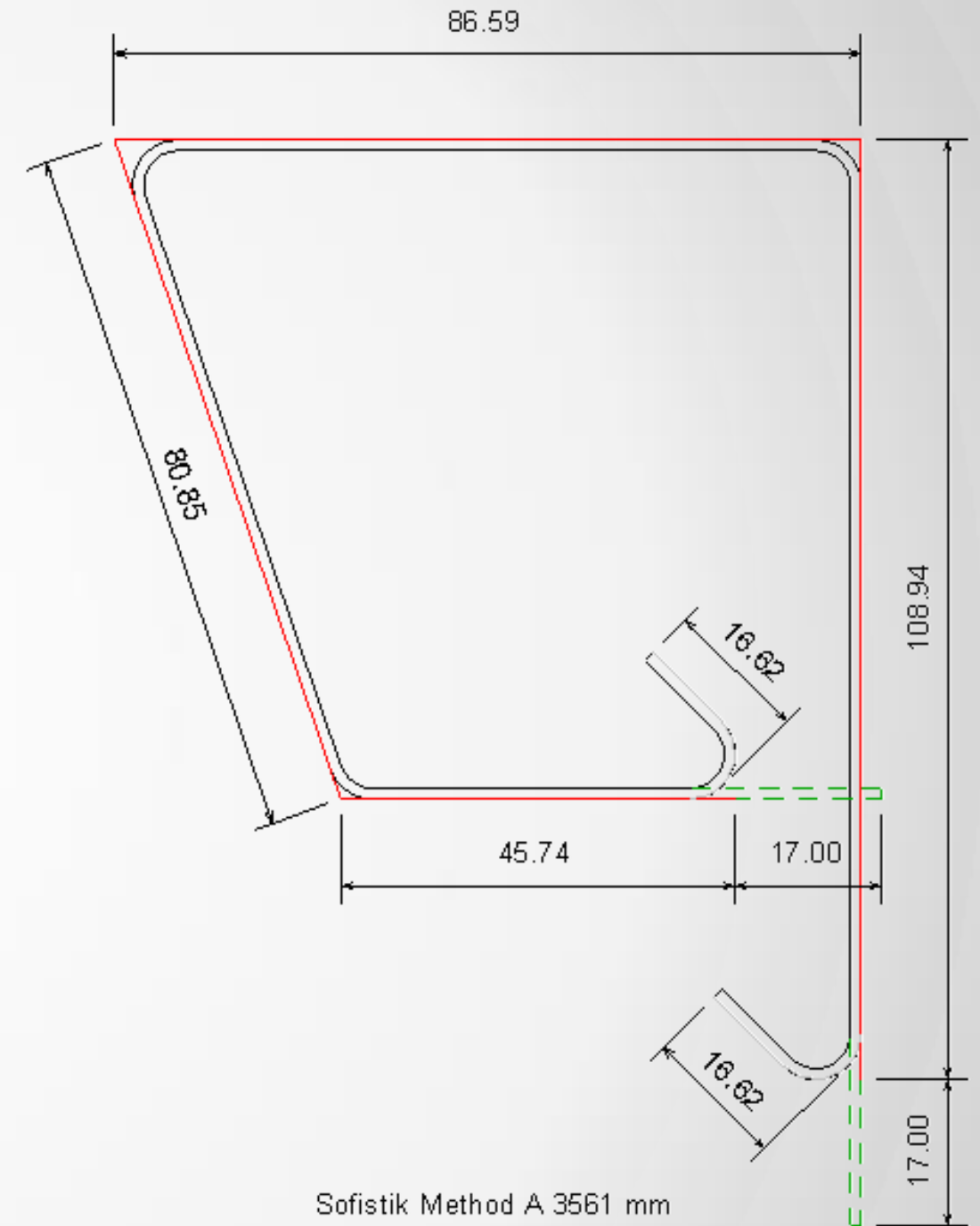
Structural	
Reinforcement Volume	0.0004 m ³
Dimensions	
Bar Length	345.13
Total Bar Length	345.13
A	17.00
B	108.94
C	84.48
D	78.74
E	45.74
F	0.00
G	17.00
H	122.00
J	0.00
K	30.00
O	0.00
R	0.00
aA	108.947°
Identity Data	
Image	
Comments	
Mark	aa
Sub-Zona	
Pertenece a	
Ensamble	
SOFiSTiK_Layer	
SOFiSTiK_Bar_Length	356.11
SOFiSTiK_Hosts_Mark	
SOFiSTiK_Rebar_Set	
SOFiSTiK_ShapeCode	
SOFiSTiK_Mark_TotalQua...	
SOFiSTiK_SubMark	
SOFiSTiK_Running_Length	<input checked="" type="checkbox"/>
Host Category	Structural Framing
Phasing	



Revit BarLength = 3451 mm

Sofistik Method B = 3451 mm

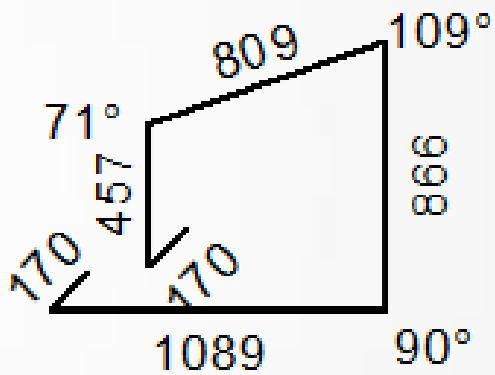
Center Line



Sofistik Method A 3561 mm

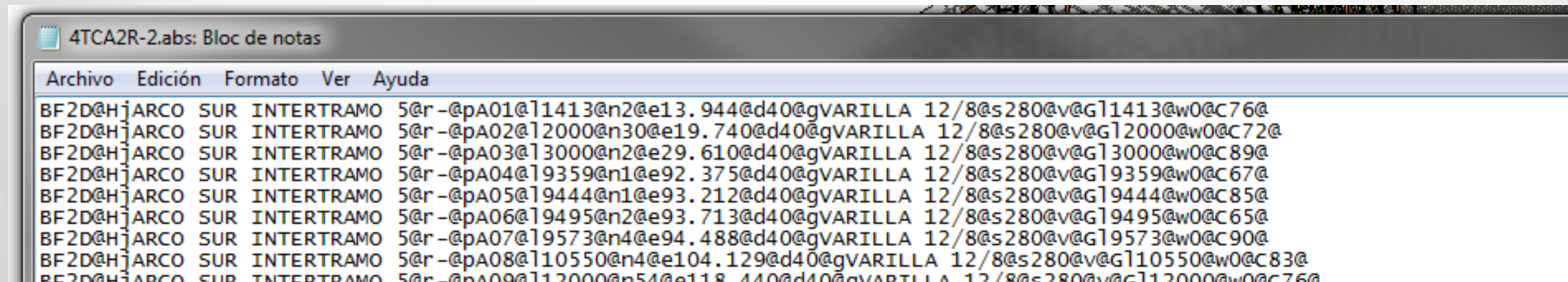
Exterior Rebar Faces

SOFiSTiK Schedule Example

BENDING SCHEDULE Steelgrade: VARILLA 4/8																																																		
Bar-mark	Bar-diameter	Length of each bar	Total number	Total length	dbr ds	Shape code	End-hook		Bending dimensions																																									
									a	b	c	d	e	R	h																																			
AA	12.7	3561	2	7122	6.299	99	2	2	<div><p>The bending diagram shows a path starting at (0,0) and ending at (1089, 866). The segments are: 1. Horizontal from (0,0) to (1089,0) with length 1089 and angle 90°. 2. Vertical from (1089,0) to (1089,866) with length 866 and angle 109°. 3. Diagonal from (1089,866) to (764, 866+261) with length 809 and angle 71°. 4. Diagonal from (764, 1127) to (0, 1127) with length 764 and angle 135°. The total length is 1089 + 866 + 809 + 764 = 3561.</p><table><tr><th>Nr.</th><th>dx</th><th>dy</th><th>l</th><th>>°</th></tr><tr><td></td><td></td><td></td><td>170</td><td>135</td></tr><tr><td>1</td><td>1089</td><td>0</td><td>1089</td><td>90</td></tr><tr><td>2</td><td>0</td><td>866</td><td>866</td><td>109</td></tr><tr><td>3</td><td>-764</td><td>-261</td><td>809</td><td>71</td></tr><tr><td>4</td><td>0</td><td>-456</td><td>457</td><td>135</td></tr><tr><td></td><td></td><td></td><td>170</td><td></td></tr></table></div>							Nr.	dx	dy	l	>°				170	135	1	1089	0	1089	90	2	0	866	866	109	3	-764	-261	809	71	4	0	-456	457	135				170	
Nr.	dx	dy	l	>°																																														
			170	135																																														
1	1089	0	1089	90																																														
2	0	866	866	109																																														
3	-764	-261	809	71																																														
4	0	-456	457	135																																														
			170																																															

SOFiSTiK BVBS File

- When creates the Schedule, in the same directory creates a file with the name of the Revit file and the extension *.abs if the file is opened on notepad shows all the rebar information in the standard BVBS format.
- This file feeds the software of the rebar bender machines



```
4TCA2R-2.abs: Bloc de notas
Archivo  Edición  Formato  Ver  Ayuda
BF2D@H  ARCO    SUR    INTERTRAMO  5@r-@pA01@11413@n2@e13.944@d40@gVARILLA 12/8@s280@v@g11413@w0@c76@
BF2D@H  ARCO    SUR    INTERTRAMO  5@r-@pA02@12000@n30@e19.740@d40@gVARILLA 12/8@s280@v@g12000@w0@c72@
BF2D@H  ARCO    SUR    INTERTRAMO  5@r-@pA03@13000@n2@e29.610@d40@gVARILLA 12/8@s280@v@g13000@w0@c89@
BF2D@H  ARCO    SUR    INTERTRAMO  5@r-@pA04@19359@n1@e92.375@d40@gVARILLA 12/8@s280@v@g19359@w0@c67@
BF2D@H  ARCO    SUR    INTERTRAMO  5@r-@pA05@19444@n1@e93.212@d40@gVARILLA 12/8@s280@v@g19444@w0@c85@
BF2D@H  ARCO    SUR    INTERTRAMO  5@r-@pA06@19495@n2@e93.713@d40@gVARILLA 12/8@s280@v@g19495@w0@c65@
BF2D@H  ARCO    SUR    INTERTRAMO  5@r-@pA07@19573@n4@e94.488@d40@gVARILLA 12/8@s280@v@g19573@w0@c90@
BF2D@H  ARCO    SUR    INTERTRAMO  5@r-@pA08@110550@n4@e104.129@d40@gVARILLA 12/8@s280@v@g110550@w0@c83@
BF2D@H  ARCO    SUR    INTERTRAMO  5@r-@pA09@112000@n54@e118.440@d40@gVARILLA 12/8@s280@v@g112000@w0@c76@
```


Lessons Learned

- Its very important to know the bender mandrels diameter before sending to production and change the hook bend diameter on Revit
- SOFiSTiK uses the Mark for its schedule, all the rebars that their mark are empty the program omits them in the schedule.

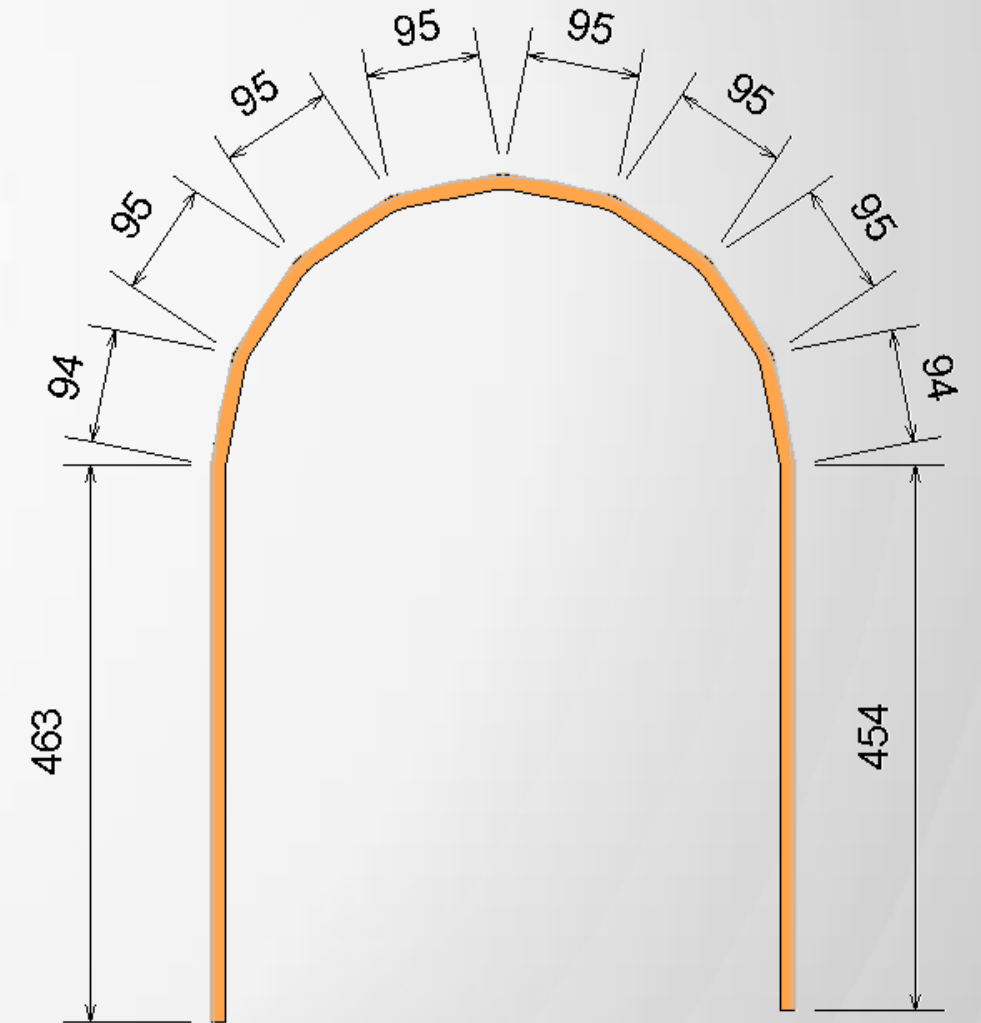
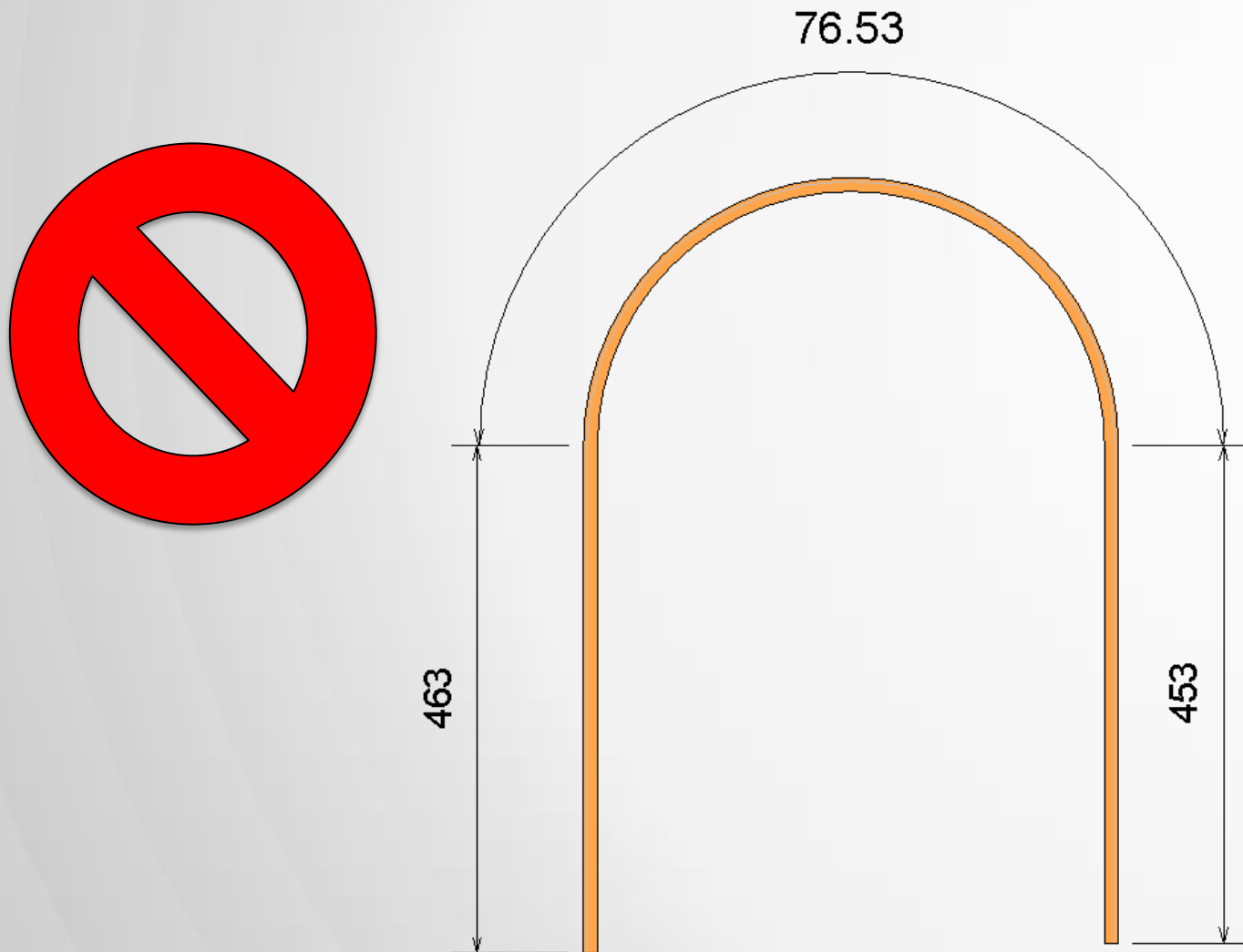


Lessons Learned

- The mark needs to be unique, if the pieces are different SOFiSTiK finds inconsistent information and omits the duplicated rebar from the schedule
- Its necessary to check the rebar max stock length on the plant for each rebar diameter and to cut them in the model.

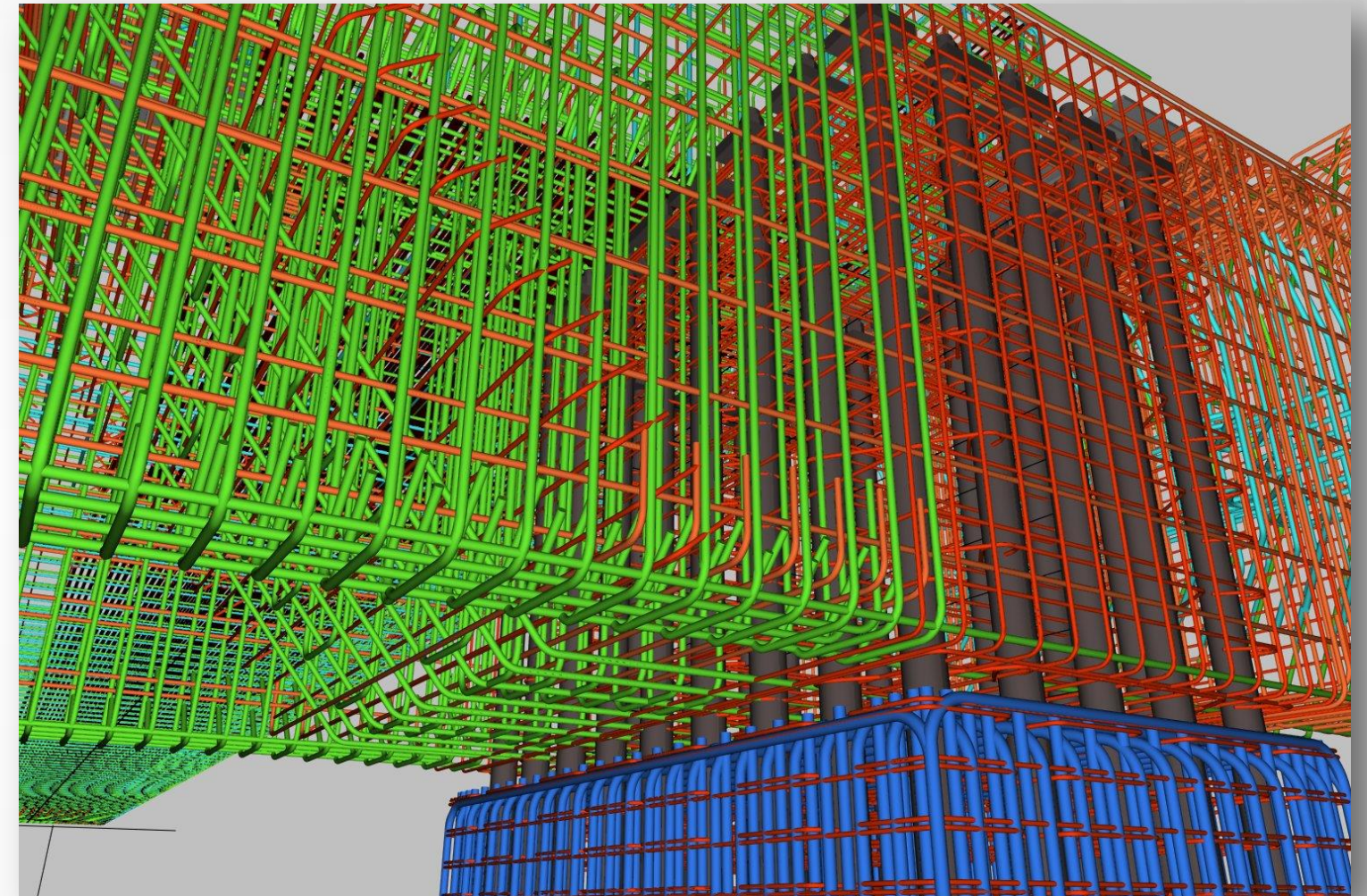
Lessons Learned

- The rebar segments must be linear, if the shape contains arcs needs to be segmented.



Clash Detection in Navisworks

- From the Fabricator point of view the most important clashes to find are:
 - The connections with other elements
 - Ducts of strands
 - The space between TA girders is enough to fit the central
- Almost all clashes detected are corrected on the fabric and not on site



Autodesk Navisworks Manage 2015 Untitled.nwf

Home Viewpoint Review Animation View Output Render BIM 360

Append Refresh Reset All... File Options Select Save Selection Select All Select Same Selection Tree Find Items Quick Find [Q]Sets Hide Require Hide Unselected Unhide All Links Quick Properties Properties Clash Detective TimeLiner Quantification Autodesk Rendering Animator Scripter Appearance Profiler Batch Utility Compare DataTools

Project Select & Search Visibility Display Tools

Clash Detective

There are currently no clash tests defined. Add Test

Rules Select Results Report

Selection A

Standard

- 140913_EST_FUSTE4C_Q18_1285.nwc
- 20140826_2E_4TA2Q-09.nwc

Selection B

Standard

- 140913_EST_FUSTE4C_Q18_1285.nwc
- 20140826_2E_4TA2Q-09.nwc

Settings

Type: Hard Tolerance: 0.1

Link: None Step (sec): 0.1

☐ Composite Object Clashing

Run Test

Clash Detective Selection Tree

AutoSaved: C:\Users\GRAJALESH\AppData\Roaming\Autodesk Navisworks Manage 2015\AutoSave\Untitled.Autosave6.nwf

1 of 1 3292 MB

Autodesk Navisworks Manage 2015 Interseccion.nwf

Home Viewpoint Review Animation View Output Sectioning Tools Render BIM 360

Enable Sectioning Planes Current: Plane 1 Alignment: *Custom* Link Section Planes Move Rotate Scale Fit Selection Save Viewpoint

Clash Detective

Test 1 Last Run: jueves, 27 de noviembre de 2014 11:35:17 a.m. Clashes - Total: 3 (Open: 2 Closed: 1)

Name	Status	Clashes	New	Active	Reviewed	Approved	Resolved
Test 1	Done	3	1	1	0	0	1

Add Test Reset All Compact All Delete All Update All

Rules Select Results Report

Selection A

- Standard
- COO_AS_Intertramo5_v6.rvt
 - <No level>
 - Level 1
 - Generic Models
 - Roofs
 - Structural Columns
 - Structural Framing
 - Q1
 - Q2
 - Q3
 - Q4
 - Q5
 - R1
 - R2
 - TA4
 - TC4
 - TCA4
 - TA6d
 - TA6i
 - TA6d_a
 - TA6i_a
 - TC6
 - Walls
 - superficie_v3.dwg

Selection B

- Standard
- COO_AS_Intertramo5_v6.rvt
 - <No level>
 - Level 1
 - Generic Models
 - Roofs
 - Structural Columns
 - Structural Framing
 - Q1
 - Q2
 - Q3
 - Q4
 - Q5
 - R1
 - R2
 - TA4
 - TC4
 - TCA4
 - TA6d
 - TA6i
 - TA6d_a
 - TA6i_a
 - TC6
 - Walls
 - superficie_v3.dwg

Settings

Type: Hard (Conservative) Tolerance: 0.000 m

Link: None Step (sec): 0.1

☒ Composite Object Clashing

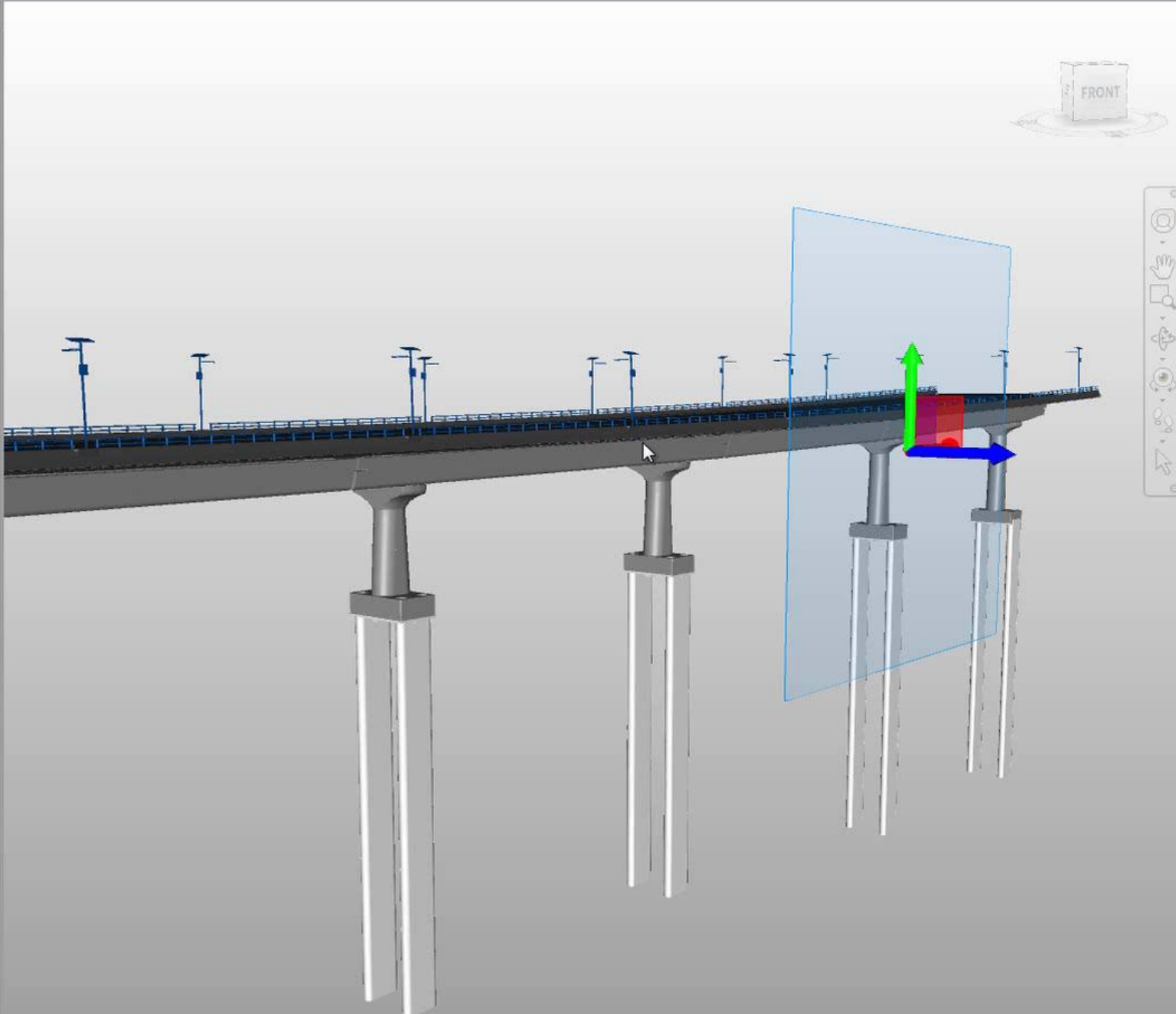
Run Test

Clash Detective Selection Tree

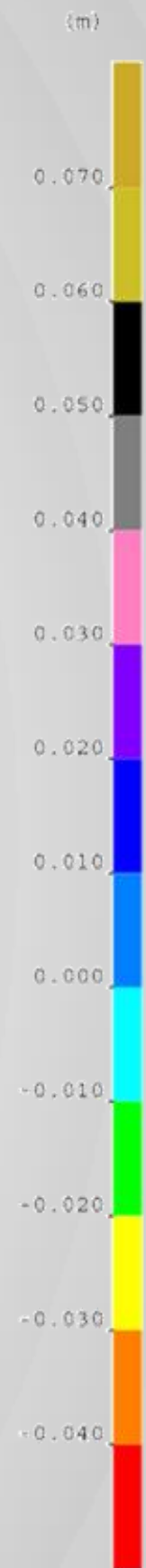
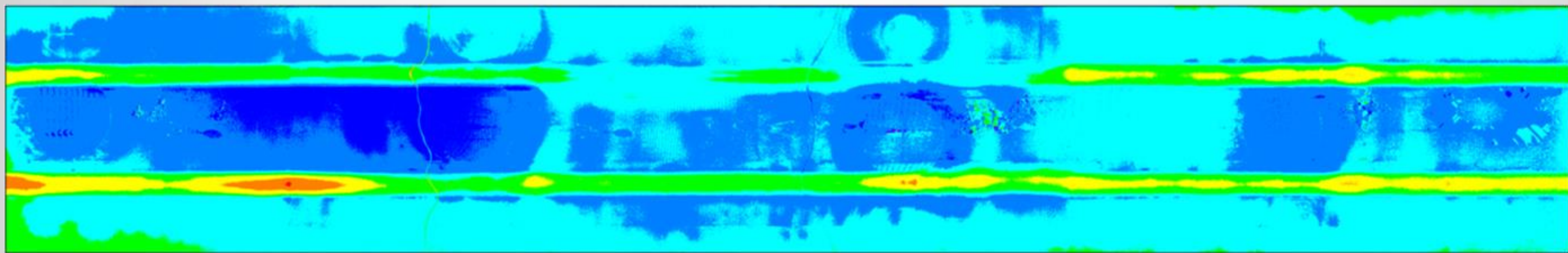
Recovered AutoSave: C:\Users\GRAJALESH\AppData\Roaming\Autodesk Navisworks Manage 2015\AutoSave\Interseccion.Autosave6.nwf

1 of 1 634 MB

3D View Walkthrough

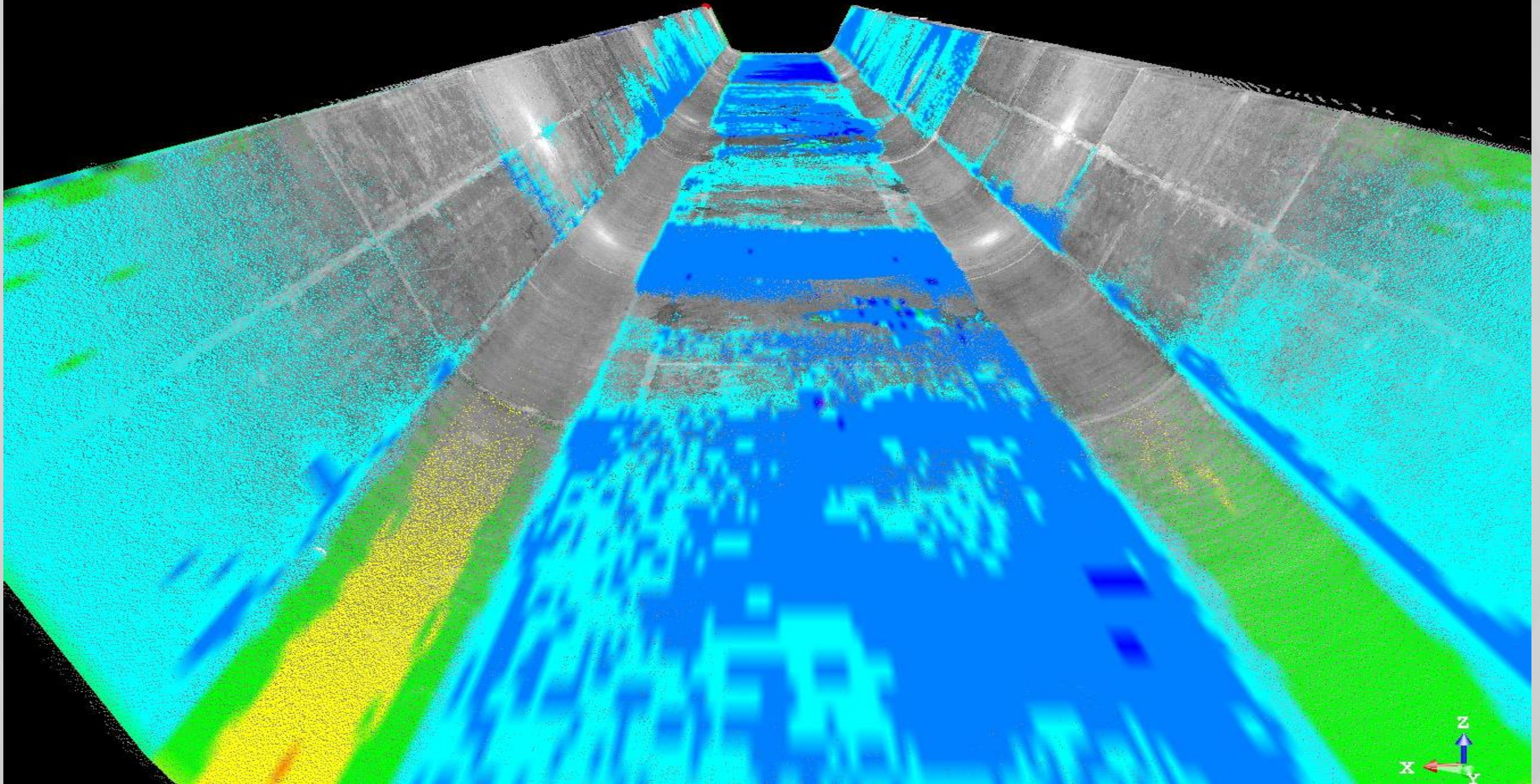


Point Clouds comparison using CloudCompare



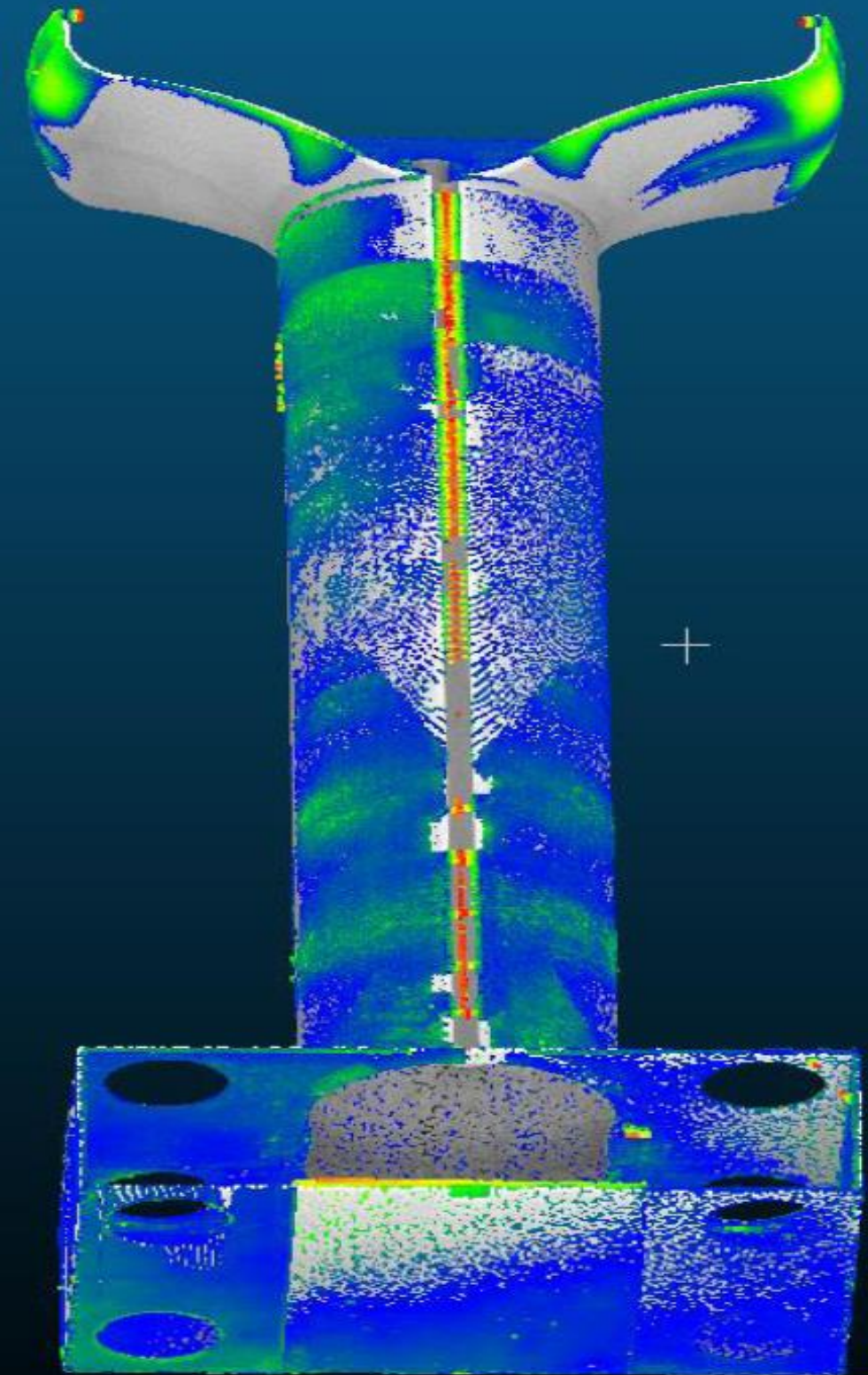
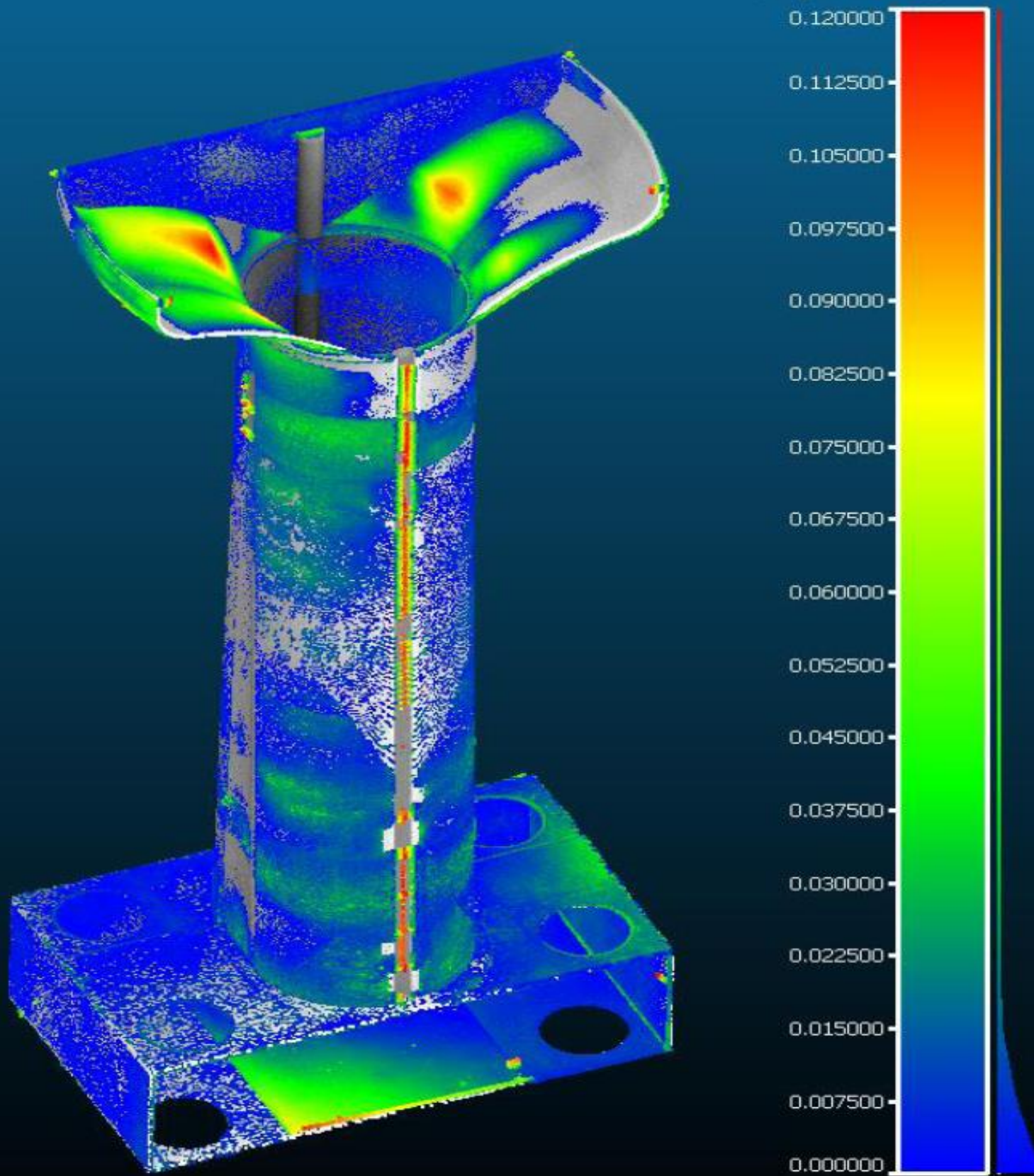
10.000 m





La escala gráfica está en [m]

C2C absolute distances[Height Function][k=6]



Fabrication Process







10.02.2011 09:25



OBSERVACIONES		ORDEN DE COMPRA		O B R A		REMISION / No. DE ENVIO	
Prueba		1100		SW			
HABITADO DE ACERO DE REQUERO APROBADO		ELEMENTO		FECHA		PREZAS	
ICA		FC02-C				62	
CLIENTE		DIMENSIONES		PLANO		NILOS	
[Redacted]		4100		ICA		[Redacted]	



10.02.2011 09:27













































Q&A

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- hector.grajales@ica.mx

Session Feedback

- Via the Survey Stations, email or mobile device
- AU 2014 passes given out each day!
- Best to do it right after the session
- Instructors see results in real-time



